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BAY GEOLOGICAL SERVICES LTD



Revised Applications for Take, Use and Discharge of Tranche 2 Groundwater Ruataniwha Basin

August 2021



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Ruataniwha Basin

Revised Applications for Take, Use and Discharge of Tranche 2 Groundwater Ruataniwha Basin

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Contents

EXEC	CUTIVE SU	JMMARY	1
1	Intro	duction	4
	1.1	Tranche 2 Deep Groundwater Allocation >	4
	1.2	The Tranche 2 Applicants	4
	1.3	Collaborative Approach	5
	1.4	Maintaining the PC6 Minimum Flows	6
2	Plan	Change 6	8
3	Tranc	che 2 Applicants	10
	3.1	Te Awahohonu Forest Trust (TAFT)	10
	3.2	Papawai Partnership	11
	3.3	Tuki Tuki Awa	12
	3.4	Plantation Road Dairies (PRD)	13
	3.5	Springhill Dairies (formerly Ingleton Farms)	14
	3.6	I & P Farming (formerly Abernethy Partnership)	15
	3.7	Buchanan Trust No.2	16
	3.8	Purunui Trust	17
4	Cons	ent Categories	19
	4.1	Resource Management (National Environmental Standards for Freshwater)	
	Regu	lations 2020 (NES-F)	19
	4.2	Hawke's Bay Regional Resource Management Plan (RRMP)	21
	4.3	Summary	24
5	Appr	oach Taken	25
	5.1	Irrigable Area and Crop Water Demand	25
	5.2	Augmentation Take and Discharge Locations	25
	5.3	Consent Durations	26
	5.4	Protocols for the Takes and Augmentation	26
6	Grou	ndwater Modelling	29
	6.1	Scenarios Modelled	31
	6.2	Revised Overall Tranche 2 Proposal	31
7	Statu	tory Matters	35
8	Asses	sment of Environmental Effects	36
	8.1	Surface Water Flow Effects	36
	8.2	Groundwater Level and Well Interference Effects	39
	8.3	Positive Effects	42
9	Asses	sment of Planning and Policy Documents	46
	9.1	National Policy Statement for Freshwater Management (NPS-FM) 2020	46
	9.2	Hawke's Bay Regional Policy Statement (RPS)	47

	9.3	Hawke's Bay Regional Resource Management Plan (RRMP)	49
10	Section	n 104D Assessment	51
11	Conse	nt Conditions	52
12	Part 2 RMA		
	11.1	Section 5 - Purpose of the RMA	53
	11.2	Section 6 - Matters of National Importance	53
	11.3	Section 7 - Other Matters	54
	11.4	Section 8 - Treaty of Waitangi	55
	11.5	Conclusions	55
13	Summ	ary and Conclusion	56
APPEN	DIX A –	Agreed Interpretation of RRMP Policies	59
APPEN	DIX B – A	Aqualinc 2021 Groundwater Modelling Report	60
APPEN	DIX C – '	Wetlands and Surface Water Effects Assessment	61
APPEN	DIX D —	Well Interference Assessment Report	62
APPEN	DIX E –S	hallow Groundwater Bores Assessment	63
APPEN	DIX F - P	Proposed Resource Consent Conditions	64

EXECUTIVE SUMMARY

Plan Change 6 (PC6) to the Hawke's Bay Regional Resource Management Plan (RRMP) became operative on 1 October 2015. PC6 set groundwater allocation limits for the Ruataniwha Basin which is located within the upper Tukituki River catchment. Included within those limits was an allocation of deep groundwater that PC6 labelled "Tranche 2".

In response to the establishment of the Tranche 2 allocation, during the period 19 November 2014 to 1 May 2017 the following seven parties lodged resource consent applications to abstract Tranche 2 deep groundwater¹:

- Te Awahohonu Forest Trust (TAFT)
- Springhill Dairies (formerly Ingleton Farms)
- Tuki Tuki Awa
- Plantation Road Dairies
- I & P Farming (formerly Abernethy Partnership)
- Papawai Partnership
- Buchanan Trust No. 2

Plantation Road Dairies also amended their original application in January-April 2020, to reduce the volume of water they had applied for from 6,081,499 m³/year to 3,751,225 m³/year. Consequently, additional water was made available to Papawai Partnership to increase the volume they originally applied for by 1,052,455 m³/year, and to I & P Farming to increase their original volume by 722,888 m³/year. Purunui Trust applied in April 2020 for the remaining water (544,931 m³/year).

A fundamental element of Tranche 2 groundwater abstraction is the RRMP requirement to augment river flows to mitigate the impact of the Tranche 2 deep groundwater abstractions on surface water bodies. The augmentation water is also abstracted from deep groundwater, and it forms part of the Tranche 2 allocation. This means that each Tranche 2 applicant must specify the maximum volume of deep groundwater they propose to abstract for irrigation and the maximum they propose to abstract for augmentation purposes.

The augmentation concept included in PC6 was novel, not only for Hawke's Bay, but for New Zealand generally. It was not instigated by the Hawke's Bay Regional Council (HBRC) but was introduced by the Board of Inquiry into the Tukituki Catchment Proposal in their June 2014 decision. Not surprisingly, the augmentation concept was not approached consistently by all of the applicants.

Each Tranche 2 groundwater abstraction will have an effect on groundwater levels and river flows across a wide area. The effects of all eight applicants then combine to produce Ruataniwha Basin-wide effects. Consequently, at a meeting with HBRC officials in March 2018, it was agreed that the applicants would work together in a collaborative manner to assess (or reassess) the cumulative effects of their proposed Tranche 2 abstractions.

This report is based on the outcomes of the collaborative modelling and effects assessments. It supersedes and replaces the individual assessments of environmental effects (AEEs) that were originally lodged by each of the eight applicants. However, while this AEE replaces the original AEEs, the original applications for resource consent remain in place.

 $^{^{1}}$ Two other parties also lodged applications. The first was HBRIC, the investment arm of the Hawke's Bay Regional Council. The second was J M Bostock Ltd. Both of those applications have been withdrawn.

Importantly, when the original Tranche 2 applications were lodged, it was unclear how the PC6 augmentation provisions should be interpreted and implemented. For example, it was unclear whether, or not, the effects of the deep groundwater augmentation abstractions on surface flows would themselves need to be mitigated by further augmentation, and so on. Some applicants (including TAFT, Springhill Dairies, Plantation Road Dairies and Buchanan No. 2 Trust) assumed that would be the case, while others did not.

Consequently, this report focuses on the total amount of water originally sought by each applicant, as it is understood that this cannot be increased in the absence of fresh consent applications.

Irrigation demand for each applicant has been calculated for the irrigable areas applied for using the Aqualinc in-house soil-water balance model, IrriCalc. From these calculations, the 90-percentile (or 9 in 10 years) irrigation demand (in mm/year) has been calculated, assuming that all existing and future irrigated land use is pasture, which typically has a larger seasonal water demand than other land uses. This demand is then multiplied by the irrigated area to calculate a maximum irrigation volume.

To provide greater flexibility for applicants to use Tranche 2 groundwater for the duration of the consents, the Applicants are seeking conditions that enable the volume of Tranche 2 groundwater to be used to irrigate crops and/or horticulture, or a mixture of pasture, crops and horticulture. While the volume of water used will be the same or less for any applicant irrigating solely pasture, the amount of land over which this water is applied may be greater than the area modelled in the Aqualinc report (in Appendix B to this report).

The proposed revised areas of the Applicants' properties to be irrigated are set out in the Revised Overall Tranche 2 Proposal in Table 1, in Section 6.2 of this report, and are set out in the proposed set of draft consent conditions in Appendix F to this report.

The applicants therefore seek final conditions that allow for flexibility of use of any Tranche 2 water taken. This will enable crop rotations (temporary) and permanent transitions to other land uses. It is not possible at this stage to specify exactly when, and where that landuse change will occur within the consented land areas. Any change in land use, to a less water demanding crop, will also allow a greater area of land to benefit from irrigation. This will increase the positive effects arising from the water take, while any effects will be addressed in the land use consent. The precise wording of these conditions will need to be further discussed with Council.

As an outcome of the modelling undertaken by Aqualinc, some adjustments to the rates of take of Tranche 2 groundwater for irrigation are also required to optimise the balance between irrigation and augmentation water use and mitigate effects on rivers during low-flow periods. For all but one applicant, the rates are proposed to be less than the rate originally applied for. For Tuki Tuki Awa, it is proposed that their original rate of take be increased (from 78 L/s daily average to 94 L/s daily average). This small increased take on one site is predicted to have negligible effects, as demonstrated in this AEE.

A consent duration of 20 years is sought for all of the applications. For consistency, and given the collaborative approach being applied, it is requested that a 20-year consent duration be applied to <u>each</u> of the eight applications.

This AEE is prepared in accordance with the requirements of Section 88 and the Fourth Schedule of the Resource Management Act 1991 (RMA), and it is intended to provide the information necessary to fully understand the proposal and any actual or potential effects that the proposed activity may have on the environment.

This AEE concludes that with the imposition of appropriate consent conditions, any actual and potential adverse environmental effects of the proposed groundwater takes will be negligible or no more than

minor, with the exception of the security of supply of groundwater to 4 existing bores within the Ruataniwha Basin which may be adversely affected to a minor extent. To the extent that some form of notification of this application is considered appropriate, then any such notification should be limited to those affected bore owners/users.

The proposed activity is consistent with the purpose and principles of the RMA.

In terms of section 104(1)(b), on the basis of the above assessment, the proposal is generally consistent with, and is not contrary to, the relevant objectives and policies of the National Policy Statement for Freshwater 2020, and the Hawke's Bay Regional Resource Management Plan (including the Regional Policy Statement).

Rule TT4 of the RRMP, as notified (as part of PC6), did not include provision for Tranche 2 groundwater takes and Rule TT4 became operative on 1 October 2015. Three of the applications were lodged prior to 1 October 2015. Regardless of this, based on legal advice received as to the effect of s 88A, Resource Management Act 1991 in circumstances where a previous rule becomes inoperative after lodgment, we consider that all eight applications should be assessed as Discretionary Activities. However, in the event that Non-Complying Activity status is required for the three applications lodged earlier, we consider that there is no difficulty in those applications passing both of the 'gateway' tests under s104D(1)(a) and (b) of the RMA.

Given the above, consent can be granted to all of the applications pursuant to sections 104, 104B, and 107, and, if necessary, s 104D, of the RMA, subject to the imposition of conditions under section 108 of the RMA (as proposed in Appendix F of this report).

1 Introduction

1.1 Tranche 2 Deep Groundwater Allocation >

Plan Change 6 (PC6) to the Hawke's Bay Regional Resource Management Plan (RRMP) became operative on 1 October 2015. PC6 set groundwater allocation limits for the Ruataniwha Basin which is located within the upper Tukituki River catchment. Included within those limits was an allocation of groundwater that PC6 labelled "Tranche 2".

Policy POL TT8(ca) of the RRMP now states:

"To manage the taking of surface water and groundwater in the Tukituki River catchment by:

[...]

(ca) Enabling additional groundwater to be abstracted as a discretionary activity (Table 5.9.5 Tranche 2) provided that river flows are augmented to maintain the relevant minimum flows specified in Table 5.9.3 commensurate to the scale of effect of the Tranche 2 groundwater take."

Table 5.9.5 sets a Tranche 2 deep groundwater allocation of 15,000,000 m³/year collectively for Zone 2 (Ruataniwha Basin north of the Waipawa River) and Zone 3 (Ruataniwha Basin south of the Waipawa River) of the Ruataniwha Plains.

Rule TT4(b) applies to the take and use of new groundwater takes (applied for after 4 May 2013) that are located within the Ruataniwha Basin Groundwater Allocation Zones 1 to 3. Such takes and uses are Discretionary Activities, subject to compliance with conditions, including a requirement that no new groundwater takes from Groundwater Allocation Zones 2 and 3 utilising Tranche 2 groundwater may be exercised under the rule unless, and until, augmentation flows are discharged that are commensurate to the scale of effect of the proposed takes.

1.2 The Tranche 2 Applicants

In response to the establishment of the Tranche 2 allocation, during the period 19 November 2014 to 1 May 2017 seven parties lodged resource consent applications to abstract Tranche 2 deep groundwater. Those parties are:

- Te Awahohonu Forest Trust (TAFT)
- Springhill Dairies (formerly Ingleton Farms)
- Tuki Tuki Awa
- Plantation Road Dairies
- I & P Farming (formerly Abernethy Partnership)
- Papawai Partnership
- Buchanan Trust No. 2

Plantation Road Dairies also amended their original application in January-April 2020, to reduce the volume of water they had applied for from 6,081,499 m³/year to 3,751,225 m³/year. Consequently, additional water was made available to Papawai Partnership to increase the volume they originally applied for by 1,052,455 m³/year, and to I & P Farming to increase their original volume by 722,888 m³/year. Purunui Trust applied in April 2020 for the remaining water (544,931 m³/year).

Details of each applicant's proposal, in terms of their irrigable area, intended crop type and the amount of Tranche 2 water applied for, are provided in Section 3 below.

1.3 Collaborative Approach

A fundamental element of Tranche 2 groundwater abstraction is the RRMP requirement to augment river flows to mitigate the impact of the Tranche 2 deep groundwater abstractions on surface water bodies. The augmentation water is also abstracted from deep groundwater, and it forms part of the Tranche 2 allocation. This means that each Tranche 2 applicant must specify the maximum volume of deep groundwater they propose to abstract for irrigation and the maximum volume they propose to abstract for augmentation purposes.

The augmentation concept included in PC6 was novel, not only for Hawke's Bay, but for New Zealand generally. It was not instigated by the Hawke's Bay Regional Council (HBRC) but was introduced by the Board of Inquiry into the Tukituki Catchment Proposal in their June 2014 decision. Not surprisingly, the augmentation concept was not approached consistently by all of the applicants.

Each Tranche 2 groundwater abstraction will have an effect on groundwater levels and river flows across a wide area. The effects of all eight applicants then combine to produce Ruataniwha Basin-wide effects. Consequently, at a meeting with HBRC officials in March 2018, it was agreed that the applicants would work together in a collaborative manner to assess (or reassess) the cumulative effects of their proposed Tranche 2 abstractions.

Aqualinc Research Ltd was engaged by the collaborative group to model the effects of the Tranche 2 deep groundwater abstractions and develop an augmentation regime that achieved the outcome specified by RRMP Policy POL TT8. Bay Geological Ltd (BGSL) was engaged to assess the effects of the Tranche 2 abstractions on existing shallow bores, and on wetlands and surface water bodies; and Lattey Group completed an assessment on the potential for well interference. The Aqualinc report forms Appendix B to this Report, the BGSL reports form Appendix C and Appendix E to this report, and the Lattey Group report forms Appendix D.

This AEE is based on the outcomes of the collaborative modelling and effects assessments. It supersedes and replaces the individual assessments of environmental effects (AEEs) that were originally lodged by each of the eight applicants.

Importantly, when the original Tranche 2 applications were lodged, it was unclear how the PC6 augmentation provisions should be interpreted and implemented. For example, it was unclear whether, or not, the effects of the deep groundwater augmentation abstractions on surface flows would themselves need to be mitigated by further augmentation, and so on. Some applicants (including TAFT, Springhill Dairies, Plantation Road Dairies and Buchanan No. 2 Trust) assumed that would be the case, while others did not.

Consequently, this report focuses on the total amount of water originally sought by each applicant, as it is understood that this cannot be increased in the absence of fresh consent applications. As noted earlier, while this AEE replaces the original AEEs lodged, the original applications remain in place and those applications define the scope of the applications, with the exception of some proposed amendments, as outlined in this report.

Irrigation demand (assuming pasture) has been calculated for the irrigable areas originally applied for using Aqualinc's in-house soil-water balance model, IrriCalc. From these calculations, the 90 percentile (or 9 in 10 years) irrigation demand was calculated. Once the irrigation demand was calculated, the irrigation volume

was deducted from each applicant's total volume applied for to yield a volume of water available for augmentation. Various scenarios were then modelled to assess the effects of each applicant's take (singularly and cumulatively with all other applicants) and the optimum scale and timing for the commencement and duration of the augmentation takes and discharges. As discussed below, the purpose of this analysis was to ensure that, as required by Policy TT8(ca) and Rule TT4(b), any augmentation regime proposed will be commensurate with the nature of the effects generated.

The assessment undertaken by Aqualinc assumes that all Tranche 2 water is used for irrigation, except for Tuki Tuki Awa. If only some of Tuki Tuki Awa's Tranche 2 water is used to fill in gaps when their Tranche 1 water is unavailable, then the overall effects will be less than assessed: less Tranche 2 water will be taken for irrigation and augmentation will remain the same.

1.4 Maintaining the PC6 Minimum Flows

PC6 was briefly discussed in Section 1.1 above. In addition to POL TT8(ca), PC6 also introduced Rule TT4(b) to the RRMP. That rule applies to groundwater takes located within Ruataniwha Basin Groundwater Allocation Zones 1 to 3 and contains three conditions, standards and terms:

- a. The take, in addition to all existing consented takes but excluding takes consented in association with in-stream dams, does not result in any exceedance of the allocation limits in Table 5.9.4, 5.9.5 or 5.9.6 (whichever is applicable); and
- **b.** The take complies with the relevant minimum flow regime.
- c. No new groundwater takes from Groundwater Allocation Zones 2 and 3 utilising Tranche 2 groundwater may be exercised under this rule unless and until augmentation flows are discharged that are commensurate to the scale of effect of the proposed take, during the same irrigation season as the Tranche 2 groundwater takes are exercised, to each of the Waipawa River and the Upper Tukituki River or one or more of their respective tributaries at a rate of up to 715 l/s to each river catchment at the highest practicable elevation as required to maintain the relevant downstream minimum flows specified in Table 5.9.3

As mentioned above, the implication of clause (a) of Rule TT4 is that one of the Tranche 2 applicants, Purunui Trust, cannot be granted all of the water they applied for because doing so would breach the Table 5.9.5 Tranche 2 groundwater allocation of 15,000,000 m³/year.

Along with POL TT8(ca), the requirements of clauses (b) and (c) of Rule TT4 have underpinned the modelling undertaken by Aqualinc and their optimisation of the augmentation regime. That is discussed in detail in Section 5 of this report. However, by way of overview, it is proposed that the Tranche 2 applicants commence augmentation discharges to their respective adjoining surface waterbodies before the river flows at the flow management sites listed in RRMP Table 5.9.3 reach their respective Minimum Flows – essentially setting higher surface water trigger flows for the commencement of Tranche 2 augmentation. The augmentation discharges will occur whenever those higher trigger flows are reached, and regardless of whether or not irrigation is operating (except for Tuki Tuki Awa who propose to take Tranche 2 groundwater only when their existing surface water take is restricted).

Importantly, the augmentation discharges over-compensate for the effects of the Tranche 2 deep groundwater abstractions on surface water on a catchment-wide basis. In other words, notwithstanding the effects of the Tranche 2 deep groundwater abstractions, the associated Tranche 2 augmentation discharges result in higher river flows (during low flow periods) at the Waipawa River at SH2 and Tukituki River at Tapairu Road Minimum Flow sites located on the eastern (lower) side of the Ruataniwha Basin than would be the case in the absence of the Tranche 2 deep groundwater abstractions. Further detail is set out

in Section 5 below. The requirements of Policy TT8(ca) and Rule TT4(b) will therefore be met by the proposed take augmentation regime.

2 Plan Change 6

The following provides a background to PC6 and the decision of the Board of Inquiry (BOI) to establish the "Tranche 2" groundwater allocation, in addition to the "Tranche 1" fully allocation groundwater volume of approximately 28.5million $m^3/year.^2$

In the decision of the BOI on PC6 (dated 18 June 2014) the BOI considered the sustainable volume of groundwater that could be abstracted from the Ruataniwha aquifer.

They referred to the evidence of groundwater modellers (Dr Baalousha and Mr Weir) and noted that neither of them suggested there was any physical difficulty with extracting more than 30 million m³ from the aquifer (at a catchment-size scale). Rather, the limitation on abstraction arose from the surface flow effects of lowering groundwater levels (particularly the reduced flow of some spring flows and the effects of reduced stream flows on habitat). The BOI therefore considered it necessary to have regard to two particular effects: firstly, the effect of abstracting groundwater on surface water flows, and secondly, the effect of lowering groundwater levels.

Adopting a conservative approach, the BOI considered that extraction of 45million m³/year from the Ruataniwha aquifer would reduce flows in the Waipawa and Upper Tukituki Rivers (combined) by 1430 L/s. Of that 780 L/s would be attributed to existing takes. The evidence presented at the hearing for PC6 suggested that such a deduction could have an adverse effect on in-stream habitat. The BOI therefore considered that unless there was some way of mitigating the effect on surface flows it would be very difficult to justify any increase in groundwater takes above the limit of 28.5million m³/year, being the volume proposed by HBRC.

The BOI referred to the evidence of Mr McIndoe, who was confident that the groundwater system could easily sustain a 45million m³/year take. Mr Weir reached a similar conclusion, that there would be a relatively small change in the groundwater level, with the greatest changes occurring near pumping wells and the changes would vary with depth. The BOI accepted that a small reduction in groundwater levels did not present a problem. However, they considered that any reduction in minimum flows resulting from the extraction of groundwater presented an adverse effect that would need to be overcome.

On the basis of the evidence presented to it, the BOI came to the following conclusions:

- If possible, it was desirable for the limit on extraction from the aquifer to be lifted to 43.5million m³/year (plus 1.5million m³/year for "permitted" uses), as this was a sustainable yield and it would support the economic wellbeing of the rural community;
- But that could only be achieved if adverse effects on surface flows were adequately avoided, remedied or mitigated;
- Any reduction in the minimum flows that were set by PC6 would need to be mitigated to the extent and for the period that the minimum flow regime would be compromised; and
- Deep groundwater within the Ruataniwha aquifer represented the only source from which the necessary mitigation could be achieved.

The BOI concluded that augmentation of low river flows from deep groundwater (or from any storage source that might become available) was a realistic possibility. If extraction were lifted to 43.5million m 3 /year the

² "Final Report and Decisions of the Board of Inquiry into the Tukituki Catchment Proposal", Volume 1 of 3: Report and Decisions, dated 18 June 2014, paragraphs 559-585, pages 173-179.

expert evidence indicated that around 12-13,000ha could be irrigated which would represent an increase of 6-7,000ha beyond the current situation.

The BOI recognised that some groundwater users might be remote from access to rivers or streams requiring augmentation, but they did not believe that was an insurmountable hurdle. One way the BOI thought it might be overcome was by a group approach on the part of the irrigators. The BOI stated:

"[575] Obviously the issue of augmentation will require further consideration and it is possible that a further plan change will be required. We contemplate that this augmentation regime can be in place by 1 July 2018 when the new minimum flow/allocation regime will come into force (except for the earlier review of existing consents). At this stage it is only necessary for the Board to provide a framework within which the allocation limit for Zones 2 and 3 can be lifted to 43.5 million $\rm m^3$ /year provided the necessary augmentation regime is in place. Details can be worked out later. The Board anticipates that allocation of this additional 15 million $\rm m^3$ /year will be managed through the consent process."

The BOI considered whether the augmentation regime should apply on a pro rata basis across all consents for the extraction of groundwater from the Ruataniwha aquifer, or whether it should only apply to consents for the additional 15million m³/year. The BOI recognised that allocation of a further 15million m³/year to new consent holders, or by way of increased abstraction for existing consent holders, would provide them with an additional and new allocation of the Ruataniwha resource for commercial purposes. As such, the BOI did not consider it unreasonable or unfair to require them to carry responsibility for mitigating the adverse effects on the environment arising directly and indirectly from this additional abstraction. The BOI therefore considered that they should be responsible for the 'whole' of the augmentation requirement.

Given the above, the BOI identified that it would be necessary for the allocation regime proposed under PC6 to be modified so that between 28.5million m³/year and 43.5million m³/year (Tranche 2) allocation would be possible if an augmentation regime was provided for. The allocation would be a Discretionary Activity (provided the augmentation requirement was in place).

This is as much direction as the BOI provided in its decision, so it is not very clear how the Tranche 2 allocation is intended to work in practice.

Given this uncertainty, a meeting was held with HBRC staff and the Applicant's Planners (Rob van Voorthuysen (van Voorthuysen Environmental Ltd) and Janeen Kydd-Smith (Sage Planning HB Limited)) and technical expert, Susan Rabbitte (Lattey Group), on 10 April 2018 at the HBRC offices in Napier. The purpose of the meeting was to reach agreement between those present on answers to questions relating to the implementation of the Regional Plan policies for taking Tranche 2 groundwater. This was intended to assist preparation of this AEE and consideration of the subject applications. Notes of that meeting are contained in **Appendix A** of this report. This AEE has been prepared on the basis of the approach agreed at this meeting.

3 Tranche 2 Applicants

The following sub-sections introduce the eight Applicants and detail each property, irrigation demands, and the proposed groundwater volumes applied for.

3.1 Te Awahohonu Forest Trust (TAFT)

Application Site: Gwavas Station - 5740 State Highway 50 and 97 Matheson Road, Tikokino (Figure 1).

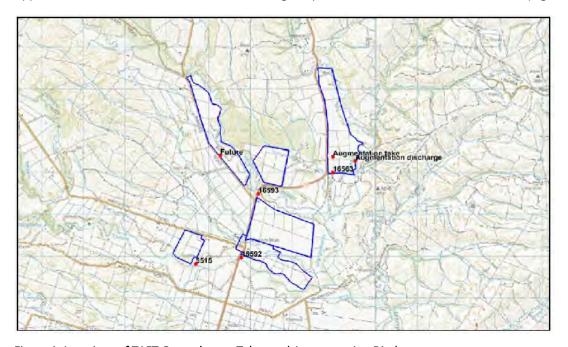


Figure 1: Locations of TAFT Groundwater Takes and Augmentation Discharge

TAFT applied in November 2014 for 4,914,920 m³/year of Tranche 2 groundwater to irrigate 540 ha of pasture on the property located within the Mangaonuku Sub Catchment (T2³). It was proposed that 2,890,000 m³/year of the Tranche 2 groundwater take be used for irrigation and the remaining 2,024,920m³/year be used for augmentation.

TAFT does not hold any current groundwater resource consent for the property.

Groundwater for irrigation is proposed to be taken from five bores, comprising four existing bores (Well Nos. 16563, 16592, 16593 and 5515) and one future new deep bore. The bores are generally in the vicinity of the Mangamauku and Mangamate Streams. The combined rate of take of the Tranche 2 groundwater is proposed to not exceed 420 l/s and no one point of take is proposed to exceed a rate of 100 l/s.

Existing Well No. 16563 has a depth of 162.2 m below ground level (bgl) and is screened below 145 m depth, across blue gravel /blue clay layer. Well No. 16592 is an exploratory bore with a depth of 220.8 m bgl and is screened below 193.16 m across a coarse pink-grey ash and gravel layer. Well No. 16593 is also an exploratory bore with a depth of 222.3 m bgl and is screened below 138.30 m across a grey ash/pumice layer. Well No. 5515 has a depth of 66.0 m bgl and is screened below 54 m across a gravel layer.

The 90-percentile water demand for pasture is calculated to be approximately 580 mm/year (using 'IrriCalc'), which over an irrigated area of 490 ha equates to an annual volume of 2,841,220 m³/year m³/year.

³ As identified on the map in Schedule XIVc Tukituki River Sub Catchments to the RRMP.

However, a larger area of 850 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture could be irrigated with 2,841,220 m³/year m³/year of Tranche 2 groundwater.

The augmentation volume will be abstracted from one or more new future deep bores and discharged directly to the Mangaonuku Stream at a rate of 189 l/s (daily average) (Figure 1).

A consent duration of 20 years was sought by TAFT.

3.2 Papawai Partnership

Application Site: 1041 State Highway 50, Ongaonga (Figure 2).

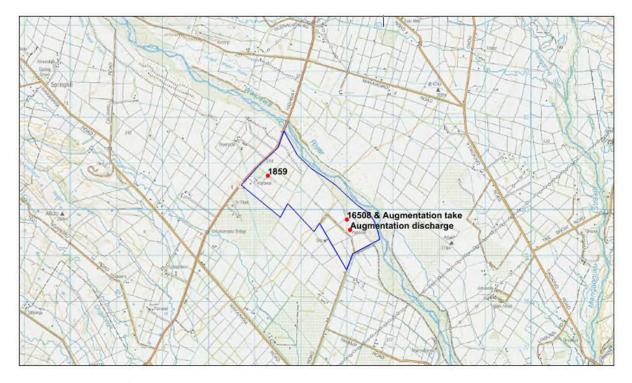


Figure 2: Locations of Papawai Partnership Groundwater Takes and Augmentation Discharge

Papawai holds current groundwater resource consent (WP 140555T) from Well Nos. 1859 and 16508 at a combined rate of 130 l/s. The consent grants a maximum volume of 608,212 m³/year (1 July to 30 June in consecutive calendar years) and 120,960 m³ in any 28-day period. The consented annual volume is not adequate to provide sufficient irrigation application rates.

Papawai Partnership have therefore made two applications for Tranche 2 groundwater. The original was submitted in March 2017, to take up to 423,062 m³/year of groundwater, and a new application was submitted in August 2019 to take up to 1,052,455 m3/year of Tranche 2 groundwater. These combine to a total of 1,475,517 m³/year and comprises 1,010,817 m³/year for irrigation and 464,700 m³/year for augmentation. The applicant seeks to increase production and more efficiently farm non-irrigated areas of the property located within the Waipawa Sub Catchment (T1⁴). The irrigation component of the proposed Tranche 2 take will supplement the existing consented take of 608,212 m³/year to provide adequate irrigation of 181 ha of pasture, or a larger area of 320 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture.

⁴ As identified on the map in Schedule XIVc Tukituki River Sub Catchments to the RRMP.

The requested Tranche 2 groundwater irrigation volume will be abstracted from Well Nos. 1859 and 16508 which are currently consented for 130 l/s; therefore, a portion of the effects of the Tranche 2 take will be regarded as part of the 'existing environment'.

The augmentation volume of 464,700 m³/year will be abstracted from existing Well No. 16508 (or from other bore(s) on the property) and discharged into a shallow, disused well located approximately 300 m to the south (Figure 2). The discharge well is thought to be directly connected to the nearby Waipawa River; however, it will be tested prior to augmentation.

A consent duration of 20 years was sought by Papawai.

3.3 Tuki Tuki Awa

Application Site: 406 Tukituki Road, Takapau (Figure 3)

Tuki Tuki Awa has applied for Tranche 2 groundwater to provide security of water supply to more efficiently farm irrigated areas of the property within the Upper Tukituki Sub Catchment (T4) and the Tukipo Sub Catchment (T5)⁵.

The Applicant owns and operates a dairy farm on Tukituki Road near Takapau, comprising approximately 136 ha of crops and pasture. The farm is currently irrigated using Surface Water Consent No. WP120320T which grants a volume of 174,180 m³ in any 28-day period at a rate no greater than 78 l/s; and 560 m³ in any 28-day period for dairy shed use. However, the consent is subject to low flow restrictions when the Tukituki River is at or below the relevant low flow level at specific monitoring sites.

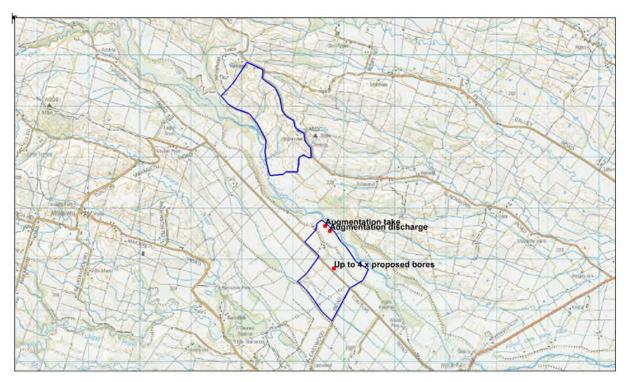


Figure 3: Locations of Tuki Tuki Awa Groundwater Takes and Augmentation Discharge

The Application for Tranche 2 water will not increase the volume of water applied to the property but will utilise the groundwater as a top-up during periods when the Tukituki River is on low-flow restrictions.

⁵ As identified on the map in Schedule XIVc Tukituki River Sub Catchments to the RRMP.

The Applicant proposes the following:

- i) To take groundwater from four proposed 300 mm diam. bores screened greater than 50 m below depth;
- ii) To abstract up to 103 l/s of groundwater for irrigation of crops and pasture along with the required stream augmentation;
- iii) The annual volume of groundwater taken between 1st July and the following 30th June shall not exceed 952,400 m³; and
- iv) A consent duration of 20 years is sought.

In February 2015, the Applicant lodged an application for groundwater made available through PC6 of 952,400 m³/year comprising 882,800 m³/year irrigation and 129,600 m³/year augmentation.

To determine required irrigation across the property, IrriCalc was used to model water use on the Applicant's property for the period 1972 to 2014 and derived from the daily irrigation water use data the 90-percentile (1 in 10-year drought) annual volume. The irrigation seasonal depth was calculated as approximately 450 m³/ha for the property which equates to an annual volume (assuming pasture) of 678,100 m³/year. This is less than the volume of water sought by Tuki Tuki Awa for irrigation purposes (which was 822,800 m³/year) but would be adequate to irrigate approximately 136 ha of pasture, or crops and/or horticulture, or a mixture of pasture, crops and horticulture.

The 90-percentile augmentation volume of 129,600 m³/year is to be abstracted from one or more of the proposed four wells to be drilled on the property adjacent to the Tukituki River, into which the replenishment discharge would be pumped (Figure 3). During wet years, the farm is not likely to irrigate over the whole 150-day season, and therefore the full Tranche 2 allocation may not be utilised.

A review undertaken for the application revealed that there are no bores within 2 km of the Applicant's property; and due to the proposed wells being screened greater than 50 m depth, the take will not be considered directly stream depleting.

The resource consent application sought a consent duration of 20 years.

3.4 Plantation Road Dairies (PRD)

Application Site: 1404 Ongaonga Road, and Wakarara Road, Ongaonga (Figure 4).

PRD originally applied to take $6,000,000 \text{ m}^3/\text{year}$ of Tranche 2 groundwater. After their application was lodged, they changed their proposal and reduced the volume of groundwater sought to $3,751,225 \text{ m}^3/\text{year}$ from deep bores on the property (Figure 4), located in the lower basin between the Waipawa and Tukituki Rivers within the Kahahakuri Sub Catchment (T3 6).

The 90-percentile water demand for pasture is calculated to be approximately 600 mm/year (using 'IrriCalc'), which over an irrigated area of 403 ha equates to a volume of approximately 2,418,225 m³/year. This is less than the total volume of irrigation water initially sought by PRD for irrigation purposes (2,775,914 m³/year). A larger area of up to 459 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture, may be irrigated from 2,418,225 m³/year. The balance of the Tranche 2 groundwater taken will be used for augmentation (i.e., 1,333,000 m³/year).

13 | Page

⁶ As identified on the map in Schedule XIVc Tukituki River Sub Catchments to the RRMP.

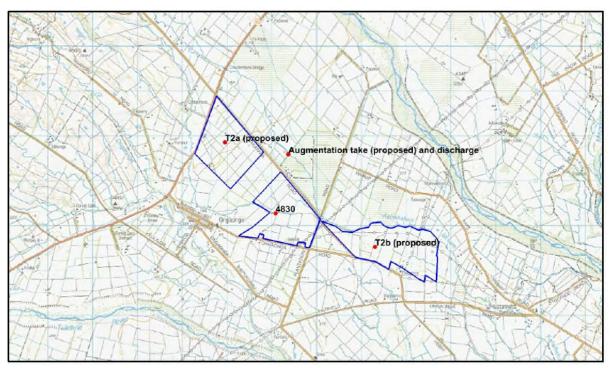


Figure 4: Locations of Plantation Road Dairies Groundwater Takes and Augmentation Discharge

It is proposed to take Tranche 2 groundwater for irrigation from an existing bore (Well No. 4830) which has a depth of 137 m bgl, and from two future deep bores (shown as T2a and T2b in Figure 4). Additional augmentation water will be taken from one or more new bores located on adjacent land also owned by PRD and will be discharged directly into the Kahahakuri Stream, immediately beside this location, at a rate of 103 l/s (daily average).

No specific consent duration was requested by the applicant in their resource consent application, but they seek a consent duration of 20 years to match the duration being sought by the other applicants.

3.5 Springhill Dairies (formerly Ingleton Farms)

Application Site: 665 State Highway 50 and 36 Butler Road, Tikokino (Figure 5).

Springhill Dairies applied in January 2014 to take up to 1,005,213 m³/year of Tranche 2 groundwater from deep bores located around their property to supplement existing consented takes (with a combined volume of approximately 4,029,077 m³/year) to provide adequate irrigation of 702 ha of pasture and crops located within the Mangaonuku Sub Catchment (T2) and partly within the Waipawa Sub Catchment (T1)⁷.

The 90-percentile annual water demand for pasture (assumed) is calculated to be approximately 480 mm/year (using the web-based Aqualic/Irrigation New Zealand 'IrriCalc' water allocation calculator), which over an irrigated area of 702 ha equates to 3.4 million m³/year. The irrigation demand can be met by utilising a combination of existing consented takes and a portion of the Tranche 2 volume applied for, while leaving a reasonable volume of Tranche 2 groundwater available for augmentation purposes. An irrigation volume of Tranche 2 groundwater of 588,313 m³/year) would be adequate to irrigate approximately 123 ha of pasture, or a larger area of 188 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture. The balance of the Tranche 2 groundwater taken will be used for augmentation (i.e., 416,900 m³/year).

⁷ Ibid.

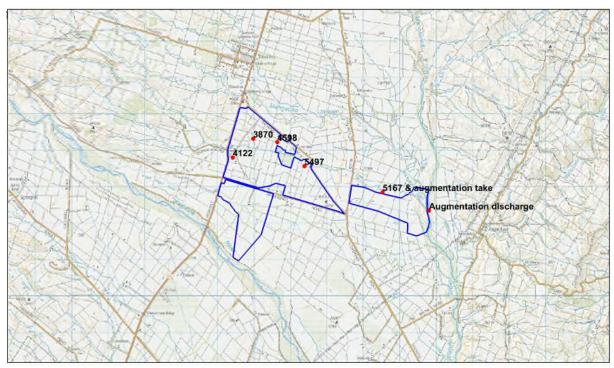


Figure 5: Locations of Springhill Dairies Groundwater Takes and Augmentation Discharge

It is proposed to take Tranche 2 groundwater for irrigation from up to five existing bores on the property (i.e. Well Nos. 5167, 4593, 1518, 3870 and 5497)⁸. Well No. 5167 will also be used for the augmentation groundwater take. The bores range in depth from 56.1 m bgl (Well No. 5497) to 124.6 m bgl (Well No. 5167). It is proposed to augment into the Mangaonuku Stream at a rate of 38 l/s (daily average). The location of the wells from which Tranche 2 groundwater will be taken, and the approximate locations of the augmentation take bore(s) and the discharge site, are shown in Figure 5.

The rates of Tranche 2 groundwater to be taken from the existing wells will not exceed (and will be less than) the Tranche 1 takes from the bores.

A consent duration of 20 years was sought by Springhill Dairies.

3.6 I & P Farming (formerly Abernethy Partnership)

Application Site: 337 Ongaonga-Waipukurau Road, Waipukurau (Figure 6).

I & P Farming Ltd (I&P) applied for Tranche 2 groundwater to increase production on their property located within the Kahahakuri Sub Catchment (T3)⁹.

In March 2017, I & P lodged an initial application for Tranche 2 groundwater totalling 477,122 m³/year. However, the volume applied for was not sufficient to irrigate the 166 ha of pasture and crops, and in August 2019 a second Application was lodged for 722,888 m³/year with groundwater planned to be abstracted from a proposed new well (or wells). The volumes applied for total 1,200,010 m³/year.

⁸ Consented groundwater will also be taken from existing Well No 4122 and used in combination with the Tranche 2 groundwater for irrigation.

⁹ As identified on the map in Schedule XIVc Tukituki River Sub Catchments to the RRMP.

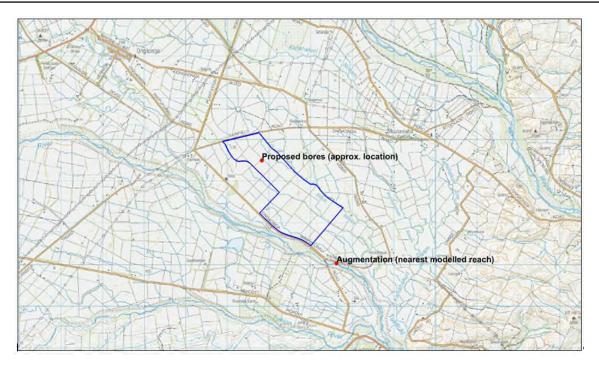


Figure 6: Locations of I & P Farming Groundwater Takes and Augmentation Discharge

There are currently no consented water takes on the property, and irrigation is required to increase crop production and pasture growth.

A proposed irrigation volume of 916,010 m³/year (based on IrriCalc's 90% water use) equates to a flow rate of 83 l/s over a 150-day irrigation season and is based on irrigation of 166 ha of pasture (assumed). A larger area of up to 310 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture, may be irrigated from 916,010 m³/year. The balance of the Tranche 2 groundwater taken will be used for augmentation (i.e., 284,000 m³/year).

The rate of the augmentation portion of 284,000 m³/year is 22 l/s (daily average) which will be discharged into an unnamed stream on their property that eventually flows into the lower reaches of the Tukituki River immediately south of the farm.

A consent duration of 20 years was requested by the applicant for the 2017 and 2019 resource consent applications.

3.7 Buchanan Trust No.2

Application Site: 19 Ngaruru Road, Ongaonga (Figure 7).

Buchanan Trust No.2 originally applied to take up to 1,631,018 m³/year of Tranche 2 groundwater from an existing well (Well No. 16408, with a depth of 119.8 m bgl) and three proposed new deep bores (shown as proposed T2a, T2b and T2c in Figure 7) located around their property, within the Kahahakuri Sub Catchment, to irrigate 242.6 ha of pasture. At the time of their original application (April 2017), only 1,145,794 m³/year was available from the total Tranche 2 allocatable volume, equating to approximately 70% of the water they had applied for. The Buchanan Trust subsequently advised Hawke's Bay Regional Council in November 2019, that the lesser volume available to them for both irrigation and augmentation was 1,145,794m³/yr, which was less than they had applied for, but was now considered by them to be sufficient for their future needs¹0.

 $^{^{10}}$ Email from Susan Rabbitte (Lattey Group Ltd) to Paul Barrett (HBRC Team Leader Consents), dated 5 November 2019.

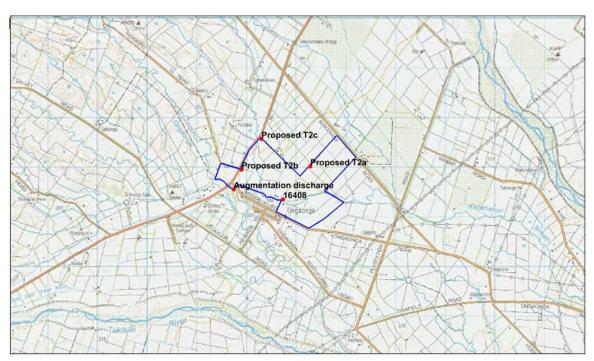


Figure 7: Locations of Buchanan Trust No.2 Groundwater Takes and Augmentation Discharge

It is proposed that $786,594 \text{ m}^3/\text{year}$ of the Tranche 2 groundwater taken will be used for irrigation, and the balance ($359,200 \text{ m}^3/\text{year}$) will be used for augmentation. Water for augmentation may be taken from one or more of the proposed bores on the site.

The 90-percentile annual water demand for pasture is calculated to be approximately 600 mm/year (using IrriCalc), which results in the 131 ha of pasture (assumed) being fully irrigable from the 786,594 m³/year volume. However, a larger area of up to 230 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture could be irrigated from the 786,594 m³/year volume. The balance of the Tranche 2 groundwater taken will be used for augmentation (i.e., 359,200 m³/year).

The rate of the augmentation proposed is 51 l/s (daily average) which will be discharged into the nearby Ongaonga Stream, which converges with the Tukituki River approximately 4 km south of the property.

No specific consent duration was requested by Buchanan Trust in its resource consent application, but they seek a consent duration of 20 years to match the duration being sought by the other applicants.

3.8 Purunui Trust

Application Site: 385 and 375 Swamp Road, Ongaonga (Figure 8).

Purunui Trust applied to take up to 1,575,000 m³/year of Tranche 2 groundwater in April 2020 to irrigate 175 ha of pasture/process crops on their property located within the Kahahakuri Sub Catchment (T3)¹¹.

 $^{^{11}}$ As identified on the map in Schedule XIVc Tukituki River Sub Catchments to the RRMP.

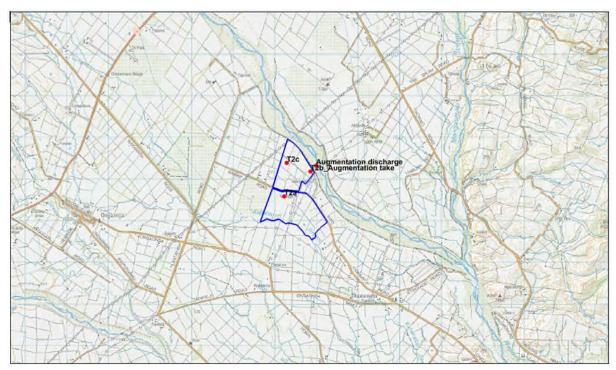


Figure 8: Locations of Purunui Groundwater Takes and Augmentation Discharge

It is proposed to take the Tranche 2 groundwater from three future new deep bores located around their property (shown as T2a, T2b and T2c in Figure 8) comprised of:

- 1,050,000 m³/year (at a volume not exceeding 252,000 m³ within any 28-day period) for irrigation, capped at meeting full water demand of up to a one-in-ten-year drought; and
- An additional 525,000 m³/year (at a volume not exceeding 126,000 m³ within any 28-day period) to provide river augmentation to mitigate the effects of the irrigation take during dry periods.

As Purunui Trust is last in the queue of Tranche 2 groundwater applications, the full volume applied for by the Trust is not available due to the 15 million m³/year cap on the combined Tranche 2 takes (RRMP Table 5.9.5). Instead, a total volume of 554,921 m³/year is available, which equates to approximately 35% of the water applied for. Assuming both the irrigation and augmentation volume is scaled equally, then the Tranche 2 volume remaining for Purunui Trust would be made up of 370,321 m³/year for irrigation and a further 184,600 m³/year for augmentation. This volume is adequate to irrigate approximately 62 ha of pasture (assumed), or a larger area of up to 93 ha of less water-intensive crops and/or horticulture, or a mixture of pasture, crops and horticulture.

The 90-percentile annual water demand for pasture is calculated to be approximately 600 mm/year (using IrriCalc), which over an area of 62 ha, equates to an annual volume of $370,321 \, \text{m}^3$ /year volume for irrigation. The balance of the Tranche 2 groundwater taken will be used for augmentation (i.e. $184,600 \, \text{m}^3$ /year).

The rate of the augmentation proposed is 14 l/s (daily average) which will be discharged into an existing, unused large diameter shallow well located near the Waipawa River. The discharge well is thought to be directly connected to the nearby Waipawa River; however, it will be tested prior to augmentation. The rates of Tranche 2 groundwater taken from the wells for irrigation will not exceed 14 l/s.

No specific consent duration was requested by Purunui Trust in its resource consent application.

4 Consent Categories

4.1 Resource Management (National Environmental Standards for Freshwater) Regulations 2020 (NES-F)

The NES-F came into force on 3 September 2020.

The NES-F includes standards for farming activities. This includes agricultural intensification, conversion of land on farms to dairy farm land, irrigation of dairy farm land, and arable and horticultural land uses. It also includes standards for vegetation clearance, earthworks or vegetation clearance and the taking, use, damming, diversion or discharge of water outside, but within, 100 m and 10 m (the distance applying depending on the relevant standards) of 'natural wetlands' 12.

Potential effects on wetlands and minor waterbodies associated with the subject Tranche 2 applications have been assessed by BGSL¹³ (see report in Appendix C and Section 8.1.2 below). In undertaking this assessment, BGSL sought information from the Tranche 2 applicants about local wetlands, and guidance from the Hawke's Bay Regional Council, who provided information detailing wetland areas within southern and central Hawke's Bay (see the mapped wetlands in Figure 14 in Section 8.1.2 below) identified as part of the Council's State of the Environment (SOE) programme. The majority of the wetlands lie outside the study area and outside the influence of the proposed Tranche 2 groundwater takes and discharges. Only one of the SOE mapped wetlands (Mangatewai Wetland) is located within the study area, near the upper reaches of the Mangatewai River.

Two wetlands were inspected by BGSL off Swamp Road, Ongaonga, which are located centrally within the Ruataniwha Basin, south of the Waipawa River (Figures 9 and 10). A third wetland was inspected by BGSL near SH50, south of the Tukipo River (Figure 11).



Figure 9: Swamp Road wetlands, Ongaonga

¹² Defined in the National Policy Statement for Freshwater Management 2020 as: "means a wetland (as defined in the Act) that is not (a) a wetland constructed by artificial means (unless it was constructed to offset impacts on, or restore, an existing or former natural wetland); or (b) a geothermal wetland; or (c) any area of improved pasture that, at the commencement date, is dominated by (that is more than 50% of) exotic pasture species and is subject to temporary rain-derived water pooling."

¹³ Letter from Bay Geological Services Ltd to Hawke's Bay Regional Council, dated 27 May 2021 (Reference BGS201-07)



Figure 10: Swamp Road wetlands and surface water features



Figure 11: SH50 wetland (south of Tukipo River)

All wetlands were identified as 'natural' wetlands¹⁴. The BGSL assessment of effects on the wetlands (and minor streams) found that in the central and eastern areas, large wetlands were observed in the field which appeared to be fed by a possible subsurface divergent channel of the Waipawa River,

¹⁴ Refer to Table 1 of the BGSL report (page 6).

upwelling in areas of lower elevation, where changes in lithology resulted in poor confinement. The assessment concluded that the estimated changes in surface water levels associated with the Tranche 2 proposals would have a 'negligible' effect on the three wetlands.

This AEE relates to an assessment of effects on the environment for the take of Tranche 2 groundwater and the discharge of a portion of that groundwater for augmentation. It does not include an assessment of effects relating to any applications for resource consents to Hawke's Bay Regional Council that may be needed by the subject applicants in the future (e.g. to drill wells, or install infrastructure for irrigation or augmentation, if within 100 m/10 m of a natural wetland).

Given the above, it is considered that no consents are currently required under the NES-F in relation to the subject applications.

4.2 Hawke's Bay Regional Resource Management Plan (RRMP)

4.2.1 Groundwater Takes

Policy POL TT8(ca) of the Hawke's Bay Regional Resource Management Plan (RRMP) is to enable additional groundwater to be abstracted as a Discretionary Activity (Table 5.9.5) provided that river flows are augmented to maintain the relevant minimum flows specified in Table 5.9.3 commensurate to the scale and effect of the Tranche 2 groundwater take.

Table 5.9.5 of the RRMP sets out the following groundwater allocation limits, including the Tranche 2 allocation limit of 15million m³/year for Groundwater Allocation Zones 2 and 3 (Schedule XVII) collectively:

Table 5.9.5: Groundwater Allocation Limits

Groundwater Allocation Zones (Schedule XVII)	Allocation Limit (m³/year)	
Zone 1 – Otane Basin	4,134,000	
Zone 2 - Ruataniwha Basin north of the Waipawa River	Tranche 1 7,224,000	
Zone 3 - Ruataniwha Basin south of the Waipawa River	Tranche 1 21,277,000	
Zones 2 and 3 collectively	Tranche 2 15,000,000	
Rest of the catchment	No limit set32	

The subject applications are located within Groundwater Allocation Zones 2 and 3, as identified in Figure 12 below.

Policy POL TT14(e) specifies that new takes within Table 5.9.5 complying with the minimum flow regime shall be a Discretionary Activity. Policy POL TT14(fa) specifies that, except as provided for in POL TT14(a) to (f), takes (including those that do not comply with the minimum flow regime) shall be Non-Complying Activities. POL TT14(fc) states that, for takes granted under POL TT14(e) to (fa) the consent duration shall be no more than 20 years.

Rule TT4 specifies that the take and use of groundwater comprising new groundwater takes located within Groundwater Allocations Zones 1 to 3 (applied for after 4 May 2013), excluding takes associated with a Community Irrigation Scheme involving an in-stream dam or any other in-stream dam, is a Discretionary Activity, subject to compliance with the following Conditions/Standards/Terms:

- a. The take, in addition to all existing consented takes but excluding takes consented in association with in-stream dams, does not result in any exceedance of the allocation limits in Table 5.9.4, 5.9.5 or 5.9.6 (whichever is applicable); and
- b. The take complies with the relevant minimum flow regime.

c. No new groundwater takes from Groundwater Allocation Zones 2 and 3 utilising Tranche 2 groundwater may be exercised under this rule unless and until augmentation flows are discharged that are commensurate to the scale of effect of the proposed take, during the same irrigation season as the Tranche 2 groundwater takes are exercised, to each of the Waipawa River and the Upper Tukituki River or one or more of their respective tributaries at a rate of up to 715 l/s to each river catchment at the highest practicable elevation as required to maintain the relevant downstream minimum flows specified in Table 5.9.3

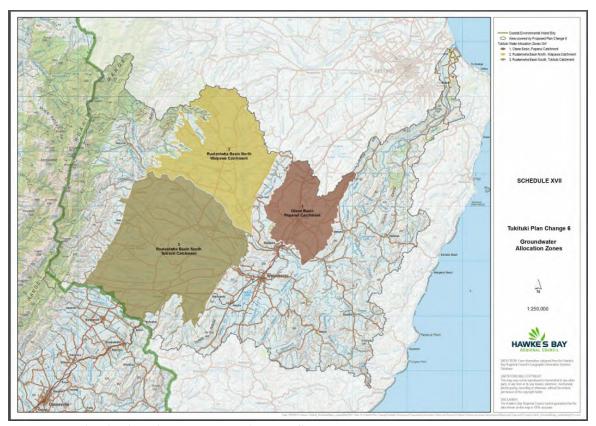


Figure 12: Tukituki Plan Change 6 Groundwater Allocation Zones

Rule TT4 as notified (as part of PC6) did not include provision for Tranche 2 groundwater takes. Rule TT4 (as referred to above) became operative on 1 October 2015. Three of the subject applications for Tranche 2 groundwater were lodged prior to 1 October 2015¹⁵. However, we consider it would be reasonable and appropriate to also assess them as Discretionary Activities, to be consistent with the status of the other five applications.

4.2.2 Augmentation Discharges

In accordance with Policy POL TT8(ca) of the RRMP, it is proposed to discharge a portion of the Tranche 2 groundwater taken for augmentation, as follows

- Te Awahohonu Forest Trust discharge to the Mangaonuku Stream (as shown in Figure 7, page 15 of the Aqualinc modelling report);
- Papawai Partnership discharge to an existing, disused, shallow groundwater bore located approximately 300 m south of Well No. 16508, and 600m west of Waipawa River (as shown in Figure 8, page 17 of the Aqualinc modelling report). It is thought the shallow bore is directly

¹⁵ Te Awahohonu Forest Trust (lodged 19 November 2014), Springhill Dairies (lodged 26 January 2015, and Tuki Tuki Awa (lodged 17 February 2015).

connected to the nearby Waipawa River, and a detailed investigation of the bore will be undertaken prior to discharging augmentation water to verify the feasibility of this option.

- Tuki Tuki Awa discharge to the Tukituki River (as shown in Figure 9, page 19 of the Aqualinc modelling report)
- Plantation Road Dairies discharge to the Kahahakuri Stream (as shown in Figure 10, page 21 of the Aqualinc modelling report).
- Springhill Dairies discharge to the Mangaonuku Stream (as shown in Figure 10, page 23 of the Aqualinc modelling report).
- I & P Farming discharge to an unnamed tributary of the Tukituki River (as shown in Figure 12, page 25 of the Aqualinc modelling report).
- Buchanan Trust No. 2 discharge to the Ongaonga Stream (as shown in Figure 13 of the Aqualinc modelling report, page 27).
- Purunui Trust discharge to the Waipawa River (as shown in Figure 14, page 29 of the Aqualinc modelling report) via an existing unused large-diameter shallow bore located approximately 200-300 m from the river. Purunui Trust have advised that they believe this shallow bore is directly connected to the nearby Waipawa River, though this will be confirmed prior to commencing augmentation.

The locations of the proposed augmentation discharges are shown in Figure 13 below.

The discharge of water into water is a Permitted Activity under Rule 31 of the RRMP, subject to compliance with the following conditions/standards/terms:

- a. The discharge shall not cause or contribute to the flooding of any property, unless written approval is obtained from the affected property owner.
- b. The discharge shall not cause any scouring or erosion of any land or any watercourse beyond the point of discharge.
- c. The discharge shall not cause the natural temperature of any receiving water to be changed by more than 3° C from normal seasonal water temperature fluctuations, after reasonable mixing.

The proposed augmentation discharges will be at a rate and volume required to maintain minimum flows during drier periods (in accordance with Table 5.9.3 of the RRMP), therefore, they will not cause or contribute to the flooding of any property. Erosion protection (e.g. establishment of rip rap) will be provided at the point of discharge where necessary, to avoid scouring or erosion of any land or watercourse beyond the point of discharge. It is considered unlikely that the discharges would cause the natural temperature of the receiving water to change by more than 3°C after reasonable mixing.

It is therefore considered that the augmentation discharges are a Permitted Activity under Rule 31 of the RRMP.

4.2.3 Discharge Structures

As set out above, most augmentation discharges will be to rivers, via pipes. A small structure (e.g. headwall and/or rip rap) may need to be constructed at the end of each pipe at the point of discharge, to fix the pipe to the land and ensure that the discharge from it is appropriately dispersed, to avoid erosion.

Rule 72 of the RRMP provides for the erection or placement of any structure in, on, under or over the bed of a river, not expressly regulated by other rules within the RRMP, and any associated disturbance of the

river bed, any associated discharge of sediment and associated damming or diversion of water as a Permitted Activity, subject to compliance with conditions.

None of the structures will occupy an area of river bed greater than 10m^2 , change the course of a river, be made of material toxic to aquatic ecosystems, prevent the passage of fish past the structure, cause or increase any risk of flooding or damage to properties during flood events (including trapping debris), require the diversion of water during construction for a period greater than 5 consecutive days (it is anticipated that no diversions will be required), cause any erosion or scouring, or interfere with fish spawning. It is therefore anticipated that all structures will comply with the conditions of Rule 72 and will therefore be a Permitted Activity.

The rivers associated with the augmentation discharges are located within the Hawke's Bay Regional Council's Upper Tukituki River Control and Drainage Scheme. This scheme protects the River plains from frequent flooding of the Upper Tukituki River and its tributaries, the Waipawa, Makaretu, Mangaonuku and Tukipo Rivers. Rule 71 of the RRMP specifies that the erection of any structure in, on or under the bed of a river, or within 6 metres of the bed, undertaken by persons other than the local authority or persons acting on behalf, within a land drainage or flood control scheme area, is a Discretionary Activity.

4.3 Summary

On the basis of the above assessment of the applications against the relevant regulations and rules, resource consents are required as follows:

• Discretionary Activity resource consents under Rule TT4 and Rule 71.

As noted above, three of the subject applications for Tranche 2 groundwater were lodged prior to 1 October 2015¹⁶. However, we consider it would be reasonable and appropriate to also assess them as Discretionary Activities, to be consistent with the status of the other five applications.

¹⁶ Te Awahohonu Forest Trust (lodged 19 November 2014), Springhill Dairies (lodged 26 January 2015, and Tuki Tuki Awa (lodged 17 February 2015).

5 Approach Taken

The Tranche 2 groundwater applicants propose to abstract groundwater and mitigate the consequential stream depletion effects via further abstraction to directly augment river flows during drier periods (when takes from the rivers are restricted). The concept of augmentation is based on using groundwater stored in the aquifer system during wetter periods to mitigate stream depletion effects during drier periods. The augmentation takes will also result in additional stream depletion effects, but this too will be delayed and spread over space and time through the storage response of the aquifer system. While there is on average more water abstracted from the basin's aquifer system (compared to current), the rationale is founded on the principle of using the groundwater system to smooth, buffer and delay stream depletion effects using groundwater storage that is later replenished naturally.

5.1 Irrigable Area and Crop Water Demand

Irrigation demand for each applicant has been calculated for the irrigable areas applied for using Aqualinc's in-house soil-water balance model, IrriCalc. Importantly, we understand that IrriCalc is endorsed by HBRC (it is specified in Policy 47 of the recently notified TANK Plan Change to the RRMP). From these calculations, the 90-percentile (or 9 in 10 years) irrigation demand (in mm/year) has been calculated, assuming that all existing and future irrigated land use is pasture, which typically has a larger seasonal water demand than other land uses. This demand is then multiplied by the irrigated area to calculate a maximum irrigation volume.

Policy 32 of the RRMP is to allocate groundwater for irrigation purposes on the basis of actual crop water requirements up to a maximum equal to that required during a one in ten-year drought. The maximum volumes of Tranche 2 groundwater proposed to be taken and used for irrigation have been calculated for each Applicant by Aqualinc on the basis of the water being used to irrigate pasture. If land uses other than pasture are irrigated (e.g. cropping and/horticulture, or a mixture of pasture, horticulture and cropping), then the seasonal water use and associated recharge will be less for the same irrigated area, or a larger area could be irrigated for the same seasonal volume. In these cases, the Aqualinc report has assessed that the modelled effects on river flows will be either less or similar (respectively) than the assessment of the modelled effects for pasture only¹⁷.

During wet periods, the properties will not irrigate throughout the whole season, and therefore the full Tranche 2 allocation may not be utilised.

To provide greater flexibility for applicants to use Tranche 2 groundwater for the duration of the consents, it is requested that the Applicants be able to utilise the maximum volume of Tranche 2 groundwater calculated to irrigate a larger area of their properties for crops and/or horticulture, or for a mixture of pasture, crops and horticulture, than the area modelled to irrigate pasture in the Aqualinc report.

The proposed revised areas of the Applicants' properties to be irrigated are set out in the Revised Overall Tranche 2 Proposal in Table 1, in Section 6.2 of this report.

5.2 Augmentation Take and Discharge Locations

Each applicant's augmentation discharge location is shown on Figure 13 below. Additional detail is provided in Sections 3.3 to 3.10 of the Aqualinc report.

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¹⁷ Refer to Section 3.2, page 13, of the Aqualinc report.

It is possible that some applicants may take water for augmentation from one or more bores on their property. If the same volume of water was abstracted for a different number of bores than assumed in the Aqualinc report, it would make very little difference to the catchment-scale effects of the takes assessed by the modelling.

It is also recognised that some applicants may in the future seek additional consents from the Council, as may be necessary, to utilise alternative sources of water for augmentation, such as from the construction of water storage dams on their properties. Objective OBJ TT5 of the RRMP is (subject to Objectives TT1, TT2 and TT4) to enable the development of on-farm storage and Community Irrigation Schemes that improve and maximise the efficient allocation and efficient use of water. Therefore, there is support within the RRMP for such alternatives.

5.3 Consent Durations

As set out in Sections 3.1-3.8 of this report, all but three applications requested a consent duration of 20 years. No consent duration was specified in the applications from Plantation Road Dairies, Buchanan Trust and Purunui Trust.

Policy TT14(fc) specifies that, for takes granted under Policy TT14(e) to (fa), including new takes within the Table 5.9.5 Allocation Limits complying with the minimum flow regime, the consent duration shall be no more than 20 years.

For consistency, and given the collaborative approach being applied, it is requested that a 20-year consent duration be applied to <u>each</u> of the eight applications.

5.4 Protocols for the Takes and Augmentation

To aid practical management, operation and compliance of the Tranche 2 takes, several protocols for the takes and augmentation are proposed which relate to:

- Staged development and transitional implementation
- Automated monitoring of river flows
- Defining a 'Water Year' and associated start date of irrigation and augmentation.

5.4.1 Staged Development and Transitional Implementation

Some of the applicants have existing bores from which they will be able to irrigate as soon as consents are granted. Other applicants have partial development of bores, and the remainder currently have no infrastructure and will wait for consents to be granted prior to commencing any significant investment in infrastructure for Tranche 2 water. Therefore, the full uptake of Tranche 2 water will not be instantaneous upon granting of consents, but will be transitional, progressively developing ('ramping up') over several years.

To practically manage the effects of takes throughout this transition period, each consent holder will nominate (to HBRC) a maximum seasonal volume of Tranche 2 water they expect to need that season, proportional to the scale of their property development. This volume will become the maximum allocation volume for that season only and the augmentation discharge rate will be pro-rated on this same basis. This volume will then be reassessed in subsequent years. For example, a property with a 1-in-10-year maximum seasonal volume of 1 million m³/year proposes to augment the adjacent river at 20 L/s when low-flow triggers are reached. If by the first irrigation season the property is 40% developed, then the maximum irrigation volume for that year would be 400,000 m³ (i.e., 40% of 1M m³) and the augmentation rate would be 8 L/s (i.e., 40% of 20L/s). If by the following year development

expanded to 60%, then the maximum irrigation volume for that year would be $600,000~\text{m}^3$ and the augmentation rate would be 12~L/s. The nomination of the maximum seasonal volume of Tranche 2 water would continue on this basis, up to 100% development.

This transitional implementation proposed is based on the principle that, while the effects of pumping propagate over a relatively large distance, the effects are largely seen in the vicinity of the abstraction location. Consequently, effects and subsequent augmentation to mitigate these effects (during low flows) are proportional to the scale of the take.

If a Tranche 2 consent holder is unsure of the level of development expected for an upcoming season, they will not want to under predict their water needs. Hence, they would be more likely to nominate a higher percentage than a lower one. The nominated percentage of development then dictates (and locks in) the rate of augmentation for that year, regardless of whether the irrigation volume is used or not. It is therefore more likely that the augmentation rate will be over-compensated in these transitional years, which will provide benefits to the rivers greater than modelled.

5.4.2 Automated Monitoring of River Flows

It is proposed that the Applicants' Tranche 2 augmentation discharges will be controlled daily based only on the existing three monitoring sites currently automatically monitored by HBRC (i.e., Waipawa at SH2, Tukituki at Tapairu Road, and Tukipo at SH50). If automatic flow recorders are installed in the future at the other two low-flow trigger sites modelled (i.e., Tukipo at Ashcott Road, and Mangaonuku u/s Waipawa), then these can be added to the daily control regime.

The Aqualinc modelling report¹⁸ notes that this triggering regime (ie only 3 flow recorders) is expected to make little difference in the very dry years when augmentation is needed the most (all rivers experience low flows). There may be some 'unders and overs' in the wetter years where the three continuously monitored sites do not fully represent the other two sites. In these cases, the augmentation may not be triggered when these smaller streams are below the low flow trigger. However, it is expected that the targeted over-compensation at other times will provide a buffer that will partially mitigate effects on these streams at these times.

5.4.3 'Water Year' Definition and Associated Start Dates of Irrigation and Augmentation

The Tranche 2 applicants will have two volumetric limits: an irrigation volume; and an augmentation volume, both defined by a 9-in10 year season. If the water year was to commence on 1 July (as is currently defined by HBRC), it is possible that augmentation to rivers may be needed through winter at the start of the water year, when there is no irrigation pumping. While it is acknowledged that the effects of irrigation continue after pumping stops (as it takes time for the aquifer system to recover), there is a small possibility that augmentation water will be used in the cooler, wetter winter months (if minimum flows are triggered) resulting in insufficient water later in the warmer, drier parts of the season when augmentation is needed most (i.e., late summer/autumn).

For the purposes of the subject applications, it is therefore proposed that the water year be defined as commencing at the start of the irrigation season, nominally 1 October. Then, given that effects from the deep pumping take time to propagate to the surface, it is proposed that the augmentation year starts 1 month after this (i.e., 1 November). The Aqualinc modelling report notes that this is expected to have the following consequences¹⁹:

¹⁸ Section 3.12.2, page 47.

¹⁹ Section 3.12.3, pages 47-58.

- Augmentation water is 'saved' for the driest times of the year when it is needed most, usually well beyond the start of the irrigation season.
- Delaying the start of augmentation will mean that, in most years, the augmentation volume will not be fully used before winter. This unused water can then be discharged throughout winter when low flows are triggered.
- Continuing to augment during winter results in the equivalent of full-year augmentation for most years (9 in 10) but provides the added assurance that there will be augmentation water available in the driest parts of the driest seasons when it is needed most (rather than potentially running out just before it is needed).
- There may be the occasional time when low flows are reached during winter, but the augmentation volume is fully used, and augmentation cannot continue. Based on historical records, this would occur infrequently (1 year in 10).

6 Groundwater Modelling

In 2013, Aqualinc developed a three-dimensional numerical flow model of the Ruataniwha basin as part of the PC6 hearing. The model was updated and has been used to test the hydraulic response of the groundwater and surface water system in the basin from the subject proposed Tranche 2 groundwater take applications. This work has been completed in a collaborative manner with HBRC and the eight applicants.

Since the original model documented in Weir (2013²⁰), the model has been updated and recalibrated, with a particular focus on matching low (dry-period) river flows to align with HBRC's water management strategies. Good matches were achieved between measured and modelled outputs. The model simulates the period 1972-2012, which incorporates a wide range of climatic variability from very wet to very dry years. Details of the model updates and the scenarios are provided in the Aqualinc report to Hawke's Bay Regional Council in Appendix B.

The updated numerical model has been used to quantify the stream depletion effects of the proposed Tranche 2 takes and the subsequent surface water augmentation requirements. This is not an exact science as there are several variables that need to be assumed or approximated. However, the modelling work has provided a realistic quantification of the magnitude, location and timing of effect.

To reduce the influence of measurement and model uncertainty, the most appropriate application of model results is to consider changes in key outputs (river flows and groundwater levels) rather than absolute values. In this regard, the model tests the effectiveness of the proposed augmentation to mitigate the changes in river low-flows that would be induced by the Tranche 2 takes.

The numerical model domain encompasses the Ruataniwha basin and does not extend to HBRC's flow monitoring site on the Tukituki River at Red Bridge, down catchment. Therefore, the flow monitoring sites on the Waipawa at SH2 and Tukituki at Tapairu Road have been considered to represent this site. So long as the combined 7-day MALF at the SH2 and Tapairu sites are maintained (or improved), then there will be no adverse downstream effects on low flows at Red Bridge as a result of the proposed Tranche 2 takes.

29 | Page

²⁰ Statement of Evidence of Julian James Weir for Ruataniwha Water Users Group (Groundwater Modelling). Expert evidence presented before the Board of Inquiry for the proposed Tukituki Catchment Plan Change 6. 7 October 2013.

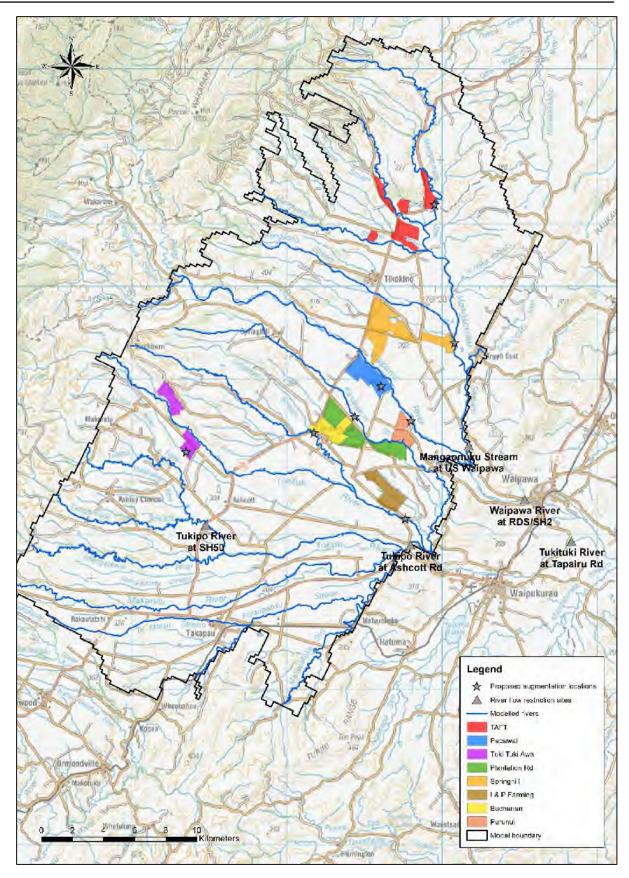


Figure 13: Location of Applicants' Properties and Augmentation Discharge Sites

6.1 Scenarios Modelled

Several scenarios of abstraction and augmentation were modelled by Aqualinc, as follows:

- Baseline Scenario (Status Quo): This includes all existing takes, but no proposed Tranche 2 takes, and represents the current irrigated area of approx. 6,000 ha within the Ruataniwha Basin (comprising 4,025 ha from groundwater and 1,975 ha from surface water), assuming the irrigated land use is pasture.
- Scenarios 1-8: One model run for each of the eight proposed Tranche 2 applicants. These model runs are all founded on the Baseline scenario, but with each applicant's proposed full irrigation needs at their proposed location. The results of these scenarios (compared to the Baseline) were then used to determine the timing of the augmentation requirements (for mitigating stream depletion effects of the irrigation take alone) that would feed into subsequent augmentation scenarios.
- Augmentation Scenarios: Initially, a combined model scenario including all applicants' takes and with each applicants' augmentation operating. Then additional scenarios that balance environmental flows and irrigated areas, as follows:
 - o Augmentation Scenario 1: This scenario includes the combined applicants' proposed irrigation and augmentation takes and assumes that augmentation occurs only when RRMP Table 5.9.3 minimum flows are reached and when irrigation is occurring.
 - o Augmentation Scenario 2: This scenario assumes that augmentation occurs whenever RRMP Table 5.9.3 minimum flows are reached throughout the year, regardless of whether irrigation is occurring or not. This scenario aims to better mitigate the temporal effects of the takes during low-flow periods through more frequent augmentation. This in turn results in a greater volume of Tranche 2 water taken for augmentation, and in some cases means individual applicants are forecasted to take more than their applied volume. Therefore, some irrigated areas (and therefore irrigation volumes) have been reduced to counter this.
 - o Augmentation Scenario 3: Under this augmentation scenario, an alternative augmentation time series has been trialed whereby low flow restrictions (and therefore augmentation) are assumed to occur sooner (at higher flow rates) than those listed in RRMP Table 5.9.3 minimum flows. These raised low-flow trigger values have been applied to the augmentation takes regardless of whether or not irrigation is occurring on the day (as was the case for augmentation Scenario 2; augmentation is assumed to occur even if the applicant is not irrigating). This scenario aims to better maintain existing users' reliability by triggering augmentation at higher river flows. In turn, this (again) results in a greater volume of Tranche 2 water taken for augmentation, which pushes some applicants total take beyond their applied volumes. So, some irrigated areas (and therefore irrigation volumes) have been further reduced to counter this.
 - o Augmentation Scenario 4: Based on the results of Augmentation Scenario 3, some river flow sites were not fully mitigated at low flows and other sites were over compensated. Therefore, this Scenario iteratively arrives at an 'optimised' solution balancing irrigation takes with augmentation discharges.

6.2 Revised Overall Tranche 2 Proposal

The balancing of irrigation demand, augmentation takes and augmentation locations to achieve the environmental flow targets (under optimised Augmentation Scenario 4) has resulted in the following seasonal allocations for each applicant (adapted from Table 28 of the Aqualinc modelling report).

Table 1: Seasonal Allocations -irrigation area and Tranche 2 groundwater volumes

Applicant	Modelled augmentation rate	Area irrigated from Tranche 2	Modelled 90 pe	Tranche 2 GW volume			
	(I/s, daily average)	groundwater (ha)	For irrigation	For augmentation (% of total)	Total	to be taken (m³/year)	
TAFT	189	850	2,841,220	2,073,700 (42%)	4,914,920	4,914,920	
Papawai	24	320	1,010,817	464,700 (31%)	1,475,517	1,475,517	
Tuki Tuki Awa	10	136	678,100	29,100 (4%)	707,700	952,400	
Plantation Rd Dairies	103	459	2,418,225	1,333,000 (36%)	3,751,225	3,751,225	
Springhill Dairies	38	188	588,313	416,900 (41%)	1,005,213	1,005,213	
I & P Farming	22	310	916,010	284,000 (24%)	1,200,010	1,200,010	
Buchanan	51	230	786,594	359,200 (31%)	1,145,794	1,145,794	
Purunui	14	93	370,321	184,600 (33%)	554,921	554,921	
		Total	9,609,600	5,145,700 (35%)	14,785,300	15,000,00 0	

As evident in the above table, the modelled total abstraction does not equate to 15 million m³/year. This is a result of the amended proposal by Tuki Tuki Awa to only use Tranche 2 groundwater when their surface water take is restricted. However, this does not mean that the balance (244,700 m³/year) is available for allocation to other applicants, because in some extreme years the full allocation sought by Tuki Tuki Awa may be used.

The values in Table 1 have been derived from the following:

- Irrigated areas and augmentation discharge rates a little smaller than applied for by some applicants;
- Adopting higher river low-flow restrictions (than those set out in Table 5.9.6 of the RRMP) to provide improved environmental low flows and to protect existing users' reliability;
- Full augmentation whenever these higher river flow restrictions are triggered, regardless of whether or not Tranche 2 irrigation is occurring; and
- Full augmentation occurring from all Tranche 2 applicants when any one of the flow monitoring sites within the basin is triggered. This acknowledges that all Tranche 2 applicants are operating collaboratively and effects from any one take can propagate across several streams.

This results in the following (modelled) changes in flows at the various river low monitoring sites for the given proposed low-flow triggers (reproduced from the Executive Summary of the Aqualinc modelling report) in Table 2:

Table 2: Modelled Changes in 7-day MALF (over the period 1972-2012) at the various river low monitoring sites for the given proposed low-flow triggers

Site						
Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa		
RRMP Table 5.9.6 minimum flow (I/s)						
2,500	2,300	150	1,043	1,170		
Assumed low-flow restriction applied (I/s) (1)						
2,725	2,360	155	1,085	1,295		
Change in mean 7-day annual low flow (MALF) (I/s)						
+3	+135	-1	+1	+2		
Change in mean 7-day MALF as a percentage of low-flow limit						
+0.1%	+5.7%	-0.6%	+0.1%	+0.2%		
1) These are higher than current RRMP Table 5.9.3 limits to provide greater environmental benefit during low flows and to protect reliability of existing users.						

The small residual negative change to 7-day MALF flow in the Tukipo River is smaller than both model uncertainty and measurement precision.

As set out in Section 5.4 of this report, several management, operational and compliance protocols are proposed to aid practical implementation of the Tranche 2 takes and augmentation.

The following key findings are noted from the modelled scenarios:

- The effects on river flows from the proposed additional Tranche 2 takes are spread over both space and time throughout the basin.
- If augmentation occurs when both existing minimum flows are reached and when irrigation would be operating, it is insufficient to mitigate depletion of 7-day MALF flows due to additional Tranche 2 takes.
- If augmentation is applied when minimum flows are reached, regardless of whether or not irrigation is operating, augmentation is more beneficial and results in an overall improvement in low flows exiting the basin in both the Tukituki and Waipawa rivers. The exception to this is Tuki Tuki Awa who propose to take Tranche 2 groundwater and augment to the Tukituki River only when their existing surface water take is restricted. This also does not adversely affect low river flows.
- For the Tranche 2 applicants, the adoption of higher minimum flow triggers (and therefore more frequent augmentation) provides greater benefit to 7-day MALF. It also better protects the reliability for existing abstraction consent holders.
- Targeted over-compensation (i.e., the positive changes in MALF) accommodates modelling prediction uncertainty (qualitatively), enhances environmental low-flows, and accommodates across-catchment effects.
- Under augmentation Scenario 4, adverse effects on surface water low flows as a result of Tranche 2 takes are either avoided (positive effects occur as evidenced by the increased flows at the Waipawa and Tukituki low flow sites), or are so minor that they fall within the margin of modelling and measurement uncertainty (i.e., for the Tukipo SH50 low flow site).

• The reported irrigation and augmentation volumes are based on a 90-percentile year. Therefore, it is possible that in extreme dry years (e.g., 1 year in 10), low flows could still be triggered after irrigation and augmentation volumes have been exhausted.

7 Statutory Matters

Section 104(1) of the Resource Management Act 1991 ("RMA") specifies the matters that the consent authorities (in this case HBRC and HDC) must have regard to when considering the applications for resource consents, as follows:

- "104 Consideration of applications
- (1) When considering an application for a resource consent and any submissions received, the consent authority must, subject to Part 2, have regard to—
 - (a) any actual and potential effects on the environment of allowing the activity; and
 - (b) any relevant provisions of—
 - (i) a national environmental standard:
 - (ii) other regulations:
 - (iii) a national policy statement:
 - (iv) a New Zealand coastal policy statement:
 - (v) a regional policy statement or proposed regional policy statement:
 - (vi) a plan or proposed plan; and
 - (c) any other matter the consent authority considers relevant and reasonably necessary to determine the application."

The matters that are to be considered by the consent authorities under section 104 of the RMA include, subject to Part 2, any actual and potential effects on the environment and any relevant objectives, policies, rules or other provisions of a Plan or Proposed Plan.

In determining applications for Discretionary and Non-Complying Activities, section 104B of the RMA states that:

- "104B Determination of applications for discretionary or non-complying activities After considering an application for a resource consent for a discretionary activity or non-complying activity, a consent authority—
- (a) may grant or refuse the application; and
- (b) if it grants the application, may impose conditions under section 108."

Under s104D(1) of the RMA, a consent authority may only make a decision to grant resource consent to a Non-Complying Activity if it is satisfied that either:

- "(a) the adverse effects of the activity on the environment (other than any effect to which section 104(3)(a)(ii) applies) will be minor; or
- (b) the application is for an activity that will not be contrary to the objectives and policies of—
 - (i) the relevant plan, if there is a plan but no proposed plan in respect of the activity; or
 - (ii) the relevant proposed plan, if there is a proposed plan but no relevant plan in respect of the activity; or
 - (iii) both the relevant plan and the relevant proposed plan, if there is both a plan and a proposed plan in respect of the activity."

An assessment of the actual and potential effects of the proposed activities on the environment is provided in Section 8, and an assessment of the proposal against the relevant objectives and policies of the relevant statutory planning documents is provided in Section 9.

8 Assessment of Environmental Effects

8.1 Surface Water Flow Effects

8.1.1 Low Flow Monitoring Sites

The proposed Tranche 2 groundwater abstractions being applied for will be taken from selected locations on each of the eight properties across the Ruataniwha Basin as displayed in Figure 13. However, groundwater modelling has determined that regardless of the point of take, effects of the proposed abstractions will impact surface water to varying degrees across the whole basin.

The Aqualinc 2021 modelling shows that proposed augmentation of rivers and streams when low-flow restrictions are imposed benefits the Tukituki and Waipawa Rivers exiting the basin, regardless of whether irrigation is occurring or not. The proposed Tuki Tuki Awa Tranche 2 take and augmentation, however, will only be activated when the farm's existing surface water take is restricted due to low flow bans in the Tukituki River. Modelling has shown that this does not adversely affect low flows.

However, if augmentation is only activated upon commencement of irrigation, the effects are such that the proposed groundwater contribution to waterways is not sufficient to offset depletion of the 7-day MALF.

In order to provide additional benefit to the waterways and mitigate prediction uncertainty and across-catchment effects, a proposed higher low-flow restriction scenario is included in the Tranche 2 Application modelling. This will result in more frequent augmentation and greater benefit to the 7-day MALF.

Table 2 outlines the existing low flow restrictions (I/s) as set out in the RRMP (Table 5.9.6) and the proposed low-flow scenario, along with the changes in mean 7-day MALF.

The Aqualinc modelling shows that proposing a higher minimum flow trigger results in positive effects at the Waipawa and Tukituki monitoring sites where increased flows are initiated. Although a small negative effect is predicted at the Tukipo (SH50) low flow site, it is so minor that it is below measurement precision and modelling uncertainty.

Generally, adverse effects on surface water as a result of the Tranche 2 groundwater abstractions are either avoided or considered minor based on a 90-percentile year; however, low flow restrictions could still be triggered during extreme dry years even after irrigation volumes have been fully utilised and augmentation discharged to surface water. As HBRC allocate water on a 9-in-10 year basis, mitigation of the very extreme years is beyond the control of the Applicants.

8.1.2 Small Streams and Wetlands

Potential effects on wetlands and minor waterbodies have been assessed by BGSL²¹ (see report in Appendix C) using the Aqualinc 2021 assessment which predicted effects on shallow groundwater and applying the estimated changes to sites inspected in the field.

There are multiple small-scale streams and drains in the catchment. To determine which sites could be considered significant and warrant further assessment, the Tranche 2 Applicants provided information to BGSL regarding their local creeks, wetlands and streams. These included reaches that typically dry

²¹ Letter from Bay Geological Services Ltd to Hawke's Bay Regional Council, titled "Ruataniwha Basin Tranche 2 Application Response to PDP's Preliminary Review of the Aqualinc Model Items 9 and 10 Only", dated 27 May 2021 (Reference BGS201-07)

up naturally in summer months, and therefore present little instream value, to those that flow or are wet year-round. Local knowledge of the Ruataniwha Plains was also used to inspect sites of interest, such as areas of recorded upwellings and spring flows.

Not every reach or wetland was inspected, as this was outside reasonable scope for the required outcome. However, sites were selected in strategic parts of the basin, these being representative of effects in other reaches across the study area.

Guidance was also sought from Hawke's Bay Regional Council (HBRC) who provided information detailing wetland areas within southern and central Hawke's Bay. HBRC provided shapefiles of the wetland State of the Environment (SOE) programme data, which was used by Aqualinc to generate a map to show the location of the wetlands.

The majority of the identified significant wetlands lie outside the study area and outside the influence of the proposed Tranche 2 groundwater takes. Only one significant wetland (near the upper reaches of the Mangatewai River) is identified within the study area (Figure 14).

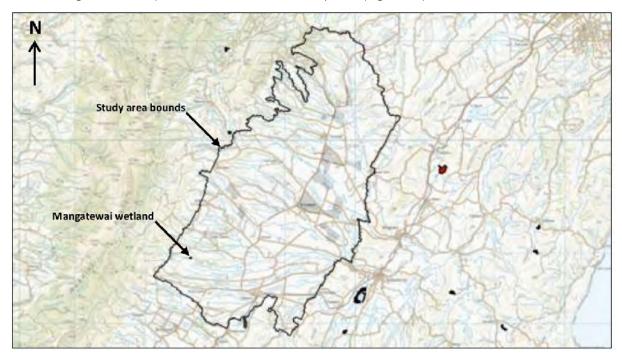


Figure 14: HBRC SOE wetland sites across central and southern Hawke's Bay.

The Ruataniwha Basin wetland and minor streams/waterways were visited by BGSL in March 2021 during a very dry period, to record the status of the surface water features during a very dry period. The sites visited are shown as green circles in Figure 15.

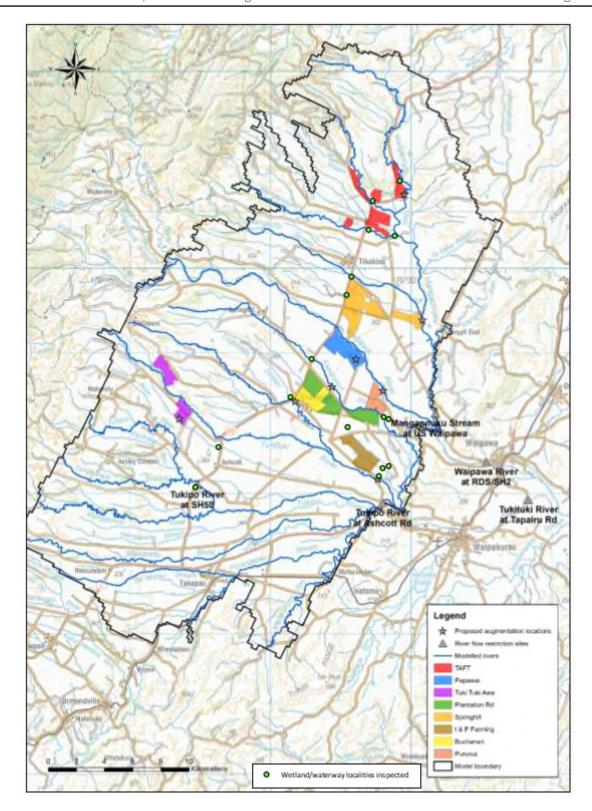


Figure 15: Surface water sites inspected by BGL (green circles).

The following surface water effects are predicted by BGSL, noting that the modelled effect is a prediction at a point in time during an extreme dry event, and not during a typical irrigation year:

• North of study area – the predicted effect on surface water features is estimated to be negligible and not likely to affect wetlands or minor streams.

- West of SH50 the predicted effects on streams that typically dry out during the summer are thought to be negligible. However, it is possible that streams may experience dry conditions slightly earlier and/or resume flowing again a little later in the season (by a matter of a few days).
- Central and eastern areas large wetlands and minor streams were observed in the field which appear to be fed by a possible subsurface divergent channel of the Waipawa River upwelling in areas of lower elevation, where changes in lithology result in poor confinement. Modelled effects on shallow groundwater as a result of the proposed Tranche 2 takes, are greatest in this area. However, anecdotal evidence and field inspections suggest that the majority of surface water levels are unlikely to result in significant effects on the majority of the natural occurring waterways, although, in places the effects will be measurable. The estimated effect across the lower reaches of the Black Stream may be such that the low flow rate observed in March 2021 could be reduced to a trickle.
- Southwest area the predicted effects on streams that typically dry out during the summer are considered negligible. However, it is possible that streams may experience dry conditions slightly earlier and/or start flowing again a little later in the season (by a matter of a few days). Therefore, minor effects are anticipated on the wetland areas inspected.

On the basis of the above assessment, it is considered that, overall, potential effects on wetlands and minor surface waterbodies will be negligible or, at worst, no more than minor.

8.2 Groundwater Level and Well Interference Effects

Potential effects on groundwater levels and on existing groundwater wells within the Ruataniwha Basin have been assessed by the Lattey Group²² (see report in Appendix D) and by BGSL²³ (see letter in Appendix E), using the Aqualinc 2021 assessment (Appendix B) which predicted effects on shallow groundwater levels and applying the estimated changes to bores inspected in the field.

A map of groundwater level difference between the status quo and augmentation Scenario 4 (being that proposed) is provided in the Aqualinc 2021 assessment²⁴ and presented in Figure 16. This demonstrates how shallow groundwater levels are predicted to change spatially during dry periods. Aqualinc 2021 predicts that shallow groundwater levels will lower up to a maximum of 0.8 m under augmentation scenario 4, focused near the applicants' properties. Elsewhere, shallow groundwater levels are predicted to change less than 0.3 m.

²² "Ruataniwha Basin Tranche 2 Irrigation Water Permit Consent Application – Assessment of Well Interference Effects, Central Hawke's Bay", prepared by Lattey Group, dated December 2020.

²³ Letter from Bay Geological Services Ltd to Hawke's Bay Regional Council, titled "Ruantaniwha Basin Tranche 2 Application Investigation of Shallow Groundwater Bores Identified in Assessment of Well Interference Effects". dated 06 August 2021 (Reference BGS201-08).

²⁴ Page 45.

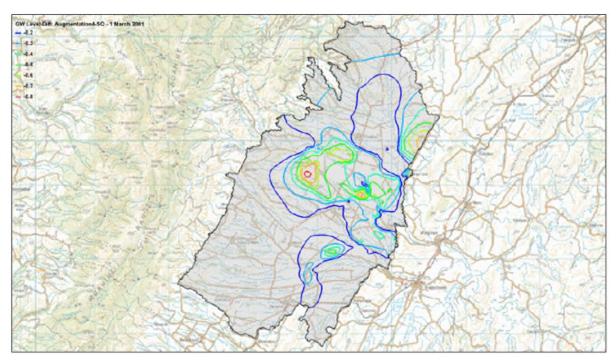


Figure 15: Difference in shallow (layer 1) groundwater levels at March 2001 between status quo and augmentation scenario 4

The Lattey report (2020) carried out a basin-wide well interference assessment associated with the proposed Tranche 2 groundwater takes. The assessment considered all recorded wells within the Ruataniwha Basin. There are many wells (703) and Lattey managed these by dividing them into groups based on their recorded total well depths. The initial focus was on wells within the 0 m to 50 m depth range and these were assessed against the Aqualinc 2021 model shallow (layer 1) impact predictions. To assess the potential impact on deeper wells, the modelled drawdown impact at a deeper level (layer 6) was reviewed separately against the deeper wells. It was not possible to fully consider wells of unknown depth, but, if it was assumed that they were sufficiently deep to enable "efficient" taking of water, Lattey considered they were unlikely to be adversely impacted.

Of a total of 657 wells (this value excluded 46 unknown depth wells) that were individually assessed, 94 were initially flagged in the well groups and 24 remained flagged following some further consideration. Of those flagged, 14 wells were in the depth range of <50 m and 10 were >50 m deep. Lattey 2020 considered that 96% of the wells assessed would not be adversely affected by the proposed Tranche 2 groundwater takes, but advised that further investigation was needed to assess whether the remaining 14 wells would be adversely affected or not.

BGSL were then engaged to assess the 14 remaining wells. They noted that, a rule of thumb suggested that if predicted well interference within a neighbouring bore was greater than 20% of the remaining water column in the bore, then there was a possibility that the neighbouring bore might be adversely affected by the additional drawdown. They noted that an adverse impact on the 14 wells identified could not be ruled out, as they had values greater than 20% and their use was not specified, or they were domestic or stockwater bores.

BGSL inspected the shallow wells in the field to gather additional information on well status, apart from one site at Well No. 5211 where no contact was able to be made with the landowner. Three of the 14 bores were found to be either abandoned, not in existence, or used as monitoring wells. Of the remaining 11 wells, the majority were shallow and provided domestic and stockwater, operated by

surface pumps, apart from the deepest well (Well No. 10978) which was a submersible pump. The remaining 11 bores identified as having potential to be adversely affected are set out in Table 3.

Table 3: Remaining bores that have potential to be adversely affected

Details-of-Flagged-Well-within-Ruataniwha-Basin□						
Well·No.¶ (diam.)¶ Owner¤	Screen¶ (m) bgl¶ (aquifer)□	SWL¶ (m)·datumo	USE¶ (1,2)¤	Pumpo	Predicted·DD·(m)· (as·%·of¶ Remaining·Head)□	Potential·for¶ adverse·effect·(Y/N)□
1357¶ (100mm)≖	(<u>well</u> -is-5.18·m·deep)¶ (limestone)¤	-2.00¤	domestic-&- stockwater: supply=	surface¤	0.72·(65%)¤	Yes, due to large potential drawdown≖
2773¶ (150mm)¤	12.00-18.00¶ (<u>brown</u> ·gravel)¤	-7.10=	domestic- water-supply¤	surface¤	0.21-(84%)¤	Yes, due to large potential drawdown, and that well currently pumps gravels during summer months
2902¶ (100mm)¤	2.11-4.11¶ (bl/brn;gravel)¤	-1.45¤	domestic- water-supply¤	surface¤	0.36·(42%)¤	Yes, due to large potential effect on remaining heads
3590¶ (100mm)¤	(<u>well</u> -is-7.30·m·deep)¶ (?-limestone)¤	-5.50¤	domestic-&- stockwater: supply=	surface¤	0.13·(52%)¤	Yes, due-to-large-potential- drawdown and the-bore-can- run-dry-in-the-summer- months.=
3664¶ (100mm)¤	14.03-25.60¶ (<u>brown</u> , white- limestone¶ brown-gravel)¤	-8.02¤	domestic- water-supply¤	surface¤	0.11·(82%)¤	Yes, due to large potential effect on remaining head¤
3690¶ (100mm)¤	(<u>well</u> -is-7.50·m·deep)¶ (?-red-gravel)¤	-4.30¤	domestic- water-supply¤	surface¤	0.47·(32%)¤	Yes, due-to-large-potential- drawdown, even though the- bore-taps-a-spring-fed-stream- that-has-never-run-dry=
3843¶ (300mm)¤	10.50-16.50¶ (<u>bm</u> -gravel)¤	-4.17¤	pastoral· farming¶ irrigation¤	not-used¤	0.08-(33%)¤	Yes, due-to-large-potential- effect-on-remaining-head- (noting-that-the-well-is-not- currently-used)
5211¶ (300mm)¤	9.00-18.50¶ (<u>bm</u> -gravel)¤	-3.90¤	cropping¶ irrigation¤	likely- surface¤	0.39·(27%)¤	Yes, due to large potential drawdown and the potential effect on remaining heads
5532¶ (1200mm)¤	(<u>well</u> -is-5·m·deep)¶ (<u>grev</u> ·gravel)¤	-1.92=	miscindustry¶ wastewater:- washwater¤	surface¤	0.28·(25%)¤	Possibly-No, due to the water take abstracted adjacent to and within the bed of the Waipawa River. ### Waipawa River.**
10978¶ (100mm)¤	21.88-22.82¶ (gravel)¤	-9.44≖	domestic-&- stockwater- supply¤	submersible¤	0.57-(66%)=	Yes, due-to-large-potential- drawdown-and-potential-effect- on-remaining-head-which-may- drop-water-level-below-pump.¤
15866¶ (1000mm)≖	(<u>well</u> ·is·4·m·deep)-¤	-1.92 =	domestic water supply¤	surface¤	0.08-(<mark>62%</mark>)¤	Yes, due to large potential drawdown and the potential effect on remaining heads

BGSL advised that Well No. 5532 was described by the owner as a sump dug beside the Waipawa River bed, and it was inferred that the well would be directly recharged by the Waipawa River and not likely adversely affected should the modelled well interference of 280 mm occur.

BGSL applied bore and aquifer data, including screen elevation, static water level, seasonal variation, pump depth, and predicted well interference and concluded that 10 existing wells may experience well interference of 25% to 84% of the Remaining Head of Water, which may adversely affect security of supply to the landowners/occupiers. However, they found that six of the 10 bores were constructed relatively shallow, to depths less than 7.5 m, which resulted in a small available water column that was more sensitive to well interference. BGSL therefore considered that it was likely these wells struggled already during periods of low groundwater levels.

Policy 77(c) of the RRMP is "To manage the groundwater resource in such a manner that existing efficient groundwater takes are not disadvantaged by new takes". 'Efficient taking' is defined in the RMMP as follows:²⁵:

For the purposes of this Plan "efficient taking" of groundwater means abstraction by a bore which penetrates the aquifer from which water is being drawn at a depth sufficient to enable water to be drawn

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²⁵ Note 21 under Policy 77, on page 107 of the RMMP.

all year (i.e. the bore depth is below the range of seasonal fluctuations in groundwater level), with the bore being adequately maintained, of sufficient diameter and screened to minimise drawdown, with a pump capable of drawing water from the base of the bore to the land surface.

BGSL's assessment suggests that at least six of the ten existing bores are currently not providing efficient takes and are already disadvantaged during dry periods of low groundwater levels. Therefore, four of the existing bores may be adversely affected to a minor extent.

8.2.1 Summary

On the basis of the Aqualinc 2021, Lattey and BGSL modelling and assessments, it is concluded that:

- Groundwater will not be mined. While groundwater levels will lower further with additional Tranche 2 takes (made available under Plan Change 6), they will not continue to lower, they will simply reach a new (lower) dynamic equilibrium. Shallow groundwater levels are predicted to lower a maximum of 0.8 m in the vicinity of the Tranche 2 take locations, and less than 0.3m further afield;
- Of the 657 wells within the Ruataniwha Basin that were investigated, only 10 bores are expected to experience well interference of 25% to 84% of the Remaining Head of Water which may adversely affect security of supply to the landowners/occupiers. Of those 10 bores, it is very likely that six of them are currently not providing efficient takes and are already disadvantaged during periods of low groundwater levels. Therefore, four of the existing bores may be adversely affected to a minor extent.

8.3 Positive Effects

8.3.1 Te Awahohonu Forest Trust (TAFT)

TAFT has a long-term (100 year) vision to utilise the Tranche 2 water which will:

- Increase productivity from the land;
- Create a more resilient farming operation in Central Hawkes Bay;
- Give the Trust more flexibility in future land use opportunities;
- Create jobs for the Trust owners, Iwi, and other local residents;
- Improve the surrounding environment; and
- Help stimulate the local economy

The Trusts vision for their allocation of water is staged.

Initially water will be applied to the existing farm to allow for increased stock trading. In the first stage of irrigation the farm could trade at least an additional 3,000 - 5,000 lambs and 800 cattle each year, increasing farm output by over 25%. The farm will likely require an extra 1 or 2 full time labour units to manage the increased stock number movements and workload associated with monitoring and managing an efficient irrigation system.

Irrigation allows stock finishing to be spread more evenly over the year and involve a less intensive winter cropping regime, the result being a reduction in overall nutrient losses into the environment. The irrigation program is set to be accompanied by planting programs which will enhance the indigenous vegetation environment around waterways and across the farm.

The second stage of Irrigation for TAFT involves the utilisation of more land in the immediate area to enable new business opportunities and create employment opportunities. The possibilities include horticulture and cropping.

8.3.2 Papawai Partnership

With the aid of Tranche 2 water Papawai will have the ability to grow different crops for McCain's – sweetcorn and beans. Beans can yield 12-15 tonnes/ha with adequate irrigation, which is at the top end of the yield curve. Papawai will also be able to grow more supplement feed for its stock, to be used in times of droughts or extreme weather patterns, as we have seen in 2020. Tranche 2 water will also provide a 'safety net' to enable seed production with different companies that have approached us, growing seeds of carrots, broccoli, and onions.

Tranche 2 water will not necessarily increase yields on existing crops already grown with the use of Papawai's existing water consent, but it will assist with guaranteeing a good solid yield every time a crop is planted. A good crop maintains lower weed production and so less herbicide and insecticide will be used. If Papawai is able to maintain its business, it hopes to employ another full-time staff member, and an extra seasonal staff person when required.

The 2019/2020 year was very trying with a drought, that has given Papawai another set of problems. All of the existing consented water was used just trying to keep grass alive and growing, so that lambs could get to a weight acceptable for the meat works. Additional Tranche 2 water would enable Papawai to avoid that situation in the future.

8.3.3 Tukituki Awa

Tukituki Awa's main reason for seeking Tranche 2 water is to alleviate the reduced security of supply for its existing surface water irrigation take resulting from the increased minimum flows on the Tukituki River. In the event of low flow restrictions being imposed on surface water takes (as occurred this 2019/2020 season when low flow restrictions were imposed early in the season and not lifted until late in May) the Tranche 2 water will enable reliable irrigation and consequently on-farm production to be maintained.

In that context, the positive effects of having Tranche 2 water can be directly equated avoiding the losses that occurred this season. Those losses were a reduction of 30,000 kg milk solids (lost revenue of \$146,000) and extra feed being required (PKE, corn waste and maize silage at a cost of \$114,000) or \$260,000 in total.

8.3.4 Plantation Road Dairies Ltd

Should Plantation Road Dairies' (PRD) Tranche 2 groundwater application be granted, then the intention is to transition from dairy support farming into horticulture. The PRD property is sizeable and so this transition would occur over a 5 to 10-year period. The investment required for that transition will be significant, including the purchase of goods and services and the employment of additional staff both during the transitional phase and the operational phase of the horticultural development. At this stage there are no definitive development plans (including the likely horticultural activities and mix of plantings) and that planning will commence once the Tranche 2 water is secured.

Should this transition to horticulture proceed, then there is a very real likelihood that the environmental footprint of the property would reduce, particularly in terms of nitrogen leaching to the underlying shallow groundwater.

8.3.5 Springhill Dairies (formerly Ingleton Farms)

Springhill Dairies intends to use the Tranche 2 water to irrigate land on its run-off. There will not be any increase in cow numbers, with the main benefit being that feed crops can be grown for young stock. Feed crops could also be grown, harvested and fed to cows. These initiatives will result in Springhill being less

dependent on imported feed such as palm kernel. There will be additional employment for contractors involved in planting and harvesting these feed crops.

8.3.6 I & P Farming (formerly Abernethy Partnership)

I & P Farming operates a mixed cropping and livestock finishing farm. The land is currently unirrigated, flat and comprises a high proportion of quality soils well suited to crop production, including Hastings silt loam and Kaiapo silt sandy loam. Access to Tranche 2 groundwater will reduce volatility in cropping performance that is driven by periods of low soil moisture over the cropping season. This reduction in risk will provide I & P Farming with the confidence to optimise its cropping production and with positive effects including but not limited to:

- New investment. Initial estimates are that approximately \$3 million of infrastructure investment may be required to establish an irrigation system as well as the related re-purposing of farm operations. As well as directly benefiting suppliers, surrounding farms and the community may also benefit from this investment such as through any required power supply upgrades;
- Optimised land use. Once access to Tranche 2 water is secured, I&P Farming intends to commission a review of farm soil types and environmental factors in order to optimise the longterm sustainability of production. It is anticipated that this will result in a re-weighting of production towards higher value process vegetable crops, as well as enhancing crop mixtures and rotation to maintain soil quality;
- Significant crop production increase. Although the review of land use is yet to be undertaken, it is estimated that average yields on cereal and pea crops will increase by 3 tonnes/ha under irrigation. This increase would result in at least an additional ~550 tonnes of total crop production per annum across the intended irrigation area of approximately 184 ha.
- Reduced soil erosion risk. Irrigation will reduce the turnaround time between crops, allowing better cover to be maintained and the use of soil enhancing cover during soil water deficits; and
- Increased employment. I & P Farming currently employs 1 full time farm manager as well as engaging regular part-time labour as required and outsourcing all other needs. Increased crop production of at least 550 tonnes per annum as well as maintaining a ~\$3 million irrigation system, will lead to an increase in part-time labour as well as regular work opportunities for Hawkes Bay contracting firms and support of local agri-business suppliers. The reduced volatility in cropping performance under irrigation, should translate into greater stability of business from the farm for local suppliers.

8.3.7 Buchanan Trust No 2

The Trust currently rears bulls year-round on dry land. Tranche 2 groundwater would enable the Trust to transition to growing crops under irrigation during the summer months and trading lambs on grass during the winter. An anticipated typical year of irrigated crop yields would be 22 tonnes of maize, 23 tonnes of sweetcorn, 9 tonnes of peas, 9 tonnes of barley, 500 kg of carrot seed and 200 kg of radish seeds. The Trust may also lease some irrigated area to a squash grower.

Importantly, the Tranche 2 water will enable the Trust to establish grass for the winter. This will enable two trades of 18 lambs per hectare (or 4500 trades in total from 127 hectares) over the winter months.

The change in the Trust's farming operation will require the employment of one extra full time labour unit to manage the increased stock number movements and workload associated with monitoring and managing an efficient irrigation system.

8.3.8 Purunui Trust

The Purunui Trust currently runs a dairy farm support unit. The availability of Tranche 2 water and the ability to irrigate 60 ha of land is expected to provide the following benefits:

- ability to move into mixed cropping, lamb and some beef finishing.
- yields of crops will increase, along with better lamb finishing weights with the increase in dry matter production.
- increase the use of agricultural contractors, and people in the agri-business sector.
- growing squash, onions and carrot seed.
- reduced soil erosion on the 60 ha of light Takapau soils.
- opportunity to diversify into Pipfruit orcharding which could provide more jobs in the area. Demand for land in this sector continues to grow.
- increases in production will require an additional 1.0 FTE employees.

9 Assessment of Planning and Policy Documents

The following provides an assessment of the proposed activities against the relevant statutory planning and policy documents, as required under section 104(1)(b) and section 104D of the RMA.

Objectives and policies relevant to the proposal are contained in the following statutory planning documents:

- National Policy Statement for Freshwater Management 2020; and
- Operative Regional Policy Statement ("RPS").

The RPS is contained within the Hawke's Bay Regional Resource Management Plan (operative 28 August 2006).

The relevant objectives and policies of the statutory planning documents are set out and assessed below.

9.1 National Policy Statement for Freshwater Management (NPS-FM) 2020

The NPS-FM 2020 came into effect on 3 September 2020. It covers the management of freshwater, including groundwater, and the effects on receiving environments. On the same date the National Environmental Standards (NES) for Freshwater Regulations came into effect.

The NPS-FM directs Regional Councils to give effect to it, including Te Mana o Te Wai, as soon as practicable. The HBRC has not yet given effect to the NPS-FM 2020 in either its Regional Policy Statement or Regional Resource Management Plan.

The relevant provisions of the NPS-FM 2020 are outlined below.

The Objective of the NPS-FM is to ensure natural and physical resources are managed in a way that prioritises:

- (a) first, the health and well-being of water bodies and freshwater ecosystems
- (b) second, the health needs of people (such as drinking water)
- (c) third, the ability of people and communities to provide for their social, economic and cultural well-being, now and in the future.

Six policies in the NPS-FM are also considered relevant to this application:

Policy 3 - that freshwater is managed in an integrated way that considers the effects of the use and development of land on a whole-of-catchment basis, including the effects on receiving environments.

Policy 5 - freshwater management through a National Objectives Framework (NOF) to ensure the health and wellbeing of water bodies is maintained and (if communities choose) improved.

Policy 6 - that there is no further loss of extent of natural inland wetlands, their values are protected, and their restoration is promoted.

- Policy 7 that the loss of river extent and values is avoided to the extent practicable.
- Policy 9 that the habitats of indigenous freshwater species are protected.
- Policy 10 the habitat of trout and salmon is protected, insofar as this is consistent with Policy 9.

Policy 11 - to ensure the freshwater is allocated and used efficiently, all existing over allocation is phased out and future over allocation is avoided.

The assessment of environmental effects in Section 8 of this report has found that:

- proposing a higher minimum flow trigger results in positive effects at the Waipawa and Tukituki
 monitoring sites where increased flows are initiated. Although a low negative effect is
 predicted at the Tukipo (SH50) low flow site, it is so minor that it is below measurement
 precision and modelling uncertainty;
- generally, adverse effects on surface water as a result of the Tranche 2 groundwater abstractions will be either avoided or considered minor based on a 90-percentile year;
- overall, potential effects on wetlands and minor surface waterbodies will be negligible or no more than minor;
- groundwater will not be mined. While groundwater levels will lower further with additional Tranche 2 takes (made available under Plan Change 6), they will not continue to lower, they will simply reach a new (lower) dynamic equilibrium. Shallow groundwater levels are predicted to lower a maximum of 0.8 m in the vicinity of the Tranche 2 take locations, and less than 0.3 m further afield;
- of the 657 recorded wells within the Ruataniwha Basin (excluding 46 unknown depth wells) that were investigated, only 10 wells are expected to experience well interference of 25% to 84% of the remaining head of water within the bores that may adversely affect security of supply to the landowners/occupiers.; and
- there will be positive effects associated with the proposed take and use of the Tranche 2 groundwater (as set out in Section 8.3), including
 - o increased productivity from the land;
 - o more resilient farming operations;
 - o more flexibility in future land use opportunities; and
 - o creation of jobs and benefits for the local economy.

As the effects on the rivers, wetlands and minor surface waterbodies are expected to be negligible or no more than minor, it is expected that there will be negligible or less than minor effects on the habitats of indigenous freshwater species, trout and salmon.

The majority of wells within the Ruataniwha Basin will not be adversely affected by the proposed Tranche 2 groundwater takes. Only the security of supply to 10 wells may be adversely affected.

It is therefore considered that the proposed activities will be generally consistent with, and not contrary to, the relevant objective and policies of the NPS-FM.

9.2 Hawke's Bay Regional Policy Statement (RPS)

The Hawke's Bay Regional Policy Statement (RPS) is contained within the Operative Hawke's Bay Regional Resource Management Plan (made operative on 28 August 2006).

The following objectives and policies of the RPS are relevant to the proposal:

OBJ 1

To achieve the integrated sustainable management of the natural and physical resources of the Hawke's Bay region, while recognising the importance of resource use activity in Hawke's Bay, and its contribution to the development and prosperity of the region.

OBJ 23

The avoidance of any significant adverse effects of water takes on the long-term quantity of groundwater in aquifers and on surface water resources.

OBJ 24

The avoidance or remedy of any significant adverse effects of water takes on the operation of existing lawful efficient groundwater takes.

POL 28 DECISION-MAKING CRITERIA – EFFECTS ON EXISTING USERS

3.9.18 To require applicants to avoid, remedy or mitigate any significant interference of new takes of groundwater on existing lawfully established efficient groundwater takes, including existing efficient takes and uses of groundwater for an individual's reasonable domestic needs or the reasonable needs of an individual's animals for drinking water or takes for firefighting.

POL 32 TECHNICAL PROCEDURE - IRRIGATION TAKES

3.9.31 To allocate groundwater for irrigation purposes on the basis of actual crop water requirements up to a maximum equal to that required during a one in ten year drought. The allocation assessment will take into account information on crop type, rainfall, potential evapotranspiration rates, and best irrigation management practices. The allocation assessment may also have regard to soil type and soil moisture capacity.

OBJ 25

The quantity of water in wetlands, rivers and lakes is suitable for sustaining aquatic ecosystems, for achieving other freshwater objectives, and ensuring resource availability for a variety of purposes across the region, while recognising the impact caused by climatic fluctuations in Hawke's Bay.

OBJ 26

The avoidance of any significant adverse effects of water takes, uses, damming or diversion on lawfully established activities in surface water bodies.

POL 38 DECISION-MAKING CRITERIA – EFFECTS OF NEW TAKES

3.10.13 To avoid any significant adverse effects of new takes, uses, damming or diversion of water on lawfully established activities in surface water bodies, including any significant adverse effects on takes and uses of water for an individual's reasonable domestic needs or the reasonable needs of an individual's animals for drinking water or takes for firefighting.

Objectives Obj 23 and Obj 24 are to avoid any significant adverse effects of water takes on the long-term quantity of groundwater in aquifers and on surface water resources and to avoid or remedy any significant adverse effects of water takes on the operation of existing, lawful, efficient groundwater takes. Objective Obj 26 is the avoidance of any significant adverse effects of water takes and uses on lawfully established activities in surface water bodies. Policy 28 and Policy 38 are to avoid, remedy or mitigate any significant interference and adverse effects of new takes of groundwater on existing lawfully established efficient groundwater takes, and to avoid any significant adverse effects of new takes and uses on lawfully established activities in surface water bodies.

Policy 32 is to allocate groundwater for irrigation purposes on the basis of actual crop water requirements up to a maximum equal to that required during a one in ten-year drought. The maximum volumes of Tranche 2 groundwater proposed to be taken and used for irrigation have been calculated for each Applicant by Aqualinc assuming that the water will be used to irrigate pasture. If land uses other than pasture are applied (e.g. cropping and/or horticulture, or a mixture of pasture, cropping and horticulture), then the seasonal water use and associated recharge will be less for the same irrigated area, or a larger area could be irrigated for the same seasonal volume. In these cases, the Aqualinc report has assessed that the modelled effects on river flows will be either less or similar (respectively) than the assessment of the modelled effects for pasture only²⁶.

As outlined in Sections 6 and 8 of this report, the Aqualinc modelling and BGSL's assessments of effects on groundwater levels and surface water flows (including small streams and wetlands) have concluded

48 | Page

²⁶ Refer to Section 3.2, page 13, of the Aqualinc report.

that the effects on the rivers, wetlands and minor surface waterbodies are expected to be negligible or no more than minor, and adverse effects on the security of supply of all but 10 existing lawfully established efficient groundwater takes within the Ruataniwha Basin will be avoided. As the effects on the rivers, wetlands and minor surface waterbodies are expected to be negligible or no more than minor, it is expected that there will be negligible or less than minor effects on the habitats of indigenous freshwater species, trout and salmon.

It is therefore considered that the proposed activities are generally consistent with, and not contrary to, these objectives and policies.

9.3 Hawke's Bay Regional Resource Management Plan (RRMP)

The following objectives and policies of the RMMP are relevant to the proposal:

OBJ 44

The maintenance of a sustainable groundwater resource.

POL 77 ENVIRONMENTAL GUIDELINES - GROUNDWATER QUANTITY

- (a) To manage takes of groundwater to ensure abstraction does not exceed the rate of recharge.
- (b) To manage the available groundwater resource to ensure supplies of good quality groundwater.
- (c) To manage the groundwater resource in such a manner that existing efficient groundwater takes are not disadvantaged by new takes.
- (d) To manage takes of groundwater to ensure abstraction does not have an adverse effect on rivers, lakes, springs, or wetlands.

OBJ TT1

To sustainably manage the use and development of land, the discharge of contaminants including nutrients, and the taking, using, damming, or diverting of fresh water in the Tukituki River catchment so that:

- (a) Groundwater levels, river flows, lake and wetland levels and water quality maintain or enhance the habitat and health of aquatic ecosystems, macroinvertebrates, native fish and trout;
- (b) Water quality enables safe contact recreation and food gathering;
- (ba) Water quality and quantity enables safe and reliable human drinking water supplies;
- (c) The frequency and duration of excessive periphyton growths that adversely affect recreational and cultural uses and amenity are reduced;
- (d) The significant values of wetlands are protected;
- (e) The mauri of surface water bodies and groundwater is recognised and adverse effects on aspects of water quality and quantity that contribute to healthy mauri are avoided, remedied or mitigated; and
- (f) The taking and use of water for primary production and the processing of beverages, food and fibre is provided for.

POL TT8 ALLOCATION LIMITS

- 1. To manage the taking of surface water and groundwater in the Tukituki River catchment by:
 - [...]
 - (b) Recognising that there is a significant degree of interconnectedness between groundwater in the Ruataniwha Basin and surface water flows within the basin as a whole and consequently surface flows further downstream;
 - [...]
 - (ca) Enabling additional groundwater to be abstracted as a discretionary activity (Table 5.9.5 Tranche 2) provided that river flows are augmented to maintain the relevant minimum flows specified in Table 5.9.3 commensurate to the scale of effect of the Tranche 2 groundwater take.

Objective Obj 44 is to maintain a sustainable groundwater resource and Policy 77 seeks to manage groundwater takes to ensure abstraction does not exceed the rate of recharge, there are supplies of

good quality groundwater, existing efficient groundwater takes are not disadvantaged by new takes, and abstraction does not have an adverse effect on rivers, lakes, springs, or wetlands.

Objective TT1 is to sustainably manage the taking, using, damming, or diverting of fresh water in the Tukituki River catchment so that: groundwater levels, river flows, lake and wetland levels and water quality maintain or enhance the habitat and health of aquatic ecosystems, macroinvertebrates, native fish and trout; water quality and quantity enables safe contact recreation and food gathering and safe and reliable human drinking water supplies; frequency and duration of excessive periphyton growths that adversely affect recreational and cultural uses and amenity are reduced; the significant values of wetlands are protected; and adverse effects on aspects of water quality and quantity that contribute to healthy mauri are avoided, remedied or mitigated.

Policy TT8 recognises the significant degree of interconnectedness between groundwater in the Ruataniwha Basin and surface water flows in the Basin as a whole, and consequently surface flows further downstream. It is also to enable additional groundwater to be abstracted as a discretionary activity provided that flows are augmented to maintain the relevant minimum flows in Table 5.9.3 commensurate with the scale of effect of the Tranche 2 groundwater take.

As outlined in Sections 6 and 8 of this report, the Aqualinc modelling, Lattey and BGSL's assessments of effects on groundwater levels and surface water flows (including small streams and wetlands) have concluded that the effects on the rivers, wetlands and minor surface waterbodies are expected to be negligible or no more than minor, and adverse effects on the security of supply of all but 10 of the 657 existing, lawfully established groundwater takes within the Ruataniwha Basin will be avoided. Four of the 10 bores may be adversely affected to a minor extent.

As the effects on the rivers, wetlands and minor surface waterbodies are expected to be negligible or no more than minor, it is expected that there will be negligible or less than minor effects: on the habitat and health of aquatic ecosystems, macroinvertebrates, native fish and trout; on contact recreation, amenity and food gathering, on the safety and reliability of safe human drinking water supplies; and the water quality and quantity that contribute to healthy mauri.

It is therefore considered that the proposed activities are consistent with, and not contrary to, these objectives and policies.

10 Section 104D Assessment

This AEE has been prepared on the basis that all applications should be assessed as Discretionary Activities. This is on the basis of legal advice relating to the effect of s 88A, Resource Management Act 1991, and how that section applies to previously operative rules that become inoperative after lodgment of a resource consent application but prior to the consent being determined.

However, in the event that these applications are assessed as a Non-complying Activity, both of the threshold tests in s 104D are met. This is because:

- a) The effects of the proposed takes are minor or less (s 104D(1)(a), RMA) (refer to the assessment of effects in Section 8 above).
- b) The only identified effects that are *potentially* more than minor relate to effects on 4 existing groundwater bores. However, when the effects of the proposal overall are assessed, these effects can be considered minor.

The activity is entirely consistent with – and is certainly not contrary to – relevant objectives and policies (s 104D(1)(b), RMA) (refer assessment at Section 9.3 of this report). The opportunity to use the Tranche 2 water in the manner proposed is expressly provided for by the RRMP, subject to certain augmentation obligations which are incorporated into the current proposal.

11 Consent Conditions

A set of proposed consent conditions is provided in Appendix F of this report, which is intended to provide the starting point of a template for consent conditions for each applicant's consent (Note: it is proposed that separate resource consents be issued for each Applicant, and tailored to include their specific details (e.g., volume of groundwater for irrigation and augmentation, maximum rate of take, etc.)).

The proposed conditions accommodate the proposed protocols for the Tranche 2 groundwater takes and augmentation, as set out in Section 5.4 of this report. This includes enabling staged development and transitional implementation of the consents, inclusion of the proposed 'water year' and associated start dates for irrigation and augmentation, and the application of higher minimum flow rates for the relevant rivers to trigger the commencement of the discharge of augmentation water.

The applicants also seek final conditions that allow for flexibility of use of any Tranche 2 water taken. This will allow crop rotations (temporary) and permanent transitions to other land uses. It is not possible at this stage to specify exactly when, and on what parcels of land that landuse change will occur. Any change in land use, to a less water demanding crop, will also allow a greater area of land to benefit from irrigation. This will increase the positive effects arising from the water take, while any effects will be addressed in the land use consent. The precise wording of these conditions will need to be further discussed with Council.

12 Part 2 RMA

The matters to be considered under section 104 are subject to Part 2 of the RMA.

11.1 Section 5 - Purpose of the RMA

The cornerstone of Part 2 is the Purpose of the Act as set out in section 5(1), which is:

"To promote the sustainable management of natural and physical resources".

Section 5(2) of the RMA defines 'sustainable management' as:

"Managing the use, development and protection of natural and physical resources in a way or at a rate which enables people and communities to provide for their social, economic, and cultural wellbeing and for their health and safety while-

- (a) Sustaining the potential of natural and physical resources (excluding minerals) to meet the reasonably foreseeable needs of future generations; and
- (b) Safeguarding the life-supporting capacity of air, water, soil and ecosystems; and
- (c) Avoiding, remedying or mitigating any adverse effects of activities on the environment."

In the recent decision *RJ Davidson Family Trust v Marlborough District Council* [2018] NZCA 316 the Court of Appeal reconfirmed the relevance of Part 2 matters in the consideration of resource consents.

In this instance, Part 2 has been assessed. This is intended to assist the overall evaluation of the current proposal.

The take and use of the proposed Tranche 2 groundwater will enable the applicants to provide for their social, economic and cultural wellbeing and for their health and safety by providing a new source of water to support productive land uses. This will also have associated benefits for the wider local and regional community.

With respect to safeguarding the life-supporting capacity of air, water, soil and ecosystems and avoiding, remedying, or mitigating any adverse effects of the proposed activities on the environment, the AEE (in Section 8 of this report) has concluded that the proposed take and use of Tranche 2 groundwater will generally be negligible or no more than minor, subject to the imposition of appropriate consent conditions, with the exception of the security of supply of 10 existing wells within the Ruataniwha Basin which may be adversely affected. Therefore, overall, it is considered that the requirements of section 5(2)(b) and (c) will be achieved.

It is therefore considered that granting consent to the proposed water takes will achieve the purpose of the RMA.

11.2 Section 6 - Matters of National Importance

Section 6 of the RMA sets out the following 'Matters of National Importance' that must be recognised and provided for in managing the use, development and protection of natural and physical resources:

- "(a) The preservation of the natural character of the coastal environment (including coastal marine area) wetlands and lakes and rivers and their margins and the protection of them from inappropriate subdivision, use and development:
- (b) The protection of outstanding natural features and landscapes from inappropriate subdivision, use and development:
- (c) The protection of areas of significant indigenous vegetation and significant habitats of indigenous fauna:

- (d) The maintenance and enhancement of public access to and along the coastal marine area, lakes and rivers:
- (e) The relationship of Māori and their culture and traditions with their ancestral lands, water, waahi tapu, and other taonga;
- (f) The protection of historic heritage from inappropriate subdivision, use and development;
- (g) The protection of recognised customary activities."

Sections 6(a), 6(e) and6(g) are relevant.

Section 6(a) requires the preservation of the natural character of wetlands, lakes and rivers and their margins and the protection of them from inappropriate use and development. The AEE in Section 8 of this report has assessed that the effects of the proposed takes and discharges of Tranche 2 groundwater on the rivers, wetlands and minor surface waterbodies are expected to be negligible or no more than minor with the imposition of appropriate consent conditions. Given this, it is expected that there will be negligible or no more than minor effects on the habitats of indigenous freshwater species, trout and salmon. It is therefore considered that the proposed takes and discharges are consistent with s6(a).

Section 6(e) requires the relationship of Māori and their culture and traditions with their ancestral lands, water, waahi tapu, and other taonga to be recognised and provided for. Section 6(g) requires the protection of recognised customary activities to be recognises and provided for.

It is considered that the proposed water takes and discharges will not have any adverse effects on this relationship. It is therefore considered that the proposed take and use of water is consistent with s6(e) and s6(g).

It is therefore considered that the proposal is consistent with the relevant Matters of National Importance in Section 6 of the RMA.

11.3 Section 7 - Other Matters

Section 7 of the RMA sets out the matters that particular regard must be had to in managing the use, development and protection of natural and physical resources:

- "(a) Kaitiakitanga:
- (aa) the ethic of stewardship:
- (b) The efficient use and development of natural and physical resources:
- (ba) the efficiency of the end use of energy:
- (c) The maintenance and enhancement of amenity values:
- (d) Intrinsic values of ecosystems:
- (e) Repealed:
- *(f) Maintenance and enhancement of the quality of the environment:*
- (g) Any finite characteristics of natural and physical resources:
- (h) The protection of the habitat of trout and salmon:
- (i) The effects of climate change:
- (j) The benefits to be derived from the use and development of renewable energy.

Sections 7(a), 7(aa), 7(b), 7(d), 7(f), 7(g), 7(h) and 7(i) are considered relevant.

The Assessment of Environmental Effects (in Section 8) has concluded that the proposed take and use of Tranche 2 groundwater will generally be negligible or no more than minor, subject to the imposition of appropriate consent conditions, with the exception of the security of supply of 10 existing wells within the Ruataniwha Basin which may be adversely affected. The water taken will be used to irrigate productive land uses and to provide greater resilience for the applicants during dry summer months, when takes from rivers are restricted. Restrictions on takes are expected to become more frequent and be of longer duration in the future as a result of climate change.

It is proposed that augmentation will be applied when minimum flows in the rivers are reached, regardless of whether or not irrigation is operating. As such, augmentation will be more beneficial and will result in an overall improvement in low flows exiting the basin in both the Tukituki and Waipawa Rivers. The exception to this is Tuki Tuki Awa, who propose to take Tranche 2 groundwater and augment to the Tukituki River only when their existing surface water take is restricted. This also will not adversely affect low river flows. For the Tranche 2 applicants, the adoption of higher minimum flow triggers (and therefore more frequent augmentation) will provide greater benefit to the 7-day MALF, and it will also better protect the reliability for existing abstraction consent holders. Targeted over-compensation will accommodate prediction uncertainty, enhancing environmental low-flows, and will accommodate across-catchment effects.

Groundwater in the Ruataniwha Basin is a finite resource. Groundwater will not be mined. While groundwater levels will lower further with the proposed Tranche 2 groundwater takes (in accordance with the 15 million m³ allocation made available under PC 6), they will not continue to lower and will simply reach a new, lower, dynamic equilibrium. Shallow groundwater levels are predicted to lower a maximum of 0.8 m in the vicinity of Tranche 2 take locations, and less than 0.3 m further afield.

On the basis of the above, it is considered that the proposal is consistent with the relevant Other Matters in Section 7 of the RMA.

11.4 Section 8 - Treaty of Waitangi

Section 8 of the RMA states that, in relation to managing the use, development and protection of natural and physical resources, the principles of the Treaty of Waitangi (Te Tiriti o Waitangi) must be taken into account.

While there is no comprehensive or authoritative list of principles of the Treaty of Waitangi available for consideration, case law has indicated that these may include principles of active protection, good faith consultation and communication, and a spirit of partnership.

The proposal will not cause a change in the manner in which the principles of the Treaty of Waitangi apply.

11.5 Conclusions

Overall, the proposal is consistent with the purpose and principles of the RMA.

13 Summary and Conclusion

Plan Change 6 (PC6) to the Hawke's Bay Regional Resource Management Plan (RRMP) became operative on 1 October 2015. PC6 set groundwater allocation limits for the Ruataniwha Basin which is located within the upper Tukituki River catchment. Included within those limits was an allocation of deep groundwater that PC6 labelled "Tranche 2".

In response to the establishment of the Tranche 2 allocation, during the period 19 November 2014 to 1 May 2017 the following seven parties lodged resource consent applications to abstract Tranche 2 deep groundwater²⁷:

- Te Awahohonu Forest Trust (TAFT)
- Springhill Dairies (formerly Ingleton Farms)
- Tuki Tuki Awa
- Plantation Road Dairies
- I & P Farming (formerly Abernethy Partnership)
- Papawai Partnership
- Buchanan Trust No. 2

Plantation Road Dairies also amended their original application in January-April 2020, to reduce the volume of water they had applied for from 6,081,499 m³/year to 3,751,225 m³/year. Consequently, additional water was made available to Papawai Partnership to increase the volume they originally applied for by 1,052,455 m³/year, and to I & P Farming to increase their original volume by 722,888 m³/year. Purunui Trust applied in April 2020 for the remaining water (544,931 m³/year).

A fundamental element of Tranche 2 groundwater abstraction is the RRMP requirement to augment river flows to mitigate the impact of the Tranche 2 deep groundwater abstractions on surface water bodies. The augmentation water is also abstracted from deep groundwater, and it forms part of the Tranche 2 allocation. This means that each Tranche 2 applicant must specify how much deep groundwater they propose to abstract for irrigation and how much they propose to abstract for augmentation purposes.

The augmentation concept included in PC6 was novel, not only for Hawke's Bay, but for New Zealand generally. It was not instigated by the Hawke's Bay Regional Council (HBRC) but was introduced by the Board of Inquiry into the Tukituki Catchment Proposal in their June 2014 decision. Not surprisingly, the augmentation concept was not approached consistently by all of the applicants.

Each Tranche 2 groundwater abstraction will have an effect on groundwater levels and river flows across a wide area. The effects of all eight applicants then combine to produce Ruataniwha Basin-wide effects. Consequently, at a meeting with HBRC officials in March 2018, it was agreed that the applicants would work together in a collaborative manner to assess (or reassess) the cumulative effects of their proposed Tranche 2 abstractions.

This report is based on the outcomes of the collaborative modelling and effects assessments. It supersedes and replaces the individual assessments of environmental effects (AEEs) that were originally lodged by each of the eight applicants. However, while this AEE replaces the original AEEs, the original applications for resource consent remain in place.

²⁷ Two other parties also lodged applications. The first was HBRIC, the investment arm of the Hawke's Bay Regional Council. The second was J M Bostock Ltd. Both of those applications have been withdrawn.

Importantly, when the original Tranche 2 applications were lodged, it was unclear how the PC6 augmentation provisions should be interpreted and implemented. For example, it was unclear whether, or not, the effects of the deep groundwater augmentation abstractions on surface flows would themselves need to be mitigated by further augmentation, and so on. Some applicants (including TAFT, Springhill Dairies, Plantation Road Dairies and Buchanan No. 2 Trust) assumed that would be the case, while others did not.

Irrigation demand for each applicant has been calculated for the irrigable areas applied for using the Aqualinc in-house soil-water balance model, IrriCalc. From these calculations, the 90-percentile (or 9 in 10 years) irrigation demand (in mm/year) has been calculated, assuming that all existing and future irrigated land use is pasture, which typically has a larger seasonal water demand than other land uses. This demand is then multiplied by the irrigated area to calculate a maximum irrigation volume.

To provide greater flexibility for applicants to use Tranche 2 groundwater for the duration of the consents, the Applicants are seeking conditions that enable the volume of Tranche 2 groundwater to be used to irrigate crops and/or horticulture, or a mixture of pasture, crops and horticulture. While the volume of water used will be the same or less for any applicant irrigating solely pasture, the amount of land over which this water is applied may be greater than the area modelled in the Aqualinc report (in Appendix B to this report).

The proposed revised areas of the Applicants' properties to be irrigated are set out in the Revised Overall Tranche 2 Proposal in Table 1, in Section 6.2 of this report, and are set out in the proposed set of draft consent conditions in Appendix F to this report.

The applicants therefore seek final conditions that allow for flexibility of use of any Tranche 2 water taken. This will enable crop rotations (temporary) and permanent transitions to other land uses. It is not possible at this stage to specify exactly when, and where that landuse change will occur within the consented land areas. Any change in land use, to a less water demanding crop, will also allow a greater area of land to benefit from irrigation. This will increase the positive effects arising from the water take, while any effects will be addressed in the land use consent. The precise wording of these conditions will need to be further discussed with Council.

As an outcome of the modelling undertaken by Aqualinc, some adjustments to the rates of take of Tranche 2 groundwater for irrigation are also required to optimise the balance between irrigation and augmentation water use and mitigate effects on rivers during low-flow periods. For all but one applicant, the rates are proposed to be less than the rate originally applied for. For Tuki Tuki Awa, it is proposed that their original rate of take be increased (from 78 L/s daily average to 94 L/s daily average). This small increased take on one site is predicted to have negligible effects, as demonstrated in this AEE.

A consent duration of 20 years is sought for all of the applications. For consistency, and given the collaborative approach being applied, it is requested that a 20-year consent duration be applied to <u>each</u> of the eight applications.

This AEE is prepared in accordance with the requirements of Section 88 and the Fourth Schedule of the Resource Management Act 1991 (RMA), and it is intended to provide the information necessary to fully understand the proposal and any actual or potential effects that the proposed activity may have on the environment.

This AEE concludes that with the imposition of appropriate consent conditions, any actual and potential adverse environmental effects of the proposed groundwater takes will be negligible or no more than minor, with the exception of the security of supply of groundwater to 4 existing bores within the Ruataniwha Basin which may be adversely affected to a minor extent. To the extent that some form of

notification of this application is considered appropriate, then any such notification should be limited to those affected bore owners/users.

The proposed activity is consistent with the purpose and principles of the RMA.

In terms of section 104(1)(b), on the basis of the above assessment, the proposal is generally consistent with, and is not contrary to, the relevant objectives and policies of the National Policy Statement for Freshwater 2020, and the Hawke's Bay Regional Resource Management Plan (including the Regional Policy Statement).

Rule TT4 of the RRMP, as notified (as part of PC6), did not include provision for Tranche 2 groundwater takes and Rule TT4 became operative on 1 October 2015. Three of the applications were lodged prior to 1 October 2015. Regardless of this, based on legal advice received as to the effect of s 88A, Resource Management Act 1991 in circumstances where a previous rule becomes inoperative after lodgment, we consider that all eight applications should be assessed as Discretionary Activities. However, in the event that Non-Complying Activity status is required for the three applications lodged earlier, we consider that there is no difficulty in those applications passing both of the 'gateway' tests under s104D(1)(a) and (b) of the RMA.

Given the above, consent can be granted to all of the applications pursuant to sections 104, 104B, and 107, and, if necessary, s 104D, of the RMA, subject to the imposition of conditions under section 108 of the RMA (as proposed in Appendix F of this report).

APPENDIX A – Agreed Interpretation of RRMP Policies

TRANCHE 2 TAKES - PLAN CHANGE 6 'POLICY QUESTIONS'

Notes of meeting held on 10 April 2018 at Hawke's Bay Regional Council Offices

Present: Malcolm Miller (Consents Manager, Hawke's Bay Regional Council

Paul Barrett (Senior Planner, Hawke's Bay Regional Council) Rob van Voorthuysen (van Voorthuysen Environmental Ltd)

Susan Rabbitte (Lattey Group)

Janeen Kydd-Smith (Sage Planning HB Limited)

The purpose of the meeting was to reach agreement between those present to a number of questions (outlined below) relating to the implementation of the Regional Plan policies for taking Tranche 2 groundwater that would assist consideration of the applications going forward.

Relevant Regional Plan Policies

Policy TT8(1)(ca)

Enabling additional groundwater to be abstracted as a discretionary activity (Table 5.9.5 Tranche 2) provided that river flows are augmented to maintain the relevant minimum flows specified in Table 5.9.3 commensurate to the scale of effect of the Tranche 2 groundwater take.

Policy TT14(e), TT14(fa) and TT14(fc) and TT14(h)

- (e) New takes within the table 5.9.4, 5.9.5 or 5.9.6 Allocation Limited and complying with the minimum flow regime shall be a Discretionary Activity.
- (fa) Except as provided for in (a) to (f) above, takes (including those that do not comply with the minimum flow regime), shall be Non-complying Activities.
- (fc) For takes granted under (e) to (fa) above the consent duration shall be no more than 20 years.

Rule TT4(c)

No new groundwater takes from Groundwater Allocation Zones 2 and 3 utilising Tranche 2 groundwater may be exercised under this rule unless and until augmentation flows are discharged that are commensurate to the scale of effect of the proposed take, during the same irrigation season as the Tranche 2 groundwater takes are exercised, to each of the Waipawa River and the Upper Tukituki River or one or more of their respective tributaries at a rate of up to 715 l/s to each river catchment at the highest practicable elevation as required to maintain the relevant downstream minimum flows specified in Table 5.9.3.

Questions:

Responses to the questions, as agreed by the parties present at the meeting, are shown in red below.

1) Are some of the applications non-complying activities because Rule TT4 as notified did not include provision for Tranche 2 takes. The RRMP (with Rule TT4(c)) became operative on 1 October 2015.

Yes – some applications will be non-complying activities depending on the date they were lodged. To be granted these applications will need to meet the s104D 'gateway test'. HBRC will advise which of the applications are non-complying. Agreed.

2) Does "river flows" include both tributaries and mainstems of the Tukituki and Waipawa Rivers?

Yes, tributaries included, and a distributed affect of the Tranche 2 takes on surface water flows is likely. Agreed.

- 3) Where should the augmentation discharges occur, for example?
 - As near as possible to the groundwater take?
 Not necessarily a regime where augmentation occurs away from the site of take could be possible/preferable, but implementation also needs to be practicable.
 - In the nearest tributary that is permanently flowing?
 Yes, that is sensible and preferable as greater benefits to surface water flows would accrue from that as opposed to discharging augmentation flows above a drying reach.
 - Not upstream of a 'drying reach'?
 Preferable ., but may not be practicable.

A geographic spread of augmentation may be preferable, as long as this does not cause excessive costs to individual applicants or cost inequities across applicants.

4) Are the augmentation discharges required to be of a rate that ensures that the relevant minimum flows specified in Table 5.9.3 are not breached, or

No, given direction of POL TT8(ca)

5) Is each individual augmentation discharge required to be of a rate that offsets (equates to) the assessed depletion effect of each respective Tranche 2 take on the surface water flows in the Ruataniwha basin?

Yes

- 6) Are the augmentation discharges required to commence when the Table 5.9.3 minimum flows are reached on the relevant river with respect to each Tranche 2 take ("relevant river" being the river most directly affected by the Tranche 2 take), or
- 7) Are the augmentation discharges required to commence <u>prior</u> to exercising the new groundwater takes? Refer to Rule TT4(c) which states: "No new groundwater takes [...] may be exercised under this rule unless and until augmentation flows are discharged [...]."
- 8) Should Red Bridge also be considered a relevant downstream minimum flow site?
- 9) Are the augmentation discharges required to commence before the Table 5.9.3 minimum flows are reached on a relevant river with respect to each Tranche 2 take?

- 10) If (9) applies, what factors should be taken into account to determine when the augmentation discharges should commence, for example?
 - Effects on existing surface water takes (reliability of supply)? High flow takes not specifically considered?
 - Effects on POL TT10 high flow takes (those that existed at the time the application for Tranche 2 was lodged)?
 - Temporal effects on when minimum flows may be breached?
 - Lag time (time it takes for the effects of a Tranche 2 take to manifest in surface water)

Augmentation should commence in advance of the minimum flow occurring, and this should be a simple trigger level, that is the estimated rate of stream depletion above the relevant minimum flow. At Red Bridge, this will be the effect of the full Tranche 2 allocation above the red bridge minimum flow level (e.g. 650 L/s or similar). Will this be proportional based on the level of allocation if T2 is not fully allocated? The trigger level will need to be developed at each other minimum flow site based on the scale of effects/abstraction (or some other proportional approach e.g. catchment area) above these sites. The relevant recession curves (or similar) could be used to estimate the duration of augmentation requirement between the trigger and minimum flows to permit calculation of the appropriate augmentation volumes to be applied to each consent.

This approach would mean that effects on existing surface water user are avoided. As the full effects of the Tranche 2 takes on surface flows will build up over time (years), the above approach to the use of trigger levels may benefit surface water flows in the short to medium term (namely the rate of augmentation will exceed the level of induced stream depletion), and therefore also existing surface water abstractions where these occur in streams or rivers that are augmented.

- 11) How long should each augmentation discharge occur for:
 - Only when the relevant Tranche 2 take is occurring? No
 - Before the Tranche 2 take commences? No
 - For the period of time when a relevant minimum flow (or some other trigger level see 9 above) is breached? yes
 - For the whole 'irrigation season'? no
 - For the length of time that effects from a take will continue to occur, including possible inter-seasonal effects? no

Augmentation should occur when the river flows are below the trigger levels described above, within the irrigation season (November to April inclusive), and until the augmentation volume provided for by the consent is fully expended.

It is noted that once the augmentation volume is expended, the irrigation volume should also be fully utilised, and that irrigation could not continue to occur after the augmentation is fully expended.

The length of the augmentation period (low flow period + stream depletion effect) determines how much augmentation volume is required (i.e. augmentation requirement x no. of days per

season augmentation is required). This should be approached consistently so that an equitable allocation is achieved. Irrigation demand is determined on a 1 in 10 year basis (see RPS Policy 32). The augmentation volume could be determined on the same basis. [HBRC to look at doing this analysis].

12) Taking into account OBJ TT1 and POL TT11(1)(a) [which excludes takes deeper than 50m from direct stream depletion effects including those on wetlands] do any other effects need to be taken into account when determining the above matters (e.g. aquatic effects)?

Maybe, if specific high value reaches could be identified, and the effect of these estimated and considered.

This might be best considered on an individual basis, and could be a factor that helps determine the best augmentation location. Other mitigation could potentially be considered (e.g. riparian planting/shading).

13) Should each groundwater take for irrigation be assigned a maximum rate of take and a seasonal volume?

Yes – as per Pol TT14. Augmentation rate and volumes should also be specified.

14) Having regard to POL TT14(e) and (fc) what consent duration is appropriate?

20 years

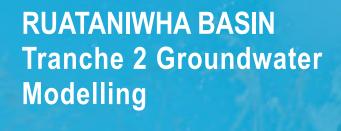
15) Do the effects need to be modelled in relation to takes occurring over the appropriate consent duration?

Yes

APPENDIX B – Aqualinc 2021 Groundwater Modelling Report



Groundwater REPORT





WL18045 19/08/2021

PREPARED BY Julian Weir

aqualinc.com



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TABLE OF CONTENTS

Exe	cutive	Summary	
1	Intro	oduction and Background	3
	1.1	Collaborative Approach	3
2	Mod	lel Overview and Updates	4
	2.1	Model Calibration	5
		2.1.1 Calibration to Groundwater Levels	5
	2.2	Calibration to River Flows	9
	2.3	Calibrated Model Parameters	11
3	Sce	narios	12
	3.1	Scenario Modelling and Augmentation Rationale	12
	3.2	Baseline Scenario	13
	3.3	Te Awahohonu Forest Trust (TAFT)	15
		3.3.1 Irrigation Demand	
		3.3.2 Modelling Results for TAFT	16
	3.4	Papawai Partnership	16
		3.4.1 Irrigation Demand	17
		3.4.2 Modelling Results for Papawai Partnership	
	3.5	Tuki Tuki Awa	18
		3.5.1 Irrigation Demand	
		3.5.2 Modelling Results for Tuki Tuki Awa	
	3.6	Plantation Road Dairies	
		3.6.1 Irrigation Demand	
	0.7	3.6.2 Modelling Results for Plantation Road Dairies	
	3.7	Springhill Dairies	
		3.7.1 Irrigation Demand	
	3.8	I&P Farming	
		3.8.1 Irrigation Demand	
		3.8.2 Modelling Results for I&P Farming	
	3.9	Buchanan Trust No 2 (Buchanan)	26
		3.9.1 Irrigation Demand	27
		3.9.2 Modelling Results for Buchanan	27
	3.10	Purunui Trust	28
		3.10.1 Irrigation Demand	29

	3.10.2	Modelling Results for Purunui Trust	29
3.1	I1 Augmer	ntation Scenarios: All Takes Combined Plus Augmentation	31
	3.11.1	Summary of Individual Effects	31
	3.11.2	Restrictions	31
	3.11.3	Augmentation Scenario 1	32
	3.11.4	Augmentation Scenario 2	36
	3.11.5	Augmentation Scenario 3	
	3.11.6	Augmentation Scenario 4	
	3.11.7	Temporal Response in Calibration Wells	
3.1	12 Practica	Il Implications	46
	3.12.1	Staged Development and Transitional Implementation	46
	3.12.2	Automated Monitoring of River Flows	47
	3.12.3	'Water Year' Definition and Associated Start Dates of Irrigation and Augmentation	47
3.1	13 Summa	ry and Recommendations	48
Referen	nces		50
Referen			. 00
Appendix	A : Groundwa	ater level calibration plots	51
Appendix	B : River flow	v calibration plots	60
Appendix	C : Calibrate	d Aquifer Parameters	61
Appendix	D : Groundw	ater level hydrographs for different scenarios	66
Table 1:		er level objective function values and other statistics for the calibrated model	
Table 2:	Measured a	and modelled river flow statistics	11
Table 3:	TAFT's prop	posed Tranche 2 infrastructure	15
Table 4:		nanges in flow statistics (over the period 1972-2012) compared to status quo due to TAFT's ke alone (no augmentation)	16
Table 5:	Papawai Pa	artnership's proposed Tranche 2 infrastructure	17
Table 6:		nanges in flow statistics (over the period 1972-2012) compared to status quo due to Papawa 's irrigation take alone (no augmentation)	
Table 7:	•	wa's proposed Tranche 2 infrastructure	
Table 8:	Modelled ch	nanges in flow statistics (over the period 1972-2012) compared to status quo due to Tuki Tuk tion take alone (no augmentation)	ki
Table 9:	_	Road Dairies proposed Tranche 2 infrastructure	
Table 10:	Modelled ch	nanges in flow statistics (over the period 1972-2012) compared to status quo due to Plantations' irrigation take alone (no augmentation)	on
Table 11:		airies' proposed infrastructure	
		nanges in flow statistics (over the period 1972-2012) compared to status quo due to Springhi	
I able 12.		ranges in now statistics (over the pendu 1972-2012) compared to status quo due to Springhi pation take alone (no augmentation)	III 24

Table 13:	I&P Farming's proposed infrastructure	25
Table 14:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to I&P Farming's irrigation take alone (no augmentation)	26
Table 15:	Buchanan's proposed infrastructure	27
Table 16:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Buchanar irrigation take alone (no augmentation)	
Table 17:	Purunui Trust's proposed Tranche 2 infrastructure	29
Table 18:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Purunui Trust's irrigation take alone (no augmentation)	30
Table 19:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due for all proposed Tranche 2 irrigation takes (no augmentation)	31
Table 20:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 1	32
Table 21:	Calculated seasonal augmentation volumes for augmentation scenario 1	33
Table 22:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 2	36
Table 23:	Calculated seasonal augmentation volumes for augmentation scenario 2	36
Table 24:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 3	39
Table 25:	Calculated seasonal augmentation volumes for augmentation scenario 3	40
Table 26:	Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 4	43
Table 27:	Calculated seasonal augmentation volumes for augmentation scenario 4	43
Table 28:	Optimised areas of irrigated pasture and Tranche 2 groundwater volumes (reproduced from Table 27)	49
Table 29:	Modelled changes in 7-day MALF (over the period 1972-2012) compared to status quo due to all applicar irrigation takes with optimised augmentation (scenario 4)	
Figure 1:	Histogram of annual rainfall totals for Ongaonga	4
Figure 2:	Study area with model extent shown by the shaded polygon (reproduced from Weir, 2013)	6
Figure 3:	Location of bores used for groundwater level calibration	7
Figure 4:	Modelled versus measured groundwater levels	8
Figure 5:	Location of river flow calibration sites	10
Figure 6:	Tranche 2 applicants' farm locations for T2 water, proposed augmentation discharge locations, and key river flow monitoring sites	14
Figure 7:	TAFT's proposed bore and augmentation locations	15
Figure 8:	Papawai Partnership's existing bores and proposed and augmentation location	17
Figure 9:	Tuki Tuki Awa's proposed bores and augmentation locations	19
Figure 10:	Plantation Road Dairies existing and proposed bores and augmentation locations	21
Figure 11:	Springhill Dairies' bores and augmentation locations	23
Figure 12.	I&P Farming's bore and augmentation locations	25

Figure 13:	Buchanan's bores and augmentation locations
Figure 14:	Purunui Trust's proposed bore and augmentation locations
•	Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 1
•	Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 2
•	Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 3
Figure 18:	Example of hydraulic timing of modelled river flows in the Tukipo River at Ashcott Rd
•	Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 4

EXECUTIVE SUMMARY

In 2013, Aqualinc Research Ltd (Aqualinc) developed a three-dimensional numerical flow model of the Ruataniwha basin as part of the Tukituki catchment Plan Change 6 (PC6) hearing. Recently, the model has been updated and used to test the hydraulic response of the groundwater and surface water system in the basin from multiple proposed Tranche 2 groundwater take applications. This work has been completed in a collaborative environment with Hawke's Bay Regional Council (HBRC) and eight Tranche 2 groundwater applicants.

The Tranche 2 groundwater applicants propose to abstract groundwater, and (as required under PC6) mitigate the consequential stream depletion effects via further abstraction to directly augment river flows during drier periods (when takes from the rivers are restricted). The concept of augmentation is based on using groundwater stored in the aquifer system during wetter periods to mitigate stream depletion effects during drier periods. The augmentation takes will also result in additional stream depletion effects, but this too will be delayed and spread over space and time through the storage response of the aquifer system. While there is on average more water abstracted from the basin's aquifer system (compared to current), the rationale is founded on the principle of using the groundwater system to smooth, buffer and delay stream depletion effects using groundwater storage that is later replenished naturally.

Since the original model documented in Weir (2013), the model has been updated with new aquifer test data received from HBRC. The model was then recalibrated to measured groundwater levels and river flows, with a particular focus on matching low (dry-period) river flows to align with HBRC's water management strategies. Good matches were achieved between measured and modelled outputs. The model simulates the period 1972-2012, which incorporates a wide range of climatic variability from very wet to very dry years.

The updated numerical model has been used to quantify the stream depletion effects of the proposed Tranche 2 groundwater takes and the subsequent surface water augmentation requirements. This is not an exact science as there are several variables that need to be assumed or approximated. However, the modelling work has provided a realistic quantification of the magnitude, location and timing of effect.

Several scenarios of abstraction and augmentation were modelled to arrive at an 'optimised' solution. This includes:

- Irrigated areas and augmentation rates a little smaller than applied for by some applicants;
- Adopting higher river low-flow restrictions (than those set out in Table 5.9.6 of HRBC's Regional Resource Management Plan) to provide improved environmental low flows and to protect existing users' reliability;
- Full augmentation whenever these higher river flow restrictions are triggered, regardless of whether or not Tranche 2 irrigation is occurring; and
- Full augmentation occurring from all Tranche 2 applicants when any one of the flow monitoring sites within the basin is triggered; this acknowledges that all Tranche 2 applicants are operating collaboratively, and effects from any one take can propagate across several streams. The exception to this is Tuki Tuki Awa who propose to take Tranche 2 groundwater and augment to the Tukituki River only when their existing surface water take is restricted.

The numerical model domain encompasses the Ruataniwha basin, and does not extend to HBRC's flow monitoring site on the Tukituki River at Red Bridge, down catchment. Therefore, the flow monitoring sites on the Waipawa at SH2 and Tukituki at Tapairu Road have been considered to represent this site. So long as the combined 7-day MALF at the SH2 and Tapairu Road sites are maintained (or improved), then there will be no adverse downstream effects on low flows at Red Bridge as a result of the proposed Tranche 2 takes.

Summary of Effects

Proposed low-flow triggers and net changes in 7-day MALF for key river low monitoring sites under the optimised scenario are summarised in the table below.

Site							
Waipawa at SH2	Tukituki at Tukipo at Tukipo at Tapairu Rd SH50 Ashcott Rd			Mangaonuku u/s Waipawa			
RRMP Table 5.9.6 minimum flow (I/s)							
2,500 2,300 150 1,043 1,17				1,170			
	Assumed low-flow restriction applied (I/s) (1)						
2,725	2,360	155	1,085	1,295			
	Change in mean 7	7-day annual low	flow (MALF) (I/s)				
+3	+3 +135 -1 +1 +2						
Change in 7-day MALF as a percentage of low-flow limit							
+0.1%	+5.7%	-0.6%	+0.1%	+0.2%			
(1) Those are highe	or than current PRMI	2 Table 5 0 2 limite t	o provido greater en	vironmontal			

⁽¹⁾ These are higher than current RRMP Table 5.9.3 limits to provide greater environmental benefit during low flows and to protect reliability of existing users.

The small residual negative change to 7-day MALF flow in the Tukipo River is smaller than both model uncertainty and measurement precision.

The following key findings have been derived from the modelled scenarios:

- The effects on river flows from the proposed additional Tranche 2 takes are spread over both space and time throughout the basin.
- If augmentation is applied when flow sites reach the higher flow restrictions (discussed above), regardless of whether or not Tranche 2 irrigation is operating, augmentation results in an overall improvement in low flows exiting the basin in both the Tukituki and Waipawa rivers. As noted above, the exception to this is Tuki Tuki Awa who propose to take Tranche 2 groundwater and augment to the Tukituki River only when their existing surface water take is restricted. This also does not adversely affect low river flows.
- Targeted over-compensation (i.e. the positive changes in MALF) accommodates modelling prediction uncertainty, enhances environmental low-flows, and accommodates across-catchment effects.
- Groundwater will not be mined. While groundwater levels will lower further with the additional Tranche 2 takes, they will not continue to lower; they will simply reach a new (lower) dynamic equilibrium. Shallow groundwater levels are predicted to lower a maximum of 0.8 m in the vicinity of the Tranche 2 take locations, and less than 0.3 m further afield.

Several management, operational and compliance protocols are recommended to aid practical implementations of the Tranche 2 takes and augmentation, including:

- Staged development and transitional implementation;
- · Automated monitoring of river flows; and
- Defining a 'Water Year' and associated start date of irrigation and augmentation.

INTRODUCTION AND BACKGROUND

In 2013, Aqualinc Research Ltd (Aqualinc) developed a three-dimensional numerical flow model of the Ruataniwha basin as part of the Tukituki catchment Plan Change 6 (PC6) hearing. Recently, the model has been updated and used to test the hydraulic response of the groundwater and surface water system in the basin from multiple proposed Tranche 2 groundwater take applications.

The Tranche 2 groundwater applicants propose to abstract groundwater (primarily for irrigation use), and (as required under PC6) mitigate the consequential stream depletion effects via further abstraction to directly augment river flows during drier periods (when takes from the rivers are restricted). The concept of augmentation is based on using groundwater stored in the aquifer system during wetter periods to mitigate stream depletion effects during drier periods. The augmentation takes will also result in additional stream depletion effects, but this too will be delayed and spread over space and time through the storage response of the aquifer system. While there is on average more water abstracted from the basin's aquifer system (compared to current), the rationale is founded on the principle of using the groundwater system to smooth, buffer and delay stream depletion effects using groundwater storage that is later replenished naturally.

The numerical model has been used to quantify the stream depletion effects of the proposed Tranche 2 takes and the subsequent surface water augmentation requirements. This is not an exact science as there are several variables that need to be assumed or approximated. However, the modelling work has provided a realistic quantification of the magnitude, location and timing of effect.

To reduce the influence of measurement and model uncertainty, the most appropriate application of model results is to consider *changes* in key outputs (river flows and groundwater levels) rather than absolute values. In this regard, the model tests the effectiveness of the proposed augmentation to mitigate the changes in river flows that would be induced by the Tranche 2 takes. The modelled changes can be compared to measured values to derive absolute values, if required.

The original model development and calibration is documented in Weir (2013), much of which is not reproduced herein. The reader is referred to this publication for detail. More recently, the model has been updated with new field data, and used to run various scenarios to test the hydraulic response of the proposed Tranche 2 takes and the ability to mitigate adverse effects on surface water during low-flow periods through augmentation. Model updates, scenarios and results are documented below.

1.1 Collaborative Approach

This work has been completed in a collaborative environment with Hawke's Bay Regional Council (HBRC) and the following eight Tranche 2 groundwater applicants:

- Te Awahohonu Forest Trust (TAFT)
- Papawai Partnership
- Tukituki Awa
- Plantation Road Dairies
- Springhill Dairies (formerly Ingleton Farms)
- I & P Farming (formerly Abernethy Partnership)
- Buchanan Trust No 2
- Purunui Trust

TAFT have helpfully provided overall coordination and leadership from the perspective of the Tranche 2 Applicants, and further planning advice and support have been provided by Sage Planning, vVEnvironmental and other consultants working for some of the above applicants.

The original Ruataniwha basin flow model was developed as a MODFLOW-NWT model (Niswonger *et al*, 2011) with a 200 x 200 m size grid. It includes the main rivers and streams in the basin, land surface recharge (both dryland and irrigated) and groundwater pumping. The study area is shown in Figure 2.

The model runs with daily stress periods from 1 July 1972 to 30 June 2012. It has been calibrated against transient groundwater levels and river flows within this time span, with a particular focus on matching low (dry-period) river flows to align with HBRC's water management strategies. The graphical user interface GMS (2019) has been used to generate the MODFLOW-NWT input files, and to post-process and visualise the outputs.

Since the original model documented in Weir (2013), a review of aquifer tests in the basin has been received from HBRC following a review by PDP (2018). Aquifer test results from this work have been incorporated into the model, specifically horizontal hydraulic conductivity, vertical hydraulic conductivity and aquifer storage parameters. After this new data was integrated into the model, it was recalibrated to measured groundwater levels and river flows (again, with a particular focus on matching low river flows).

Due to the length of time required to generate new model inputs, the model run period was not changed (i.e. not beyond 2012). However, the current model run period (1972-2012) incorporates a wide range of climatic variability from very wet to very dry years. This is demonstrated in Figure 1 which presents a histogram of annual rainfall totals (for each calendar year) for Ongaonga over the period 1970-2019. The range of annual rainfall totals over the simulation period (1972-2012) is also shown on this figure, and covers the full range on record, apart from the wettest year (this was 1971). Hence, extreme dry years (when the groundwater and surface water systems are most stressed) are captured in the simulation period, and therefore there was no need to re-run the model to include post-2012 climate data.

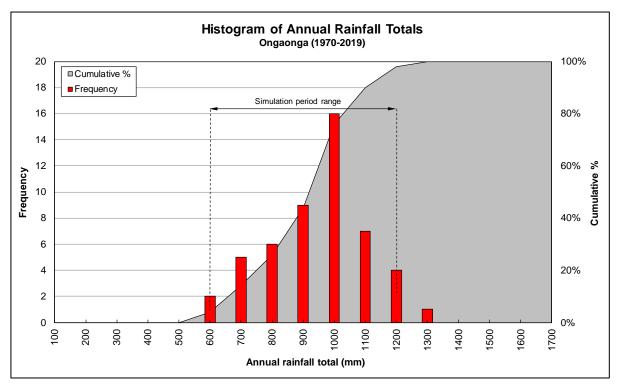


Figure 1: Histogram of annual rainfall totals for Ongaonga (data sourced from NIWA's CliFlow database)

2.1 Model Calibration

To ensure the model appropriately represents reality, the model was calibrated against measured data. This was achieved using a combination of manual trial-and-error methods and automated calibration using the software PEST (Doherty, 2010). Calibration focussed on two key datasets: groundwater levels and river flows. These are discussed in the following paragraphs. The calibration process is similar to that documented in Weir (2013).

2.1.1 Calibration to Groundwater Levels

During calibration, the modelled groundwater levels in a bore are compared to measured groundwater levels in the same bore, for all of the calibration bores. The difference between the modelled and measured groundwater level is called the 'residual'. If the residuals are all generally positive or negative, then the system is said to be biased. One of the goals during model calibration is to minimise the residuals and bias.

Groundwater level calibration was based on groundwater level measurements (supplied by HBRC) from 30 monitoring bores within the basin (shown in Figure 3). Initially a steady-state version of the Ruataniwha Basin groundwater model was run using average model inputs and outputs, and this was calibrated against average measured groundwater levels in the calibration bores.

Once suitable steady state calibration had been achieved, a transient model was then run. The transient simulation period spans 1 July 1972 through to 30 June 2012 (40 years). The model was calibrated against any measurements available within this period.

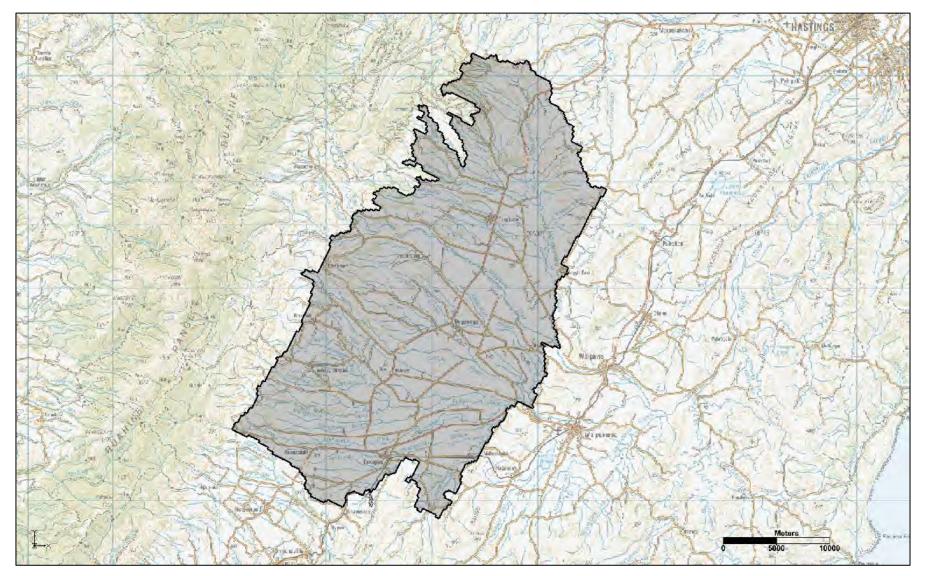


Figure 2: Study area with model extent shown by the shaded polygon (reproduced from Weir, 2013)

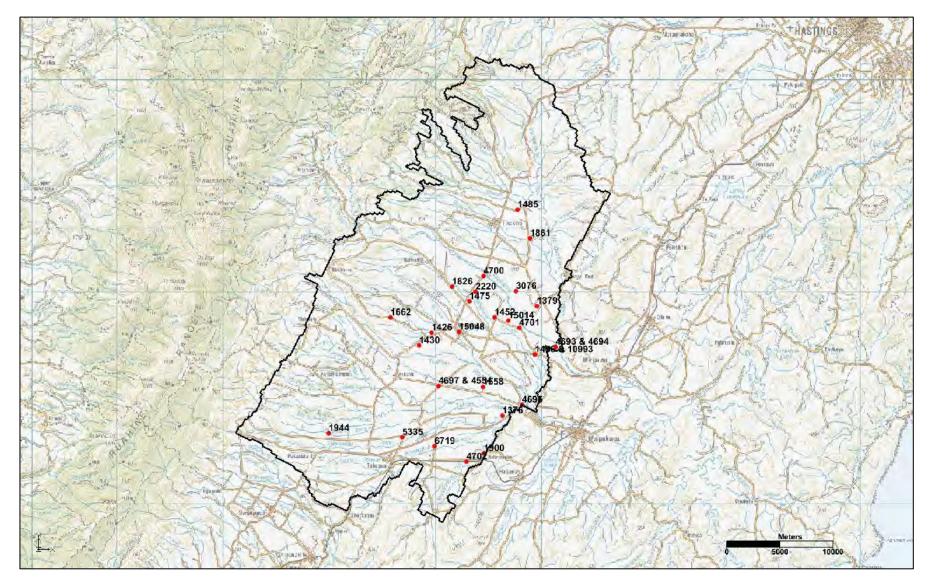


Figure 3: Location of bores used for groundwater level calibration

For groundwater level calibration, initially a 'No Irrigation' model scenario was used to simulate the groundwater level response under natural conditions. Therefore, calibration focussed on matching groundwater levels that were measured during periods that were unaffected by groundwater pumping (i.e. winter levels). The effects of groundwater abstraction were then verified against a model scenario that simulates the current level of groundwater abstraction (with no Tranche 2 takes occurring) with comparisons focussed on more recent groundwater level measurements that are affected by groundwater pumping.

The model calibrated very well. Plots of measured and modelled groundwater levels in the 30 monitoring bores are presented in Appendix A. The modelled groundwater level shown on these plots is for the full 40 year simulation period. Most bores have a plotted vertical axis range of 20 m so that a relative comparison between bores can be visualised.

Groundwater level measurements from bores 4697 and 5445 both show variations of over 30 m within each season. This appears to be a local phenomenon resulting from pumping in the bore or from another nearby bore. The model is a catchment-scale representation and has not been constructed to simulate these local effects. Hence, differences between measured and modelled groundwater levels are large for these bores. This does not jeopardise the model's ability to predict regional-scale effects.

Figure 4 presents a plot of measured versus modelled groundwater levels for the 'No Irrigation' scenario. For a model perfectly calibrated at every observation bore, all points would lie exactly along the dashed line running diagonally though the plot. The amount of scatter either side of this line provides an indication of the goodness of fit. Some scatter around this line is normal for any model that simplifies a complex real-world system. The scatter occurs as a result of measurement error and model structural uncertainty. The pumping effects in bores 4697 and 5445 (discussed in the above paragraph) are labelled on this plot.

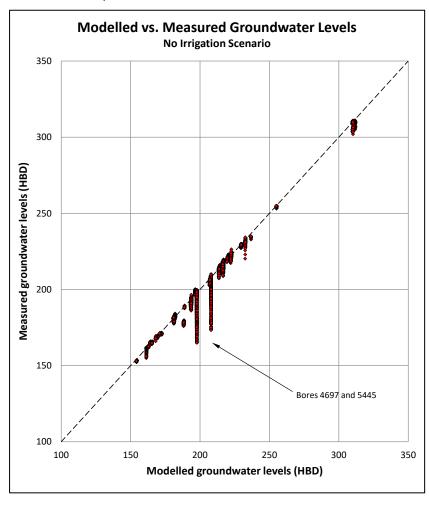


Figure 4: Modelled versus measured groundwater levels

An objective function is a mathematical formula that expresses numerically the goal that is to be achieved during calibration. For the purpose of this model, two objective functions have been considered, as follows:

- a. The mean error (ME), which assists in showing the presence of bias (systematic error); and
- b. The root mean square error (RMSE), which is a classical measure of model error. The RMSE is usually reported as a percentage of the range within which the measured values vary (this is also referred to as the 'normalised' RMSE).

One of the key goals of model calibration is to minimise the objective functions. Table 1 presents the groundwater level objective function values for the entire 40 year model simulation period. Also in Table 1 are additional statistics that have been generated and a brief description of these statistics.

Table 1: Groundwater level objective function values and other statistics for the calibrated model

Objective function or statistic	Full simulation period (1972-2012)	Definition
Mean error (ME) (m)	0.74	Average difference between modelled and measured groundwater level.
Maximum absolute residual (m)	34.6	Maximum difference between modelled and measured groundwater level.
Minimum absolute residual (m)	0	Minimum difference between modelled and measured groundwater level.
Root mean square error (RMSE) (m)	3.4	Classical measure of error.
Normalised RMSE	2.2%	RMSE normalised by the amount that the measured values vary during the simulation period.
R ² (square of the correlation coefficient)	0.98	Correlation between measured and modelled values.

The United States Army Corps of Engineers use a rule of thumb for an acceptable normalised RMSE of 10% when considering groundwater flow calibration or verification (Donnell *et al.*, 2004). The 2001 Australian Groundwater Flow Modelling Guidelines indicate that the normalised RMSE should be less than 5% (MDBC, 2001) though the revised 2012 guidelines (Barnett *et al.*) suggest that alternative (larger) values may be acceptable depending on the model scope. The normalised RMSE is 2.2% for the calibrated model (Table 1). Based on these statistics, the transient model is suitably calibrated.

Model mass balance errors are less than 0.6% for all time steps and average approximately 0.01% over the full 40 year simulation. This implies that the model has sufficiently accurately accounted for all water movements.

2.2 Calibration to River Flows

River flow calibration primarily focussed on matching low flows, as abstraction potentially causes the greatest stresses to the rivers at these times. In addition, HBRC's management regimes are typically based on low flow statistics (such as the 7-day MALF).

River flow calibration focussed on the Tukituki and Waipawa rivers as these are the two rivers that drain the Ruataniwha Basin and have the greatest length of record within the model simulation period. River flow calibration data was supplied by HBRC for the Tukituki River at Tapairu Road and Waipawa River at RDS/SH2. The flow sites are located 5-11 km downstream of the outlets from the Ruataniwha Basin, as shown in Figure 5. However, for the purposes of model calibration, it has been assumed that river gains and losses between the basin outlets and these flow monitoring sites are negligible (i.e. these sites represent the flows exiting the basin).

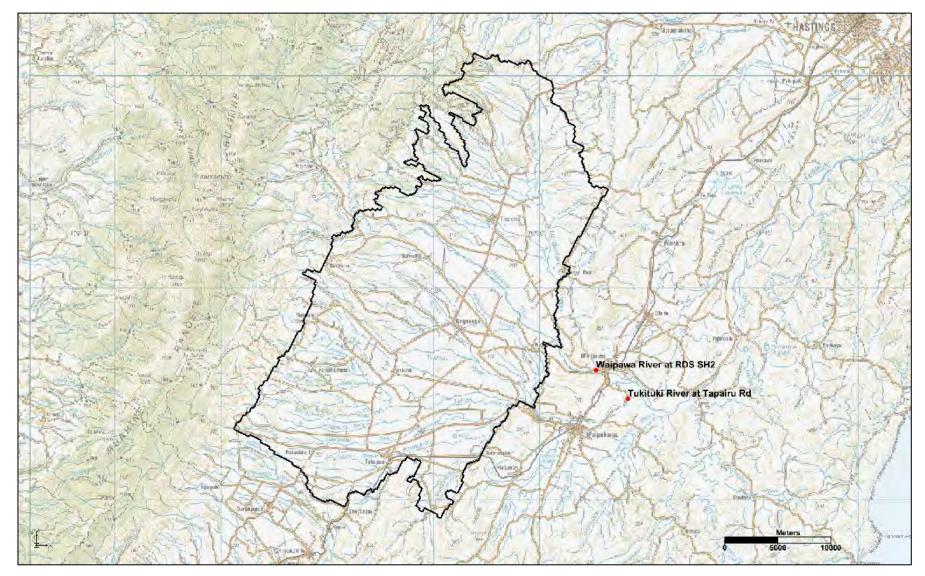


Figure 5: Location of river flow calibration sites

Appendix B presents hydrographs comparing measured and modelled flows for the calibration sites on the Tukituki and Waipawa rivers. Table 2 compares flow statistics for the different data sets.

Table 2: Measured and modelled river flow statistics

					Tapairu Rd RDS/SH2		
Data set	Time period	Average (m³/s)	MALF (m³/s)	Average (m³/s)	MALF (m³/s)		
Measured	April 1987- December 1990	17.03	2.65	16.89	3.09		
Modelled		17.87	2.81	16.05	3.13		
Modelled difference to measured		5%	6%	-5%	-1%		
Modelled	1972-2012	16.95	2.68	15.41	2.98		

Overall, modelled river flows are consistent with measured flows. Modelled flow statistics are within 6% of both the measured 7-day MALF and the measured average.

2.3 Calibrated Model Parameters

Aquifer horizontal conductivity, vertical conductivity and specific storage were assigned by interpolating between a series of points (called 'pilot points'). This resulted in spatially variable properties. Where available, aquifer test values were assigned individual fixed pilot points. Additional pilot points were then added between these values and varied throughout model calibration. River bed parameters were assigned as single values for each of 26 specific river reaches assigned throughout the model.

Specific storage (defined as the aquifer storage divided by the saturated thickness) for layer 1 was assigned separately to layers 2 and below to simulate the different hydraulic response of shallow unconfined/semi-confined aquifers compared to the deeper more confined layers.

Maps of horizontal hydraulic conductivity, vertical hydraulic conductivity and specific storages (for both layer 1 and also layers 2 and below) are shown in Appendix C.

River bed conductivities were varied for each calibration reach. Values varied between 0.0018 m/day and 5 m/day.

Subsurface flows under the two rivers at the outlets of the basin are relatively unknown but are expected to be small in comparison to river flows. The magnitude of these flows has been estimated by multiplying the groundwater gradient at these locations (based on nearby measured groundwater levels) with nearby aquifer transmissivities (reported by PDP, 2018), given the width of the groundwater opening. This yielded a total flow of approximately 0.06 m³/s combined through the two subsurface outlets. This equates to less than 0.2% of the average measured river flow.

The conductances of the general head boundaries at these outlets were adjusted so that the modelled subsurface flows over these boundaries approximately equalled the estimated flows.

3 SCENARIOS

The calibrated flow model has been used to run multiple scenarios of future takes to quantify the net change in river flows. The following scenarios have been simulated:

- Baseline: Status quo. This includes all existing takes, but no proposed Tranche 2 takes.
- Scenarios 1-8: One model run for each of the eight proposed Tranche 2 applicants. These
 models are all founded on the Baseline scenario, but with each applicant's proposed full
 irrigation needs at their proposed location. The results of these scenarios (compared to the
 baseline) have then been used to determine the timing of the augmentation requirement (for
 mitigating stream depletion effects of the irrigation take alone) that would feed into subsequent
 augmentation scenarios.
- Augmentation Scenarios: Initially, a combined model scenario including all applicants' takes and with each applicant's augmentation operating. Then additional scenarios that balance environmental flows and irrigated areas.

Further details of these scenarios are provided below along with a brief discussion on each applicant's proposed activity and their water use. An overview of the applicants' farm locations, augmentation locations and key flow monitoring sites is provided in Figure 6.

3.1 Scenario Modelling and Augmentation Rationale

When the Tranche 2 applications were lodged, it was unclear how the Tukituki PC6 provisions should be interpreted and implemented. The reason being that the Tranche 2 regime was developed by the Tukituki Board of Inquiry and not by HBRC. For example, it was unclear whether or not the effects of the deep groundwater augmentation abstractions on surface flows would themselves need to be offset by further augmentation, and so on. Some applicants (including Te Awahohonu Forest Trust and Springhill Dairies) assumed that would be the case while others did not.

Consequently, the approach taken here is to focus on the total amount of Tranche 2 water sought by each applicant and the total irrigable area applied for.

Irrigation demand is then calculated for the irrigable areas applied for using Aqualinc's in-house soil-water balance model, IrriCalc. From these calculations, the 90 percentile (or 9 in 10 years) irrigation demand has been calculated. Once the irrigation demand is calculated, then the irrigation volume is deducted from the total volume applied for to yield a volume of water available for augmentation. Various scenarios are then modelled to assess the effects of each applicants' take (singularly and in combination with all other applicants) and the optimum timing for the commencement and duration of the augmentation takes. Consequently, the division between irrigation and augmentation use varies depending on the scenarios being modelled (discussed later), but the total volume of water taken will not exceed that originally applied for. However, in some cases the original irrigable area needed to be reduced to either enable sufficient water to be available for augmentation purposes (e.g. for Papawai Partnership) or to ensure that the total volume of Tranche 2 water available (15 million m³) was not exceeded (e.g. for Purunui).

The assessments described herein assume that all Tranche 2 water is used for new irrigation, except for Tuki Tuki Awa. If only some Tranche 2 water is used to fill in gaps when Tranche 1 water is unavailable, then the overall effects will be less than assessed: less Tranche 2 water will be taken for irrigation and augmentation will remain the same.

3.2 Baseline Scenario

The Baseline (Status Quo) scenario represents the currently irrigated area of approximately 6,000 ha within the basin. This is comprised of 4,025 ha from groundwater and 1,975 ha from surface water. Surface water takes are restricted based on the relevant low flow abstraction cessation rules in HBRC's Regional Resource Management Plan (RRMP). This scenario has been used as a baseline against which other scenarios have been compared to quantify changes in river flows and groundwater levels.

For simplicity, it has been assumed that all existing and future irrigated land use is pasture, which typically has a larger seasonal water demand than other land uses. Irrigation return water is realised as an increase in land surface recharge.

If land uses other than pasture are applied (e.g. cropping, or mixed pasture and cropping), then the seasonal water use and associated recharge will be less for the same irrigated area, or a larger area could be irrigated for the same seasonal volume. In these cases, the modelled effects on river flows will be either less or similar (respectively) than the assessments presented below.

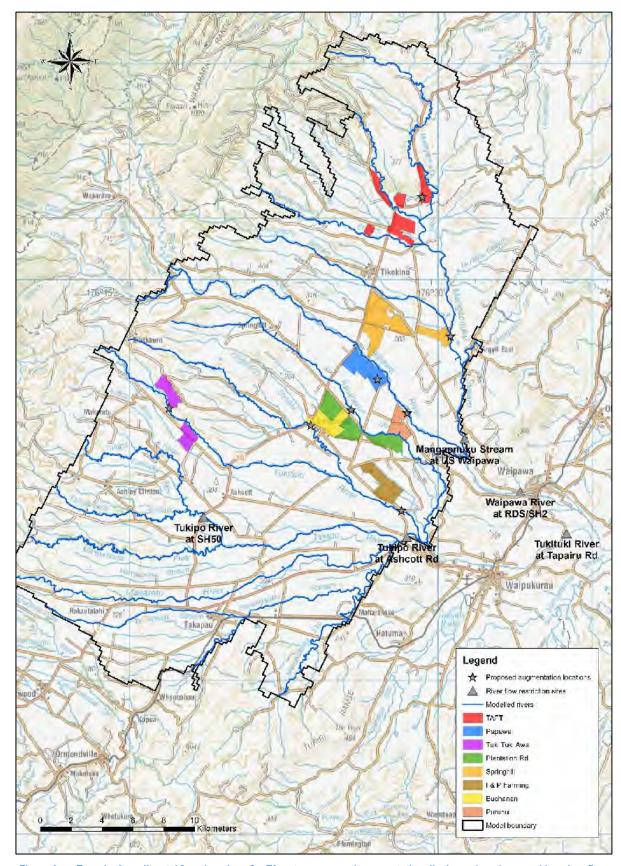


Figure 6: Tranche 2 applicants' farm locations for T2 water, proposed augmentation discharge locations, and key river flow monitoring sites

3.3 Te Awahohonu Forest Trust (TAFT)

Te Awahohonui Forest Trust (TAFT) have applied to take up to **4,914,920** m³/year of Tranche 2 groundwater from deep bores located around their Gwavas Station property, Tikokino. The proposed take is intended to irrigate 540 ha of pasture (assumed), or a larger area of less water-intensive crops or horticulture, and provide river augmentation to the Mangaonuku Stream.

The location of the bores (existing and proposed) from which TAFT have proposed to take groundwater, and the approximate locations of the augmentation take bore and the discharge site, are shown in Figure 7.

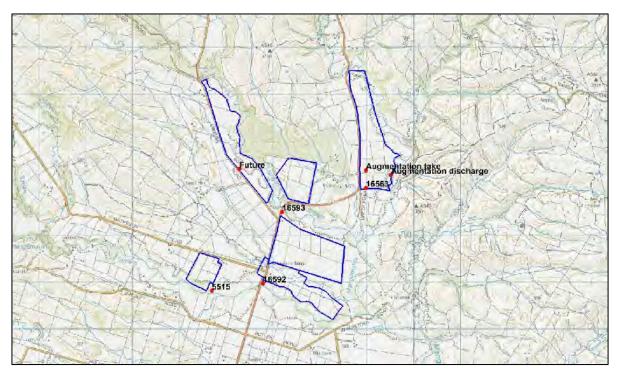


Figure 7: TAFT's proposed bore and augmentation locations (proposed irrigation areas are shown as blue outlines)

Key infrastructure for TAFT's proposed Tranche 2 take are summarised in Table 3.

Table 3: TAFT's proposed Tranche 2 infrastructure

Site	Use and/or status	Current status	Depth (m bgl)	Approx. irrigated area (ha)
Bore 16563	Irrigation	Existing	162.2	130
Bore 16592	Irrigation	Exploratory well	220.8	100
Bore 16593	Irrigation	Exploratory bore	222.3	110
Bore 5515	Irrigation	Existing	66.0	50
Future bore	Irrigation	Proposed	~ 200	150
Augmentation take	Augmentation take	Proposed	~160	-
Augmentation discharge	Discharge to Mangaonuku Stream	Proposed	-	-

3.3.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile water demand for pasture is calculated to be approximately 580 mm/year, which over an irrigated area of 540 ha equates to an annual volume of 3.1 million m³/year.

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The crop water demand has been modelled from the bores proposed to supply the property, as listed in Table 3, in proportion to the listed irrigated areas. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR for the proposed irrigated areas shown in Figure 7.

3.3.2 Modelling Results for TAFT

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites shown in Figure 6 have been processed to generate changes in flow statistics due to TAFT's irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 4. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 4: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to TAFT's irrigation take alone (no augmentation)

	Site							
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa			
	Change in average flow (I/s)							
TAFT's irrigation take only	-64	-18	0	-8	-35			
Change in mean 7-day annual low flow (MALF) (I/s)								
TAFT's irrigation take only	-48	-9	0	-6	-37			

From Table 4, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the Waipawa catchment and tributaries, as this is the catchment within which Gwavas Station is located. However, small scale effects do propagate over to the lower Tukituki catchment and tributaries.

Changes in flows allowing for all Tranche 2 applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.4 Papawai Partnership

Papawai Partnership have submitted two applications to take Tranche 2 groundwater. An original application was submitted to take up to 423,062 m³/year of groundwater, and a new application has recently been submitted to take up to 1,052,455 m³/year of groundwater. These combine to a total of 1,475,517 m³/year and comprises 1,161,000 m³/year for irrigation and 314,517 m³/year for augmentation. The applications seek to take groundwater from an existing deep bore (16508) located on their property, adjacent to the Waipawa River, and provide river augmentation to this river. The irrigation component of the proposed Tranche 2 take will supplement an existing consented take of 608,212 m³/year to provide adequate irrigation of 320 ha of pasture (assumed) or a larger area of less water-intensive crops or horticulture, from both bores 16508 and 1859 (combined).

Papawai Partnership have proposed to inject the augmentation water into an existing unused shallow bore located approximately 300 m from the Waipawa River. They have advised that they believe this shallow bore is directly connected to the nearby Waipawa River, though this will be confirmed prior to commencing augmentation. For the purpose of augmentation modelling, it will be assumed that the water will be discharge directly into the Waipawa River immediately adjacent to the shallow injection bore. Due to the fast hydraulic response between the Waipawa River and nearby shallow groundwater, this will make little difference to the modelled effects.

The location of existing bore 16508, from which Papawai Partnership have proposed to take the additional Tranche 2 groundwater, and the approximate location of the augmentation discharge site are shown in Figure 8. Also shown is the location of the other irrigation bore 1859.

Key infrastructure for Papawai Partnership's proposed Tranche 2 take are summarised in Table 5.

Table 5: Papawai Partnership's proposed Tranche 2 infrastructure

Site	Use and/or status	Current status	Depth (m bgl)	Approx. irrigated area (ha)
Bore 16508	Irrigation & augmentation take	Existing	119.6	160
Bore 1859	Irrigation	Existing	87.5	160
Augmentation discharge	Discharge to groundwater adjacent to Waipawa River	Existing	Unknown (shallow)	-

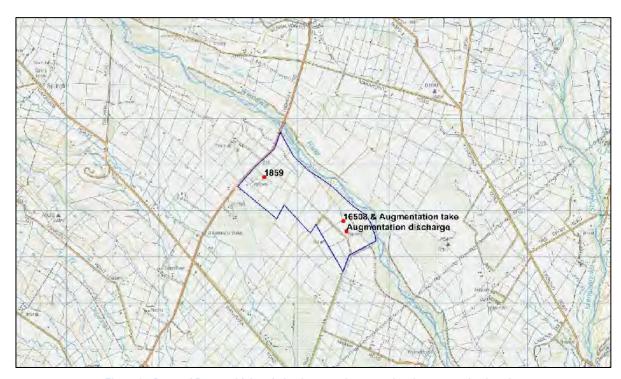


Figure 8: Papawai Partnership's existing bores and proposed and augmentation location (the total irrigated area is shown as a blue outline)

3.4.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile water demand for pasture is calculated to be approximately 560 mm/year, which over an irrigated area of 320 ha equates to an annual volume of 1.8 million m³/year. In combination with their existing consented take, this is consistent with the total volume of water sought by Papawai

Partnership. The Tranche 2 groundwater applied for is adequate to irrigate approximately 207 ha of pasture.

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The Tranche 2 crop water demand has been modelled from bores 1859 and 16508 (listed in Table 5), in proportion to the listed irrigated areas. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR.

3.4.2 Modelling Results for Papawai Partnership

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics due to Papawai Partnership's irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 6. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 6: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Papawai Partnership's irrigation take alone (no augmentation)

	Site						
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa		
	Change in average flow (I/s)						
Papawai's irrigation take only	-25	-8	0	-3	-9		
Change in mean 7-day annual low flow (MALF) (I/s)							
Papawai's irrigation take only	-18	-3	0	-2	-10		

From Table 6, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the mid-lower Waipawa catchment and tributaries, as this is the catchment within which Papawai Partnership is located. However, small scale effects do propagate over to the lower Tukituki catchment and tributaries.

Changes in flows allowing for all applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.5 Tuki Tuki Awa

Tuki Tuki Awa initially applied to take up to **952,400** m³/year of Tranche 2 groundwater from four proposed deep bores located on their southern irrigated property, adjacent to the Tukituki River. The irrigation component of the proposed Tranche 2 take is intended to replace existing surface supply for reliable and adequate irrigation of up to 136 ha of pasture (assumed) or a larger area of less water-intensive crops or horticulture.

Tuki Tuki Awa plan to maintain their surface water take and abstract only some of the proposed Tranche 2 groundwater to provide full reliability when their surface water take is on low-flow restrictions. However, the assessment described below has initially modelled a situation where all water is sourced from Tranche 2 groundwater with no change to the surface water take. This presents a worst-case scenario of effects. Any other combination of water supply from mixed sources (balancing surface and groundwater supplies) will result in smaller effects on river flows, and this is what is now proposed by Tuki Tui Awa. Therefore, a final scenario (Scenario 4, discussed later in this report) incorporates Tuki Tuki Awa's use of Tranche 2 water to only fill in the gaps in their existing surface water take when it is restricted.

Tuki Tuki Awa initially proposed to abstract Tranche 2 augmentation water from a series of new bores planned on their southern block, and discharge this directly into the Tukituki River adjacent to the block. The approximate location of the proposed new irrigation bores on the southern block, the proposed new augmentation abstraction bore, and the approximate location of the augmentation discharge site are shown in Figure 9. It is not proposed to irrigate the northern run-off block.

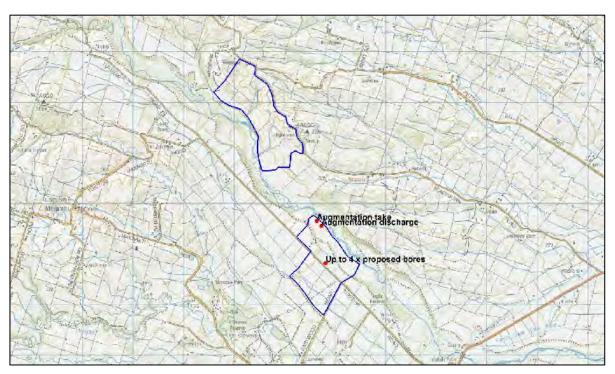


Figure 9: Tuki Tuki Awa's proposed bores and augmentation locations (the property boundaries are shown as blue outlines)

Key infrastructure for Tuki Tuki Awa's proposed Tranche 2 take are summarised in Table 7.

Table 7: Tuki Tuki Awa's proposed Tranche 2 infrastructure

Site	Use and/or status	Current status	Depth (m bgl)	Approx. irrigated area (ha)
Proposed bores x 4	Irrigation	Proposed	50 +	136
Augmentation take	Augmentation take	Proposed	50 +	-
Augmentation discharge	Discharge to Tukituki River	Proposed	-	-

3.5.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile water demand for pasture is calculated to be approximately 450 mm/year, which over an irrigated area of 136 ha equates to 612,000 m³/year. This is less than the volume of water sought by Tuki Tuki Awa for irrigation purposes (which was 882,800 m³/year) and suggests that not all of the water applied for would be used, except during extreme dry years. Alternatively, more Tranche 2 water could be used for augmentation than indicated in the application (if permitted).

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The Tranche 2 crop water demand has been modelled from the location of the proposed bores (as listed in Table 7). Although irrigation results in additional LSR compared to dryland, the proposed Tranche 2 groundwater is sought

to replace existing irrigation sourced from surface water. Hence, there will be no change to the irrigated area (and resulting LSR); the only change is from where the water is sourced.

3.5.2 Modelling Results for Tuki Tuki Awa

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics due to Tuki Tuki Awa's irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 8. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect. As discussed above, these modelled flow changes do not allow for a reduction in Tuki Tuki Awa's surface water take due to Tranche 2 groundwater substitution.

Table 8: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Tuki Tuki Awa's irrigation take alone (no augmentation)

	Site					
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa	
	Change in average flow (I/s)					
Tuki Tuki Awa's irrigation take only	-8	-11	-1	-7	-2	
Change in mean 7-day annual low flow (MALF) (l/s)						
Tuki Tuki Awa's irrigation take only	-4	-5	-1	-5	-2	

From Table 8, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the mid-lower Tukituki catchment and tributaries, as this is the catchment within which Tuki Tuki Awa is located. However, effects do propagate over to the lower Waipawa catchment and tributaries. The overall effects are relatively small compared to some other applicants because the total scale of the proposed Tranche 2 take is also relatively small.

Changes in flows allowing for all applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.6 Plantation Road Dairies

Plantation Road Dairies originally applied to take 6,000,000 m³/year of Tranche 2 groundwater. After their application was lodged, they changed their proposal and reduced the volume of groundwater sought. The 'released' Tranche 2 groundwater was therefore available to fulfil the unmet volume applied by the last person in the Tranche 2 applicant queue at the time, and then for additional allocation under new applications. The volume of Tranche 2 groundwater that Plantation Road Dairies have now applied for is 3,751,225 m³/year from deep bores around their property, located in the lower basin between the Waipawa and Tukituki rivers. Approximately 26% of the volume has been assigned to augmentation, though this may change (discussed later under different scenarios). The remaining irrigation component of the proposed Tranche 2 take (2,775,914 m³/year) will be used to irrigate up to 459 ha of pasture (assumed), or a larger area of less water-intensive crops or horticulture.

The location of the bores (existing and proposed), from which Plantation Road Dairies have proposed to take Tranche 2 groundwater, are shown in Figure 10. The proposed augmentation discharge site is also shown in this figure. It is proposed to take the additional augmentation water from a proposed new bore located in adjacent land also owned by Planation Road Dairies and discharge directly into the Kahahakuri Stream immediately beside this location.

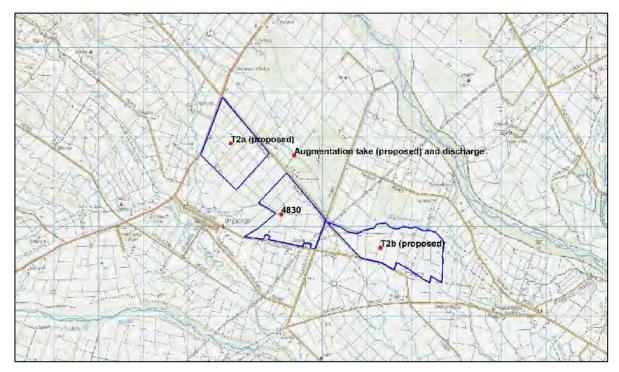


Figure 10: Plantation Road Dairies existing and proposed bores and augmentation locations (irrigation areas are shown as blue outlines)

Key infrastructure for Plantation Road Dairies proposed Tranche 2 take are summarised in Table 9.

Table 9: Plantation Road Dairies proposed Tranche 2 infrastructure

Site	Use and/or status	Current status	Depth (m bgl)	Approx. irrigated area (ha)
4830	Irrigation	Existing	137	144
T2a	Irrigation	Proposed	~ 100	155
T2b	Irrigation	Proposed	~ 100	160
Augmentation take	Augmentation take	Proposed	~ 100	-
Augmentation discharge	Discharge to Kahahakuri Stream	Proposed	-	-

3.6.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile water demand for pasture is calculated to be approximately 600 mm/year, which over an irrigated area of 459 ha equates to a volume of approximately 2.75 million m³/year. This is similar to the total volume of irrigation water initially sought by Plantation Road Dairies for irrigation purposes (2,775,914 m³/year).

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The crop water demand has been modelled from the bores proposed to supply the property, as listed in Table 9, in proportion to the listed irrigated areas. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR for the areas that were previously unirrigated.

3.6.2 Modelling Results for Plantation Road Dairies

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics due to Plantation Road Dairies' irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 10. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 10: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Plantation Road Dairies' irrigation take alone (no augmentation)

	Site					
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa	
Change in average flow (I/s)						
Plantation Road Dairies' irrigation take only	-43	-24	0	-8	-14	
Change in mean 7-day annual low flow (MALF) (l/s)						
Plantation Road Dairies' irrigation take only	-38	-13	0	-6	-16	

From Table 10, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the Waipawa catchment and tributaries, though effects do propagate over to the lower Tukituki catchment and tributaries.

Changes in flows allowing for all applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.7 Springhill Dairies

Springhill Dairies (formerly Ingleton Farms) have applied to take up to **1,005,213** m³/year of Tranche 2 groundwater from deep bores located around their property. The proposed Tranche 2 take will supplement existing consented takes with a combined volume of approximately 4,029,077 m³/year to provide adequate irrigation of 702 ha of pasture (assumed), or a larger area of less water-intensive crops or horticulture, from bores 1518, 3870, 4122, 4593, 5497 and 5167 (combined). It is proposed to provide augmentation directly into the Mangaonuku Stream.

The location of the bores from which Springhill Dairies have proposed to take groundwater, and the approximate locations of the augmentation take bore and the discharge site, are shown in Figure 11.

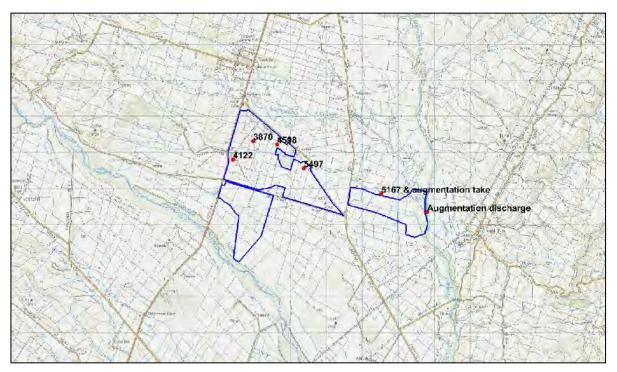


Figure 11: Springhill Dairies' bores and augmentation locations (irrigation areas are shown as blue outlines)

Key infrastructure for Springhill Dairies' proposed takes are summarised in Table 11.

Table 11: Springhill Dairies' proposed infrastructure

Site	Use and/or status	Current status	Depth (m bgl)
Bore 5167	Irrigation & augmentation take	Existing bore and irrigation take; proposed augmentation take	124.6
Bore 4593	Irrigation	Existing	84.7
Bore 1518	Irrigation	Existing	152.9
Bore 3870	Irrigation	Existing	144.7
Bore 4122	Irrigation	Existing	134.2
Bore 5497	Irrigation	Existing	56.1
Augmentation discharge	Discharge to Mangaonuku Stream	Proposed	-

For the purposes of this assessment, it has been assumed that the additional Tranche 2 water is extracted from bores 3870 and 5167.

3.7.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile annual water demand for pasture is calculated to be approximately 480 mm/year, which over an irrigated area of 702 ha equates to 3.4 million m³/year. The irrigation demand is able to be met by utilising a combination of the exiting consented takes and a portion of the Tranche 2 volume applied for, whilst leaving a reasonable volume of Tranche 2 water available for augmentation purposes. The irrigation volume initially applied for (597,997 m³/year) would be adequate to irrigate approximately 125 ha.

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The crop water demand has been modelled from the bores proposed to supply the property, as listed in Table 11. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR for the new irrigated areas.

3.7.2 Modelling Results for Springhill Dairies

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics due to Springhill Dairies' irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 12. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 12: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Springhill Dairies' irrigation take alone (no augmentation)

	Site					
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa	
	Change in average flow (I/s)					
Springhill Dairies' irrigation take only	-13	-3	0	-1	-6	
Change in mean 7-day annual low flow (MALF) (I/s)						
Springhill Dairies' irrigation take only	-11	-1	0	-1	-7	

From Table 12, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the Waipawa catchment and tributaries, as this is the catchment within which Springhill Dairies is located. However, small scale effects do propagate over to the lower Tukituki catchment and tributaries. The overall effects are relatively small compared to some other applicants because the total scale of the proposed Tranche 2 take is also relatively small.

Changes in flows allowing for all applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.8 I&P Farming

I&P Farming have submitted two applications to take Tranche 2 groundwater. An original application was submitted to take up to 477,122 m³/year of Tranche 2 groundwater, and a new application has recently been submitted to take up to 722,888 m³/year of groundwater. These combine to a total of 1,200,010 m³/year and comprises 913,862 m³/year for irrigation and 286,148 m³/year for augmentation. The applications seek to take groundwater from new deep bores (likely two new bores) located on their property. The irrigation rate applied for is intended to fully irrigate approximately 184 ha of pasture (assumed), or a larger area of less water-intensive crops or horticulture.

The general location of the bores from which I&P Farming have proposed to take groundwater, and the approximate location of the augmentation discharge site, are shown in Figure 12. It is proposed to discharge water into an existing unnamed small stream that joins the Tukituki River less than 1 km below the downgradient boundary of the property. This unnamed stream is not included in the model, and therefore it has been assumed that the augmentation discharge occurs at the confluence of this stream with the Tukituki River, as indicated in Figure 12. This site is a little higher up-catchment than the site originally proposed by I&P Farming in their initial application.

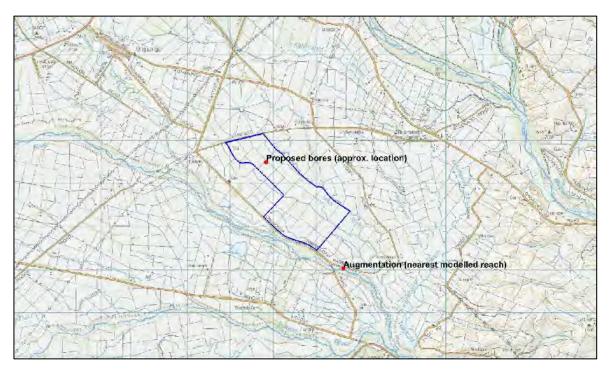


Figure 12:1&P Farming's bore and augmentation locations (irrigation areas are shown as blue outlines)

Key infrastructure for I&P Farming's proposed take are summarised in Table 13.

Table 13: I&P Farming's proposed infrastructure

Site	Use and/or status	Current status	Depth (m bgl)
Proposed bore	Irrigation & augmentation take	Proposed	60 m min (assumed)
Augmentation discharge	Discharge to unnamed stream, tributary of Tukituki River	Proposed	-

3.8.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile annual water demand for pasture is calculated to be approximately 550 mm/year. The irrigation volume applied is therefore adequate to irrigate approximately 166 ha of pasture, which is a little less than the 184 ha intended for mixed cropping. The smaller area of pasture will be assumed for modelling purposes.

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The crop water demand has been modelled from the proposed bore to supply the property, as listed in Table 13. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR for the new irrigated areas.

3.8.2 Modelling Results for I&P Farming

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics due to I&P Farming's irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 14. Although the results are presented to the nearest litre per

second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 14: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to I&P Farming's irrigation take alone (no augmentation)

	Site					
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa	
	Change in average flow (I/s)					
I&P Farming's irrigation take only	-10	-13	0	-2	-2	
Change in mean 7-day annual low flow (MALF) (l/s)						
I&P Farming's irrigation take only	-9	-5	0	-2	-2	

From Table 14, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin.

Changes in flows allowing for all applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.9 Buchanan Trust No 2 (Buchanan)

Buchanan have applied to take up to 1,631,018 m³/year of Tranche 2 groundwater from existing and proposed new deep bores located around their property. However, at the time of their original application, only **1,145,794 m³/year** was available from the Tranche 2 volume available. This equates to approximately 70% of the water applied for.

Assuming augmentation volume is 20% of the total take (this may be adjusted in subsequent scenarios, discussed later), then the Tranche 2 volume remaining for Buchanan would be made up of 915,894 m³/year for irrigation and a further 229,900 m³/year for augmentation. This reduced volume has been applied to the modelling scenarios and is adequate to fully irrigate 153 ha of pasture (assumed) or a larger area of less water-intensive crops or horticulture.

The location of the bores from which Buchanan have proposed to take groundwater, and the approximate location of the augmentation discharge site, are shown in Figure 13. It is proposed to discharge water into the nearby reach of Ongaonga Stream, which converges with Tukituki River approximately 4 km below the property.

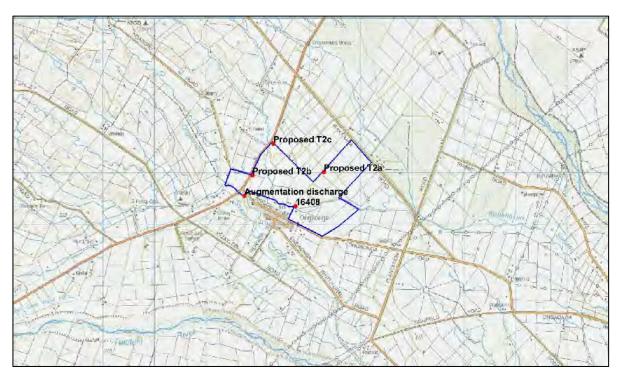


Figure 13: Buchanan's bores and augmentation locations (irrigation areas are shown as blue outlines)

Key infrastructure for Buchanan's proposed take are summarised in Table 15.

Table 15: Buchanan's proposed infrastructure

Site	Use and/or status	Current status	Depth (m bgl)
16408	Irrigation & augmentation take	Existing	119.8
Proposed T2a	Irrigation & augmentation take	Proposed	
Proposed T2b	Irrigation & augmentation take	Proposed	~ 110 m (assumed)
Proposed T2c	Irrigation & augmentation take	Proposed	(3.22.3)
Augmentation discharge	Augmentation Discharge to Ongaonga Stream		-

3.9.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile annual water demand for pasture is calculated to be approximately 600 mm/year. This results in the 153 ha of pasture being fully irrigable from the 915,894 m³/year volume.

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The crop water demand has been modelled from the proposed bore to supply the property, as listed in Table 15. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR for the new irrigated areas.

3.9.2 Modelling Results for Buchanan

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in

flow statistics due to Buchanan's irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 16. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 16: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Buchanan's irrigation take alone (no augmentation)

	Site						
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa		
	Change in average flow (I/s)						
Buchanan's irrigation take only	-17	-9	0	-4	-5		
Change in mean 7-day annual low flow (MALF) (I/s)							
Buchanan's irrigation take only	-13	-3	0	-2	-5		

From Table 16, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the Waipawa catchment.

Changes in flows allowing for all applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.10 Purunui Trust

Purunui Trust have applied to take up to 1,575,000 m³/year of Tranche 2 groundwater from three deep bores located around their property, near Ongaonga. The annual volume applied for is comprised of:

- 1,050,000 m³/year (at a volume not exceeding 252,000 m³ within any 28 day period) for irrigation of up to 175 ha (assumed all pasture), capped at meeting full water demand up to a one-in-ten year drought; and
- An additional 525,000 m³/year (at volume not exceeding 126,000 m³ within any 28 day period) to provide river augmentation to mitigate the effects of the irrigation take during dry periods.

Due to their position as last in the queue of Tranche 2 groundwater applications, the full volume applied for by Purunui Trust is not available due to the 15,000,000 m³/year cap on the combined Tranche 2 takes (RRMP Table 5.9.5). Instead, a total volume of **554,921 m³/year** is available, which equates to approximately 35% of the water applied for. Assuming both the irrigation and augmentation volume is scaled equally, then the Tranche 2 volume remaining for Purunui Trust would be made up of 369,944 m³/year for irrigation and a further 184,977 m³/day for augmentation. This reduced volume has been applied to the modelling scenarios and is adequate to fully irrigate approximately 62 ha of pasture (assumed), or a larger area of less water-intensive crops or horticulture.

The location of the bores (existing and proposed) from which Purunui Trust have proposed to take groundwater, and the approximate locations of the augmentation take bore and the discharge site, are shown in Figure 14. It is proposed to provide river augmentation into the Waipawa River via an existing unused large-diameter shallow bore located approximately 200-300 m from the river. Purunui Trust have advised that they believe this shallow bore is directly connected to the nearby Waipawa River, though this will be confirmed prior to commencing augmentation. For the purpose of augmentation modelling, it will be assumed that the water will be discharge directly into the Waipawa River immediately adjacent to the existing shallow bore. Due to the fast hydraulic response between the Waipawa River and nearby shallow groundwater, this will make little difference to the modelled effects.

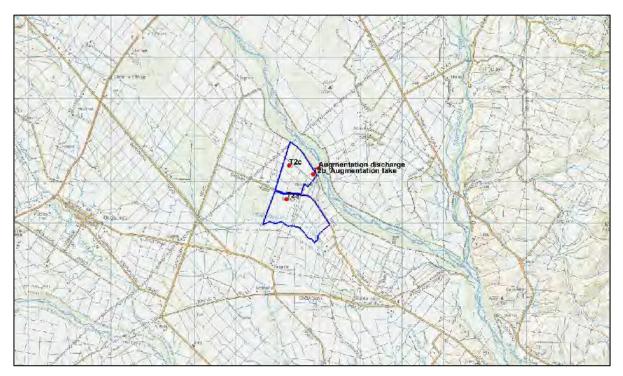


Figure 14: Purunui Trust's proposed bore and augmentation locations (proposed irrigation areas are shown as blue outlines)

Key infrastructure for Purunui Trust's proposed Tranche 2 take are summarised in Table 17.

Table 17: Purunui Trust's proposed Tranche 2 infrastructure

Site	Use and/or status	Current status	Depth (m bgl)	Approx. irrigated area (ha)
T2a	Irrigation	Proposed		31
T2b	Augmentation	Proposed	50 m min (assumed)	-
T2c	Irrigation	Proposed	()	31
Augmentation discharge	Discharge to Waipawa River	Proposed	-	-

3.10.1 Irrigation Demand

Irrigation demand has been modelled using Aqualinc's IrriCalc soil-water balance model. The 90 percentile water demand for pasture is calculated to be approximately 600 mm/year, which over an irrigated area of 62 ha equates to an annual volume of 372,000 million m³/year.

The IrriCalc modelling provides a daily time series of crop water demand (assuming an unrestricted supply) and resulting daily time series of land surface recharge (LSR). The crop water demand has been modelled from the bores proposed to supply the property, as listed in Table 17, in proportion to the listed irrigated areas. Furthermore, new irrigation results in additional LSR compared to dryland. This has also been incorporated into the model by exchanging previously dryland LSR with irrigated LSR for the proposed irrigated areas shown in Figure 14.

3.10.2 Modelling Results for Purunui Trust

The model has been run with daily outputs generated from 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites shown in Figure 6 have been processed to

generate changes in flow statistics due to Purunui Trust's irrigation take alone. Changes in these flow statistics compared to the baseline are summarised in Table 18. Although the results are presented to the nearest litre per second, the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 18: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to Purunui Trust's irrigation take alone (no augmentation)

	Site					
Scenario	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa	
	Change in average flow (I/s)					
Purunui Trust's irrigation take only	-6	-3	0	-1	-2	
Change in mean 7-day annual low flow (MALF) (I/s)						
Purunui Trust's irrigation take only	-6	-1	0	-1	-2	

From Table 18, irrigation alone (with no augmentation) results in a reduction in river flows throughout much of the basin. The largest changes occur in the Waipawa catchment and tributaries, as this is the catchment within which Purunui Trust's property is located. However, small scale effects do propagate over to the lower Tukituki catchment and tributaries.

Changes in flows allowing for all Tranche 2 applicants' irrigation takes, augmentation takes and augmentation discharges are presented later.

3.11 Augmentation Scenarios: All Takes Combined Plus Augmentation

The effects from individual applicants are summarised below. Following this, several scenarios have been developed that consider the combined effects of all applicants together with augmentation.

3.11.1 Summary of Individual Effects

Effects for individual applicants are summarised in Table 19.

Table 19: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due for all proposed Tranche 2 irrigation takes (no augmentation)

	Site				
Applicant	Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa
Change in average flow (I/s)					
TAFT	-64	-18	0	-8	-35
Papawai	-25	-8	0	-3	-9
Tuki Tuki Awa	-8	-11	-1	-7	-2
Plantation Rd Dairies	-43	-24	0	-8	-14
Springhill Dairies	-13	-3	0	-1	-6
I&P Farming	-10	-13	0	-2	-2
Buchanan	-17	-9	0	-4	-5
Purunui	-6	-3	0	-1	-2
Total	-186	-89	-1	-34	-75
Change in mean 7-day annual low flow (MALF) (I/s)					
TAFT	-48	-9	0	-6	-37
Papawai	-18	-3	0	-2	-10
Tuki Tuki Awa	-4	-5	-1	-5	-2
Plantation Rd Dairies	-38	-13	0	-6	-16
Springhill Dairies	-11	-1	0	-1	-7
I&P Farming	-9	-5	0	-2	-2
Buchanan	-13	-3	0	-2	-5
Purunui	-6	-1	0	-1	-2
Total	-147	-40	-1	-25	-81

From the above table, the changes in river flows statistics are concentrated in the Waipawa catchment where most of the new takes are located. However, effects do also occur in the lower Tukituki sites. No noticeable effect is predicted in the Tukipo River at SH50 as no properties are sited near or above this location.

3.11.2 Restrictions

To provide a reliable supply for irrigation, it is proposed that the Tranche 2 takes are operated unrestricted (supplying crop water demand as needed, day to day). To then mitigate stream depletion effects caused by the irrigation takes and to maintain reliability for existing users, surface water augmentation (sourced from groundwater) is proposed when rivers reach minimum flows. For the

Ruataniwha basin, the river flow restrictions were initially based on the following sites and flows, as specified in RRMP Table 5.9.3: Tukituki Catchment Minimum Flows framework:

•	Waipawa at RDS/SH2:	2,500 l/s
•	Tukituki River at Tapairu Road:	2,300 l/s
•	Tukipo at SH50:	150 l/s
•	Tukipo at Ashcott:	1,043 l/s
•	Mangaonuku at u/s Waipawa confluence	1,170 l/s

The locations of these flow restriction sites are shown in Figure 6.

The daily river flow time series used to generate a time series of restrictions has been modified for the base case by a time series of changes in flows resulting from all applicants (combined) taking their irrigation takes alone. This was generated by the individual scenarios discussed in Section 3.3 through to Section 3.10.

A further flow restriction site exists on the Tukituki River at Red Bridge, down catchment from the Ruataniwha basin outlet. This is below the model's extent and therefore the flow monitoring sites on the Waipawa at SH2 and Tukituki at Tapairu Road have been considered to represent this site. So long as the combined 7-day MALF at the SH2 and Tapairu sites are maintained (or improved), then the downstream low flows at Red Bridge will not be adversely affected by the proposed Tranche 2 takes.

It has been assumed that the full augmentation rate is taken (sourced from Tranche 2 water) and discharged at the locations described above for each applicant whenever restrictions on any one of the above RMP Table 5.9.3 minimum flow sites occurs. Furthermore, as all Tranche 2 applicants are operating collaboratively, and effects from any one take potentially propagate across several streams, it has been assumed that augmentation will occur by all of the applicants if any one of the RRMP Table 5.9.3 minimum flow sites experiences a low-flow restriction. As will be discussed later, the exception to this is Tuki Tuki Awa in scenario 4.

3.11.3 Augmentation Scenario 1

Scenario 1 assumes that augmentation occurs only when irrigation from Tranche 2 groundwater is occurring on each applicant's property. However, as outlined above, if one of the RRMP Table 5.9.3 minimum flow sites is triggered then augmentation commences from all Tranche 2 applicants, but also only occurs when they are each irrigating. The model has been run with the combined applicants' proposed irrigation takes and augmentation, for the period 1 July 1972 through to 30 June 2012. Time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics (compared to the baseline), which are summarised in Table 20. Again, the results are presented to the nearest litre per second, but the model's accuracy is not that precise. Therefore, these results represent the approximate scale and direction of effect.

Table 20: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 1

Site								
Waipawa at SH2	Mangaonuku u/s Waipawa							
	Change in average flow (I/s)							
-179	-80	-2	-37	-67				
Change in mean 7-day annual low flow (MALF) (I/s)								
-61	+74	-1	-28	-47				

Calculated augmentation volumes for each applicant are summarised in Table 21. Also included in this table for comparison is a summary of the total Tranche 2 groundwater volumes applied for each applicant.

Table 21: Calculated seasonal augmentation volumes for augmentation scenario 1

	Approx. area irrigated from	Modelled (Tranche 2 GW volume		
Applicant	Tranche 2 groundwater (ha) ⁽¹⁾	For irrigation	For augmentation (% of total)	Total	applied (m³/year)
TAFT	540	3,100,000	868,700 (18%)	3,968,700	4,914,920
Papawai	207	1,161,000	122,800 (8%)	1,283,800	1,475,517
Tuki Tuki Awa	136	612,000	106,900 (11%)	718,900	952,400
Plantation Rd Dairies	459	2,754,000	577,300 (15%)	3,331,300	3,751,225
Springhill Dairies	125 ⁽²⁾	597,997 ⁽²⁾	177,300 (18%)	775,297	1,005,213
I&P Farming	166 ⁽³⁾	913,862	70,300 (6%)	984,162	1,200,010
Buchanan	153 ⁽³⁾	915,894	229,900 (20%)	1,145,794	1,145,794 ⁽³⁾
Purunui	62 ⁽³⁾	448,121	66,800 (12%)	514,921	554,921 ⁽³⁾
	Total	10,502,874	2,220,000 (15%)	12,722,874	15,000,000

⁽¹⁾ These areas are based on the assumption that pasture is irrigated. However, larger areas of less intensive crops or horticulture can be irrigated within the same yearly volumes used by pasture, with little-to-no difference in the hydraulic effect.

A map of groundwater level difference between the status quo and augmentation scenario 1 is presented in Figure 15 for the dry period of 1 March 2001. This is during a time of typically lowest groundwater levels (in most monitored wells), and is consistent with the equivalent datasets presented in Weir (2013). This demonstrates how groundwater levels are predicted to change spatially during dry periods. Temporal changes in groundwater are discussed later.

From Figure 15, the greatest changes in shallow groundwater levels occur west (upgradient) of the take locations. This is because shallow groundwater level changes in areas downgradient of the take locations are mitigated (or partially mitigated) by the river augmentation; areas upgradient (and also deeper layers, not shown) do not receive the full benefit of the augmentation.

The following key observations are derived from the above results:

- Augmentation benefits most the catchment below where it is directly discharged.
- From Table 19, the proposed irrigation takes combined (without augmentation) are predicted to result in a total reduction in the 7-day MALF out of the basin of approximately 187 l/s (-147 l/s from the Waipawa and -40 l/s from the Tukituki.
- From Table 20, with augmentation, the equivalent change in the combined 7-day MALF out of the basin is an increase of approximately 13 l/s (-61 l/s in the Waipawa and +74 l/s in the Tukituki). However, reductions in flows are predicted for the Mangaonuku River (upstream of the Waipawa confluence) and the lower Tukipo River site (at Ashcott Rd).
- Therefore, the augmentation trialled under this scenario is having a partial mitigating effect, but it is insufficient to fully mitigate the effects of the proposed new Tranche 2 takes, for the scenario tested.
- Shallow groundwater levels are predicted to lower up to a maximum of 0.8 m under augmentation scenario 1. This maximum change is focussed in the vicinity of greatest abstraction (the applicants' properties). Elsewhere, shallow groundwater levels are predicted to change less than 0.3 m.

⁽²⁾ The Tranche 2 water will supplement existing takes to irrigate a larger area of land than can be irrigated from the Tranche 2 water alone. Therefore, it has been assumed above that all of the Tranche 2 irrigation water applied for is utilised.

⁽³⁾ Less than applied for.

The augmentation volumes in Table 21 were derived by summing (for each year) the proposed augmentation rate applied whenever it is triggered, and calculating the 90%ile annual volume. The total volume of Tranche 2 water abstracted (approximately 12.7 million m³) is less than that available (15.0 million m³). At face value, this might suggest that a greater volume of groundwater could be abstracted for irrigation. However, this is not the case as the effects on surface flows set out in Table 20 need to be further mitigated, and that can only occur by taking more deep groundwater for augmentation purposes and/or taking less water for irrigation. This leads into Scenario 2.

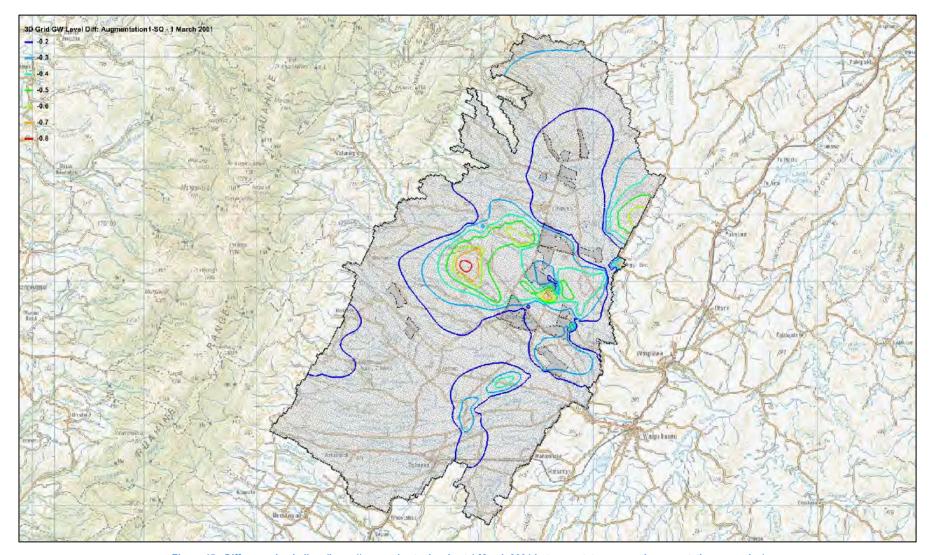


Figure 15: Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 1

3.11.4 Augmentation Scenario 2

From the above model results, augmentation is not predicted to fully mitigate the effects on low flows at some river sites. Therefore, an alternative augmentation regime has been trialled. Scenario 2 assumes augmentation occurs whenever RRMP Table 5.9.3 minimum flows are reached throughout the year, regardless of whether or not irrigation is occurring. This scenario aims to better mitigate the temporal effects of the takes during low-flow periods through more frequent augmentation. This in turn results in a greater volume of Tranche 2 water taken for augmentation, and in some cases means individual applicants are forecasted to take more than their applied volume. Therefore, some irrigated areas (and therefore irrigation volumes) have been reduced to counter this.

This scenario has been modelled, and time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics (compared to the baseline) which are summarised in Table 22.

Table 22: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 2

Site								
Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa				
	Change in average flow (I/s)							
-170	-65	-2	-40	-52				
Change in mean 7-day annual low flow (MALF) (I/s)								
-1	+145	-1	-30	0				

Calculated augmentation volumes for each applicant are summarised in Table 23 along with irrigation volumes and the necessary reduced irrigated areas, compared to the total Tranche 2 groundwater volumes and irrigated areas applied for.

Table 23: Calculated seasonal augmentation volumes for augmentation scenario 2

Applicant	from T	pprox. area irrigated from Tranche 2 groundwater (ha) ⁽¹⁾		Modelled 90 percentile annual volumes (m³/year) (1972-2012)			
Applicant	As applied	Adjusted to annual volume	For irrigation	For augmentation (% of total)	Total	applied (m³/year)	
TAFT	540	453	2,627,120	2,287,800 (47%)	4,914,920	4,914,920	
Papawai	207	207	1,161,000	290,500 (20%)	1,451,500	1,475,517	
Tuki Tuki Awa	136	136	612,000	302,600 (32%)	914,600	952,400	
Plantation Rd Dairies	459	380	2,280,525	1,470,700 (39%)	3,751,225	3,751,225	
Springhill Dairies	125 ⁽²⁾	114	545,213	460,000 (46%)	1,005,213	1,005,213	
I&P Farming	166 ⁽³⁾	166	913,862	266,300 (22%)	1,180,162	1,200,010	
Buchanan	153 ⁽³⁾	89	533,294	612,500 (53%)	1,145,794	1,145,794 ⁽³⁾	
Purunui	62 ⁽³⁾	62	381,831	173,100 (31%)	554,931	554,921 ⁽³⁾	
		Total	9,054,845	5,863,500 (39%)	14,918,345	15,000,000	

⁽¹⁾ These areas are based on the assumption that pasture is irrigated. However, larger areas of less intensive crops or horticulture can be irrigated within the same yearly volumes used by pasture, with little-to-no difference in the hydraulic effect.

⁽²⁾ The Tranche 2 water will supplement existing takes to irrigate a larger area of land than can be irrigated from the Tranche 2 water alone. Therefore, it has been assumed above that all of the Tranche 2 irrigation water applied for is utilised.

⁽³⁾ Less than applied for

A map of groundwater level difference between the status quo and augmentation scenario 2 is presented in Figure 16 for 1 March 2011. This demonstrates how groundwater levels are predicted to change spatially during dry periods. Temporal changes in groundwater are discussed later.

The following key observations are derived from these results:

- Compared to augmentation scenario 1, the more frequent augmentation provides additional benefit to the low flows exiting the catchment.
- As previously noted in Table 19, the combined effects from the proposed irrigation takes alone are predicted to result in a total reduction in the 7-day MALF out of the basin of approximately 187 l/s (-147 l/s from the Waipawa and -40 l/s from the Tukituki). Under the alternative augmentation scenario 2 results in Table 22, the combined 7-day MALF out of the basin is expected to increase by 144 l/s, a result of a -1 l/s (decrease) in the Waipawa and +145 l/s (increase) in the Tukituki.
- Overall, alternative augmentation scenario 2 is having a much larger positive effect on the 7day MALF exiting the basin, but does not fully mitigate the effects in the Tukipo River at Aschott Rd.
- Shallow groundwater levels are predicted to lower up to a maximum of 0.8 m under augmentation scenario 2. Again, this maximum change is focussed in the areas of greatest abstraction (near applicants' properties). Elsewhere, shallow groundwater levels are predicted to change less than 0.3 m.

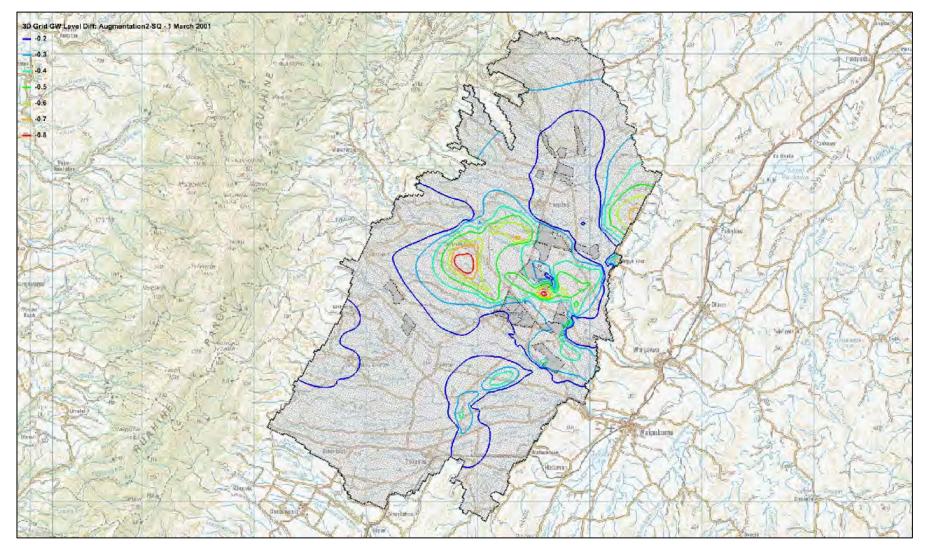


Figure 16: Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 2

3.11.5 Augmentation Scenario 3

An alternative augmentation time series has been trialled whereby low-flow restrictions (and therefore augmentation) are assumed to occur sooner (at higher flow rates) than those listed in Section 3.11.2. HBRC have previously indicated that they would consider this more favourable than operating on the low-flow restriction trigger values set out in RRMP Table 5.9.3. The following rationale has been applied to generate an alternative set of low flow trigger values when augmentation would commence:

Waipawa at RDS/SH2: 2,500 l/s current flow trigger

+ 147 l/s low flow reduction due to takes (Table 19) + 50% additional reduction for augmentation take

Round up to 2.725 I/s

Tukituki River at Tapairu Road: 2,300 l/s current flow trigger

> + 40 l/s low flow reduction due to takes (Table 19) + 50% additional reduction for augmentation take Round up to 2,360 l/s

Tukipo at SH50: 150 l/s current flow trigger

> + 1 l/s low flow reduction due to takes (Table 19) + 50% additional reduction for augmentation take Round up to 155 I/s

Tukipo at Ashcott: 1,043 l/s current flow trigger

> + 25 l/s low flow reduction due to takes (Table 19) + 50% additional reduction for augmentation take

Round up to 1,085 I/s

Mangaonuku at u/s Waipawa confluence

1,170 l/s current flow trigger

+ 81 l/s low flow reduction due to takes (Table 19)

+ 50% additional reduction for augmentation take

Round up to 1,295 I/s

These raised low-flow trigger values have been applied to the augmentation takes regardless of whether or not irrigation is occurring on the day (as was the case for augmentation scenario 2: augmentation is assumed to occur even if the applicant is not irrigating). This scenario 3 aims to better maintain existing users' reliability by triggering augmentation at higher river flows. In turn, this (again) results in a greater volume of Tranche 2 water taken for augmentation, which pushes some applicants total take beyond their applied volumes. So some irrigated areas (and therefore irrigation volumes) have been further reduced to counter this.

This scenario 3 has been modelled, and time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics (compared to the baseline) which are summarised in Table 24.

Table 24: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 3

Site								
Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa				
	Change in average flow (I/s)							
-157	-56	-2	-40	-45				
Change in mean 7-day annual low flow (MALF) (I/s)								
+17	+158	-1	-29	+11				

Calculated augmentation volumes for each applicant are summarised in Table 25 along with irrigation volumes and the necessary reduced irrigated areas, compared to the total Tranche 2 groundwater volumes and irrigated areas applied for.

Table 25: Calculated seasonal augmentation volumes for augmentation scenario 3

Applicant	Approx. area irrigated from Tranche 2 groundwater (ha) (1)		Modelled 90 percentile annual volumes (m³/year) (1972-2012)			Tranche 2 GW volume
Applicant	As applied	Adjusted to annual volume	For irrigation	For augmentation (% of total)	Total	applied (m³/year)
TAFT	540	427	2,475,320	2,439,600 (50%)	4,914,920	4,914,920
Papawai	207	207	1,165,717	309,800 (21%)	1,475,517	1,475,517
Tuki Tuki Awa	136	136	629,700	322,700 (34%)	952,400	952,400
Plantation Rd Dairies	459	364	2,182,925	1,568,300 (42%)	3,751,225	3,751,225
Springhill Dairies	125 ⁽²⁾	107	514,713	490,500 (49%)	1,005,213	1,005,213
I&P Farming	166 ⁽³⁾	166	916,010	284,000 (24%)	1,200,010	1,200,010
Buchanan	153 ⁽³⁾	82	492,594	653,200 (57%)	1,145,794	1,145,794 ⁽³⁾
Purunui	62 ⁽³⁾	62	370,321	184,600 (33%)	554,921	554,921 ⁽³⁾
		Total	8,474,300	6,252,700 (42%)	15,000,000	15,000,000

⁽¹⁾ These areas are based on the assumption that pasture is irrigated. However, larger areas of less intensive crops or horticulture can be irrigated within the same yearly volumes used by pasture, with little-to-no difference in the hydraulic effect.

A map of groundwater level difference between the status quo and augmentation scenario 3 is presented in Figure 17 for 1 March 2011. This demonstrates how groundwater levels are predicted to change spatially during dry periods. Temporal changes in groundwater are discussed later.

The following key observations are derived from these results:

- Compared to augmentation scenario 2, the more frequent and earlier-commenced augmentation provides additional benefit (again) to the 7-Day MALF of to the Waipawa and Tukituki rivers exiting the catchment.
- As previously noted in Table 19, the combined effects from the proposed irrigation takes alone are predicted to result in a total reduction in the 7-day MALF out of the basin of approximately 187 l/s (-147 l/s from the Waipawa and -40 l/s from the Tukituki). Under augmentation scenario 3 (Table 24), the combined 7-day MALF out of the basin is expected to increase by 175 l/s as a result of a +17 l/s (increase) in the Waipawa and +158 l/s (increase) in the Tukituki.
- Overall, the alternative augmentation scenario is having a larger positive effect on the 7-day MALF exiting the basin, but again does not fully mitigate the effects in the Tukipo River at Ashcott Rd.
- Similar to previous scenarios, shallow groundwater levels are predicted to lower up to a maximum of 0.8 m under augmentation scenario 3, focussed near the applicants' properties. Elsewhere, shallow groundwater levels are predicted to change less than 0.3 m.

⁽²⁾ The Tranche 2 water will supplement existing takes to irrigate a larger area of land than can be irrigated from the Tranche 2 water alone. Therefore, it has been assumed above that all of the Tranche 2 irrigation water applied for is utilised.

⁽³⁾ Less than applied for.

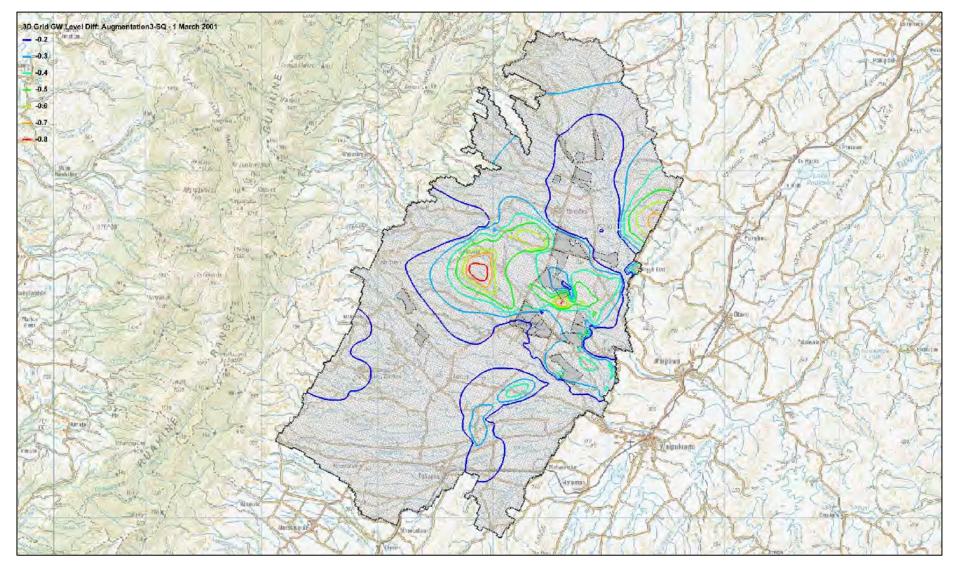


Figure 17: Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 3

Although there is a residual effect for the Tukipo River at Ashcott Rd of -29 l/s (Table 24), this equates to less than 3% of the minimum flow for this site (1,043 l/s). It could be argued that this is a minor adverse effect, as based on the modelled hydraulic response for this site, it is estimated that low-flow restrictions might commence hours (rather than days) earlier than might otherwise occur, as demonstrated in Figure 18. The green line shows how the flow in the Tukipo would recede under scenario 3 compared to the case if no Tranche 2 takes were occurring (the blue line).

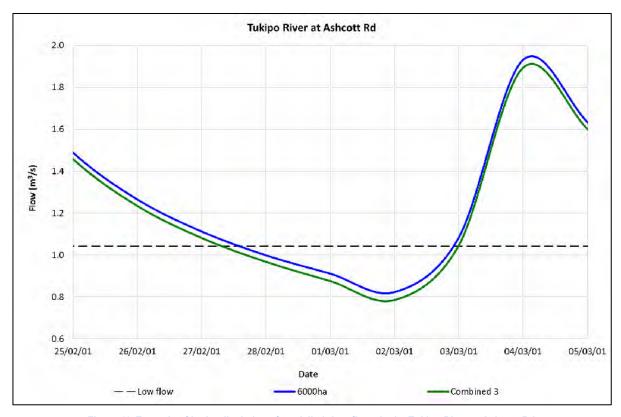


Figure 18: Example of hydraulic timing of modelled river flows in the Tukipo River at Ashcott Rd

Nevertheless, a further scenario has been explored with the aim being to avoid, or minimise to the extent practicable, the residual adverse effect on the Tukipo at Ashcott Rd. This, along with additional balancing of augmentation and irrigated area, is scenario 4 that is discussed next.

3.11.6 Augmentation Scenario 4

Based on the results of augmentation scenario 3, changes in flow in the Tukipo River at Aschott Rd are not fully mitigated. However, low flows exiting the basin are over compensated. Therefore, augmentation scenario 4 assumes a reduction of 15% to each applicant's augmentation discharge rate, with a corresponding increase in the irrigated areas such that the total volume of Tranche 2 groundwater proposed for each applicant is no more than the volume applied and the areas to be irrigated do not exceed the irrigable areas sought in the original Tranche 2 applications. However, the following exceptions apply:

- Buchanan's augmentation rate has been scaled back to 55% of the previously modelled rate due to the disproportionately high ratio of augmentation to irrigation take (compared to other applicants).
- I&P Farming and Purunui Trust's augmentation were not reduced due to the disproportionally low rate of augmentation to irrigation take (compared to other applicants).

- Papawai Partnership's augmentation was increased by 50% of the previously modelled rate due
 to the disproportionally low rate of augmentation to irrigation take (compared to other applicants)
 and the need to provide additional augmentation to the Waipawa River.
- Tuki Tuki Awa's augmentation reduced to 20% of the previously modelled rate since irrigation is only being used to gap fill and augment river flows when their river take is on low-flow restriction (discussed below).

Tuki Tuki Awa propose to use their Tranche 2 groundwater take only to gap-fill the surface water supply and augment rivers when the existing low-flow conditions on their take are triggered. This reduced take has been modelled under augmentation scenario 4.

Scenario 4 has been modelled, and time series of river flows at each of the flow restriction sites have been processed to generate changes in flow statistics (compared to the baseline) which are summarised in Table 26.

Table 26: Modelled changes in flow statistics (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with augmentation scenario 4

Site								
Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa				
	Change in average flow (I/s)							
-162	-36	-1	-7	-53				
Change in mean 7-day annual low flow (MALF) (I/s)								
+3	+135	-1	+1	+2				

Calculated augmentation volumes for each applicant are summarised in Table 27 along with irrigation volumes and the reduced irrigated areas, compared to the total Tranche 2 groundwater volumes and irrigated areas applied for.

Table 27: Calculated seasonal augmentation volumes for augmentation scenario 4

	Approx. area irrigated from Tranche 2 groundwater (ha) (1)		Modelled 90 percentile annual volumes (m³/year) (1972-2012)			Tranche 2 GW volume
Applicant	As applied	Adjusted to annual volume	For irrigation	For augmentation (% of total)	Total	applied (m³/year)
TAFT	540	490	2,841,220	2,073,700 (42%)	4,914,920	4,914,920
Papawai	207	181	1,010,817	464,700 (31%)	1,475,517	1,475,517
Tuki Tuki Awa	136	136	678,100 ⁽²⁾	29,600 (4%)	707,700	952,400
Plantation Rd Dairies	459	403	2,418,225	1,333,000 (36%)	3,751,225	3,751,225
Springhill Dairies	125 ⁽³⁾	123	588,313	416,900 (41%)	1,005,213	1,005,213
I&P Farming	166 ⁽⁴⁾	166	916,010	284,000 (24%)	1,200,010	1,200,010
Buchanan	153 ⁽⁴⁾	131	786,594	359,200 (31%)	1,145,794	1,145,794 (4)
Purunui	62 ⁽⁴⁾	62	370,321	184,600 (33%)	554,921	554,921 ⁽⁴⁾
		Total	9,609,600	5,145,700 (35%)	14,755,300	15,000,000

⁽¹⁾ These areas are based on the assumption that pasture is irrigated. However, larger areas of less intensive crops or horticulture can be irrigated within the same yearly volumes used by pasture, with little-to-no difference in the hydraulic effect.

⁽²⁾ This is Tuki Tuki Awa's 90 percentile irrigation demand for 136 ha (as applied). However, because the Tranche 2 take is to be used only when their surface water take is restricted, the Tranche 2 irrigation use in most years will be less than this. Assuming past climatic patterns, the 90 percentile demand for irrigation only when their surface water take is restricted is approximately 258,400 m³/year.

⁽³⁾ The Tranche 2 water will supplement existing takes to irrigate a larger area of land than can be irrigated from the Tranche 2 water alone. Therefore, it has been assumed above that all of the Tranche 2 irrigation water applied for is utilised.

⁽⁴⁾ Less than applied for.

A map of groundwater level difference between the status quo and augmentation scenario 4 is presented in Figure 19 for 1 March 2011. This demonstrates how groundwater levels are predicted to change spatially during dry periods. Temporal changes in groundwater are discussed later.

The following key observations are derived from these results:

- As previously noted in Table 19, the combined effects from the proposed irrigation takes alone are predicted to result in a total reduction in the 7-day MALF out of the basin of approximately 187 l/s (-147 l/s from the Waipawa and -40 l/s from the Tukituki). Under augmentation scenario 4 (Table 26), the combined 7-day MALF out of the basin is expected to increase by approximately 138 l/s as a result of +3 l/s (increase) in the Waipawa and +135 l/s (increase) in the Tukituki.
- Overall, augmentation scenario 4 is having a positive effect on the 7-day MALF exiting the basin, and also fully or nearly mitigates the effects on low flows in both the Mangaonuku and Tukipo rivers.
- Similar to previous scenarios, shallow groundwater levels are predicted to lower up to a maximum of 0.8 m under augmentation scenario 4, focussed near the applicants' properties. Elsewhere, shallow groundwater levels are predicted to change less than 0.3 m.

From the above, augmentation scenario 4 provides a reasonable balance between irrigation and augmentation while providing a positive benefit to low flows out of the catchment (low flows increase compared to the baseline scenario where no Tranche 2 water is taken). In other words, the baseline adverse effects of the Tranche 2 takes on surface water low flows exiting the basin are overcompensated under scenario 4. The small residual negative change to 7-day MALF in the Tukipo River at SH50 is smaller than both model uncertainty and measurement precision.

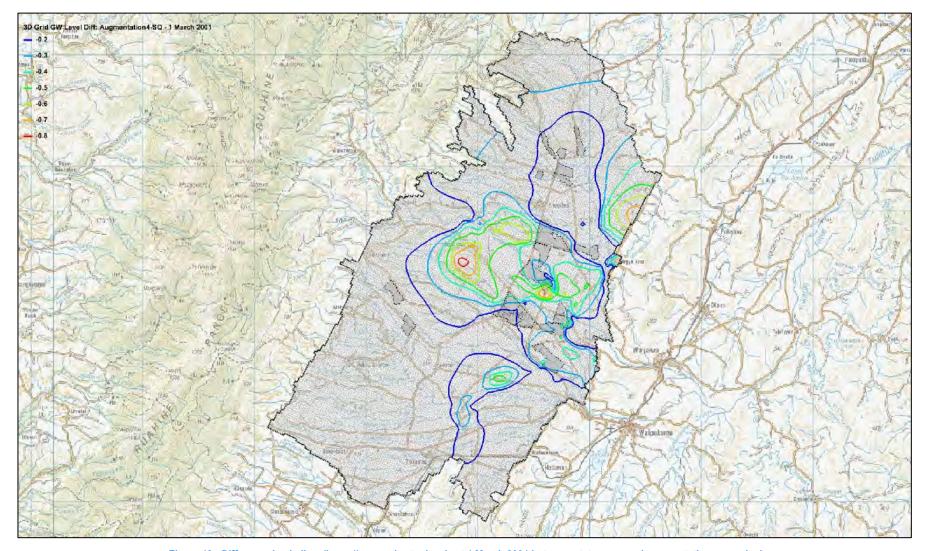


Figure 19: Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 4

3.11.7 Temporal Response in Calibration Wells

Appendix D provides groundwater level hydrographs comparing the baseline (status quo) with the four augmentation scenarios. Generally, the wells that respond less to climate and groundwater abstraction are shallower wells that are screened in the shallow unconfined aquifer that has higher storage coefficients compared to deeper wells. Groundwater levels in these shallower wells are also moderated by streams. Deeper wells experience greater seasonal variation and greater response to the additional Tranche 2 takes. However, there is no obvious temporal variation (over multiple years) to suggest that groundwater will be mined and continue to decline. Groundwater levels simply oscillate at a lower dynamic equilibrium.

For wells that are obviously influenced by pumping, the seasonal response (the saw-tooth effect) as a result of the additional Tranche 2 takes is much smaller than what is currently experienced from existing takes.

3.12 Practical Implications

To aid practical management, operation and compliance of the Tranche 2 takes, several protocols for the takes and augmentation are proposed. These proposals differ to the modelling assumptions. However, at a practical level, the differences are only marginal, and it is expected that the net effect on river flows and groundwater levels will be very similar to, or better than, those presented in the modelling above. The practical implications relate to:

- Staged development and transitional implementation;
- · Automated monitoring of river flows; and
- Defining a 'Water Year' and associated start date of irrigation and augmentation.

These are each discussed below.

3.12.1 Staged Development and Transitional Implementation

The modelling scenarios presented above assume that every Tranche 2 take operates fully from day 1 (e.g. all bores are installed; irrigation systems are fully operational with mature pasture; augmentation is fully operational; etc.). This presents the extreme (or 'final') state that is eventually expected. However, this will not be the case initially. Some property owners have existing bores from which they can irrigate as soon as consents are granted; others have partial development; and others currently have no infrastructure and are awaiting for consents to be granted before commencing significant investment. This means that the full uptake of Tranche 2 water will not be instantaneous upon granting of the consents, but will progressively develop ('ramp up') over several years. Hence, implementation will be transitional.

The Tranche 2 applicants considered a range of options to practically manage the effects throughout the transition period. The most sensible solution arrived at proposes that each consent holder nominates (to HBRC) a maximum seasonal volume of water that they expect to need that season, proportional to the scale of property development. This then becomes the maximum allocation volume for that season (only) and the augmentation discharge rate is pro-rated on this same basis. This volume is then reassessed in subsequent years. For example, let's assume that a property has a 1-in-10 year maximum seasonal volume of 1,000,000 m³/year and proposes to augment the adjacent river at 20 l/s when low-flow triggers are reached. If by the first irrigation season the property is 40% developed, then the maximum irrigation volume for that year will be 400,000 m³ and the augmentation rate would be 8 l/s. If by the following year development has expanded to 60%, then the maximum irrigation volume for that year would be 600,000 m³ and the augmentation rate would be 12 l/s. And so on up to 100% development.

This transitional implementation is founded on the principle that, while the effects of pumping propagate over a relatively large distance, the effects are largely seen in the vicinity of the abstraction location. Consequently, effects and subsequent augmentation to mitigate these effects (during low flows) are proportional to the scale of the take.

If a Tranche 2 consent holder is unsure of the level of development expected for an upcoming season, they will not want to under-predict their water needs. Hence, they would be more likely to nominate a higher percentage than a lower one. The nominated percentage of development then dictates (and locks in) the rate of augmentation for that year, regardless of whether or not the irrigation volume is used. Hence, it is more likely that the augmentation rate will be over compensated in these transitional years. This provides benefits to the rivers greater than modelled.

3.12.2 Automated Monitoring of River Flows

Of the five low-flow trigger sites modelled, HBRC currently monitor three of these automatically (Waipawa at SH2, Tukituki at Tapairu Rd, and Tukipo at SH50) with daily updates provided to consent holders. The other two sites are manually gauged, and updates are provided less frequently (such as weekly). Rivers and shallow groundwater downgradient of the augmentation discharge sites will respond rapidly to the discharge. Hence, daily controls are preferred to mitigate the low flows rapidly, but without significantly over compensating when river flows rise (e.g. if a fresh occurs between manual gaugings). Hence, the Tranche 2 applicants propose that their augmentation discharges are controlled daily based only on the existing three monitoring sites currently automatically monitored by HBRC. If automatic flow recorders are installed at the other two sites in the future, then these can then be added to the daily control regime.

This triggering regime is expected to make little difference in the very dry years when augmentation is needed the most (all rivers experience naturally low flows). There may be some 'unders and overs' in the wetter years where the three continuously monitored sites do not fully represent the other two sites. In these cases, the augmentation may not be triggered when these smaller streams are below the low flow trigger. However, it is expected that the targeted-overcompensation at other times provides a buffer that will partially mitigate effects on these streams at these times.

3.12.3 'Water Year' Definition and Associated Start Dates of Irrigation and Augmentation

HBRC currently define the allocation water year from 1 July to 30 June. On 1 July, monitoring of the annual volume is reset to 'zero' for the upcoming season. The Tranche 2 applicants have two volumetric limits: an irrigation volume and an augmentation volume, both defined by a 9-in-10 year season. If the water year commences 1 July (as currently defined by HBRC), it is possible that augmentation to rivers may be needed through winter at the start of the water year when there is no irrigation pumping. While this does acknowledge the fact that the effects of irrigation continue on beyond when pumping stops (as it takes time for the aquifer system to recover), there is a small possibility that augmentation water will be used in the cooler, wetter months (if minimum flows are triggered) resulting in insufficient augmentation water later in the warmer, drier parts of the season when the augmentation is needed most.

Because of this, the Tranche 2 applicants propose to define the water year commencing at the start of the irrigation season, nominally 1 October. Then, given that effects from the deep pumping take time to propagate to the surface, it is proposed that the augmentation year starts 1 month after this (i.e. 1 November). This has multiple consequences:

- Augmentation water is 'saved' for the driest times of the year when it is needed most, usually
 well beyond the start of the irrigation season.
- Delaying the start of augmentation will mean that, in most years, the augmentation volume will not be fully used before winter; this unused water can then be discharged throughout winter when low flows are triggered.
- Continuing to augment during winter results in the equivalent of full-year augmentation for most years (9 in 10), but provides the added assurance that there will be augmentation water available

in the driest parts of driest seasons when it is needed most, rather than potentially running out just before it is needed.

• There may be the occasional time when low flows are reached during winter, but the augmentation volume is fully used and augmentation cannot continue. Based on historical records, this would occur infrequently (1 year in 10).

This proposal to define the water year as noted above will reduce the likelihood of augmentation water being used up before the irrigation water limit is reached.

3.13 Summary and Recommendations

The following key findings have been derived from the modelled scenarios:

- The effects on river flows from the proposed additional Tranche 2 takes are spread over both space and time throughout the basin.
- If augmentation occurs when both existing minimum flows are reached and when irrigation would be operating, it is insufficient to mitigate depletion of 7-day MALF flows due to additional Tranche 2 takes.
- If augmentation is applied when minimum flows are reached, regardless of whether or not
 irrigation is operating, augmentation is more beneficial and results in an overall improvement in
 low flows exiting the basin in both the Tukituki and Waipawa rivers. The exception to this is Tuki
 Tuki Awa who propose to take Tranche 2 groundwater and augment to the Tukituki River only
 when their existing surface water take is restricted. This also does not adversely affect low river
 flows.
- For the Tranche 2 applicants, the adoption of higher minimum flow triggers (and therefore more frequent augmentation) provides greater benefit to 7-day MALF. It also better protects the reliability for existing abstraction consent holders.
- Targeted over-compensation accommodates prediction uncertainty, enhances environmental low-flows, and accommodates across-catchment effects.
- Under augmentation scenario 4, adverse effects on surface water low flows as a result of the Tranche 2 takes are either avoided (positive effects occur as evidenced by the increased flows at the Waipawa and Tukituki low flow sites), or are so minor that they fall within the margin of modelling and measurement uncertainty (i.e. for the Tukipo SH50 low flow site).
- Groundwater will not be mined. While groundwater levels will lower further with the additional Tranche 2 takes (made available under PC6), they will not continue to lower; they will simply reach a new (lower) dynamic equilibrium. Shallow groundwater levels are predicted to lower a maximum of 0.8 m in the vicinity of the Tranche 2 take locations, and less than 0.3 m further afield.
- The reported irrigation and augmentation volumes are based on a 90-percentile year. Therefore, is it is possible that in extreme dry years (e.g. 1 year in 10), low flows could still be triggered after irrigation and augmentation volumes have been exhausted.

From the optimised scenario (augmentation scenario 4), the optimum area of irrigated pasture (and corresponding volumes) and augmentation volumes are summarised in Table 28.

Table 28: Optimised areas of irrigated pasture and Tranche 2 groundwater volumes (reproduced from Table 27)

	Modelled augmentation	Approx. area	Modelled 9	Tranche 2 GW		
Applicant	rate (I/s, daily	irrigated from Tranche 2 GW (ha) ⁽¹⁾	For irrigation	For augmentation (% of total)	Total	volume applied (m³/year)
TAFT	189	490	2,841,220	2,073,700 (42%)	4,914,920	4,914,920
Papawai	24	181	1,010,817	464,700 (31%)	1,475,517	1,475,517
Tuki Tuki Awa	5	136	678,100	29,600 (4%)	707,700	952,400
Plantation Rd Dairies	103	403	2,418,225	1,333,000 (36%)	3,751,225	3,751,225
Springhill Dairies	38	123	588,313	416,900 (41%)	1,005,213	1,005,213
I&P Farming	22	166	916,010	284,000 (24%)	1,200,010	1,200,010
Buchanan	51	131	786,594	359,200 (31%)	1,145,794	1,145,794
Purunui	14	62	370,321	184,600 (33%)	554,921	554,921
		Total	9,609,600	5,145,700 (35%)	14,755,300	15,000,000

⁽¹⁾ These areas are based on the assumption that pasture is irrigated. However, larger areas of less intensive crops or horticulture can be irrigated within the same yearly volumes used by pasture, with little-to-no difference in the hydraulic effect.

From Table 28 it can be seen that the modelled total abstraction does not equate to 15,000,000 m³/year. This is a result of the proposal by Tuki Tuki Awa to only use Tranche 2 groundwater when their surface wate take is restricted. However, this does not mean that the balance (244,700 m³/year) is available for allocation to other applicants because in some extreme years the full allocation sought by Tuki Tuki Awa may be used.

Proposed minimum flow triggers and net changes in 7-day MALF for key river flow monitoring sites under the optimised scenario are summarised in Table 29.

Table 29: Modelled changes in 7-day MALF (over the period 1972-2012) compared to status quo due to all applicants' irrigation takes with optimised augmentation (scenario 4)

Site								
Waipawa at SH2	Tukituki at Tapairu Rd	Tukipo at SH50	Tukipo at Ashcott Rd	Mangaonuku u/s Waipawa				
	Assumed low-flow restriction applied (I/s) (1)							
2,725	2,360	155	1,085	1,295				
Ci	hange in mean 7-	day annual low f	low (MALF) (I/s)	(2)				
+3	+135	-1	+1	+2				
Change in 7-day MALF as a percentage of low-flow limit								
+0.1%	+5.7%	-0.6%	+0.1%	+0.2%				

⁽¹⁾ Discussed in Section 3.11.5. These are higher than current RRMP Table 5.9.3 limits to provide greater environmental benefit during low flows and to protect reliability of existing users.

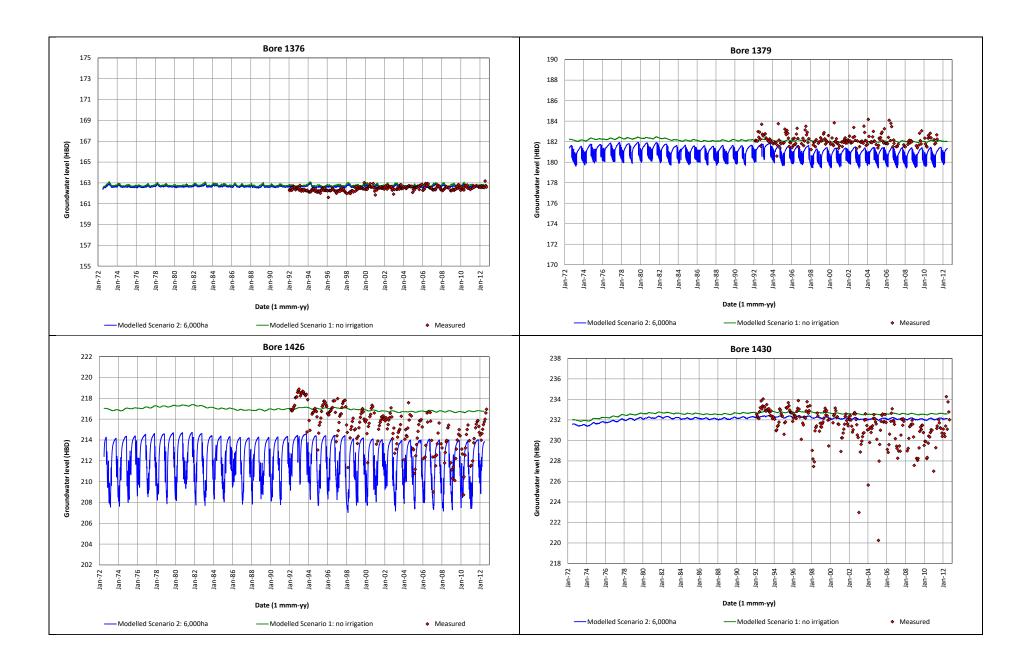
Several management, operational and compliance protocols are recommended to aid practical implementations of the Tranche 2 takes and augmentation.

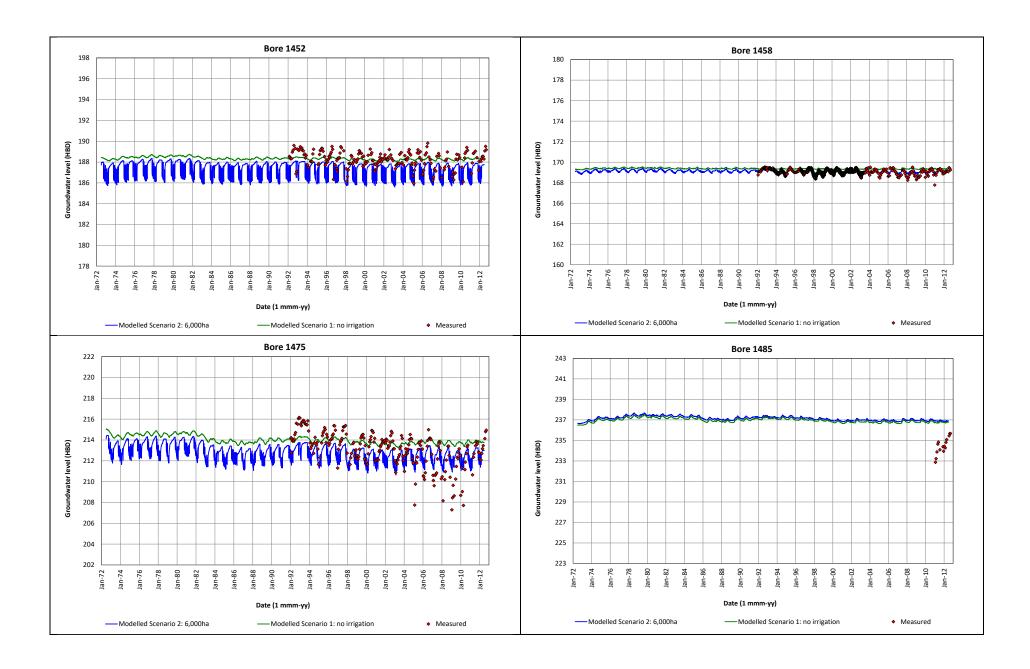
⁽²⁾ Reproduced from Table 26.

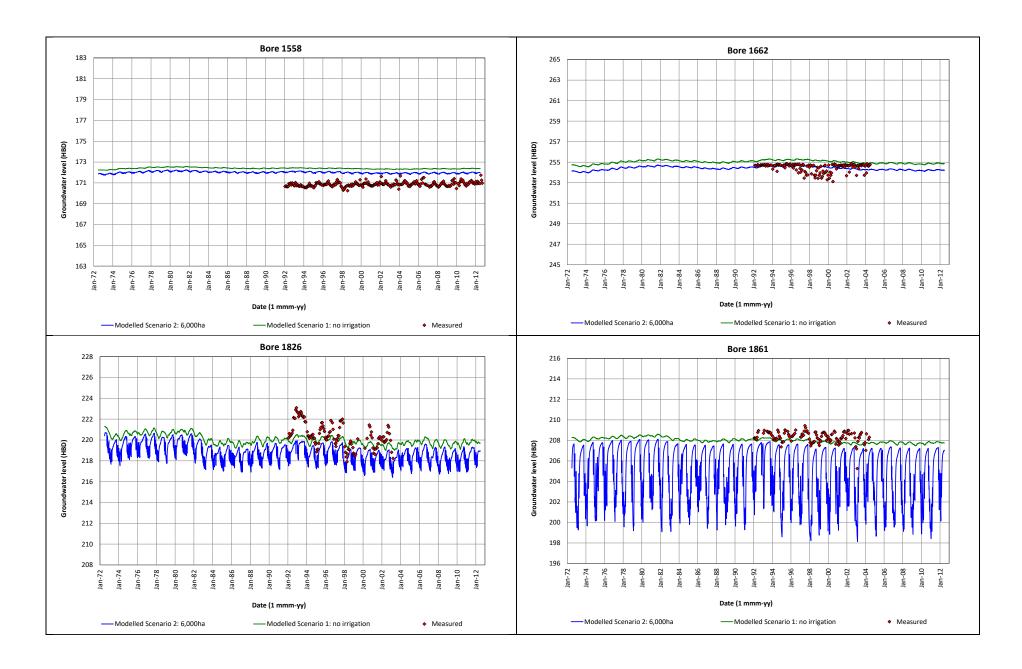
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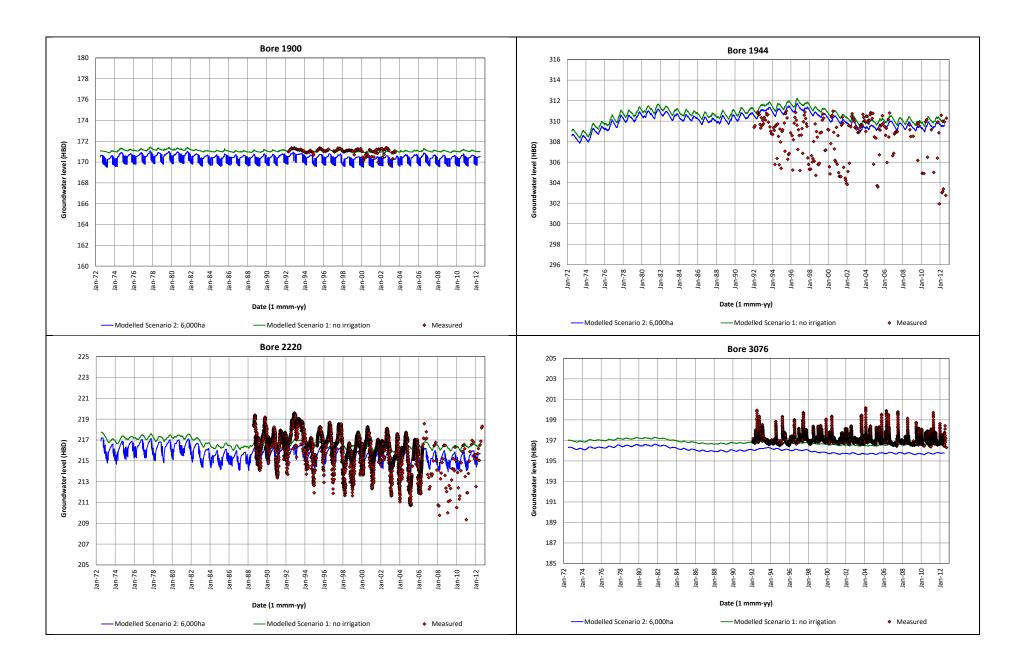
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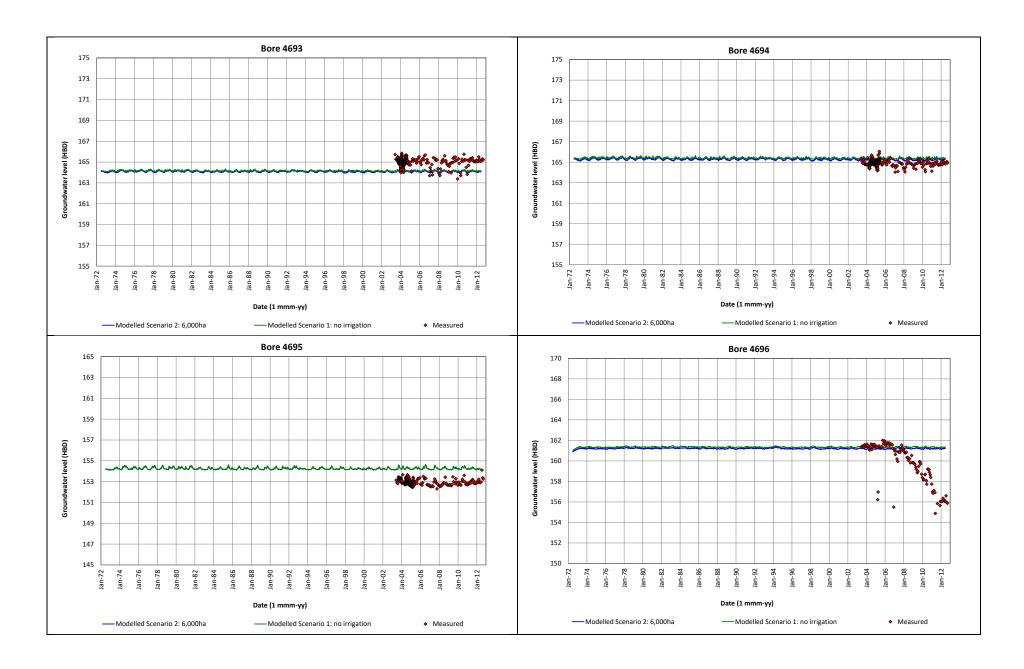
Appendix A: Groundwater level calibration plots

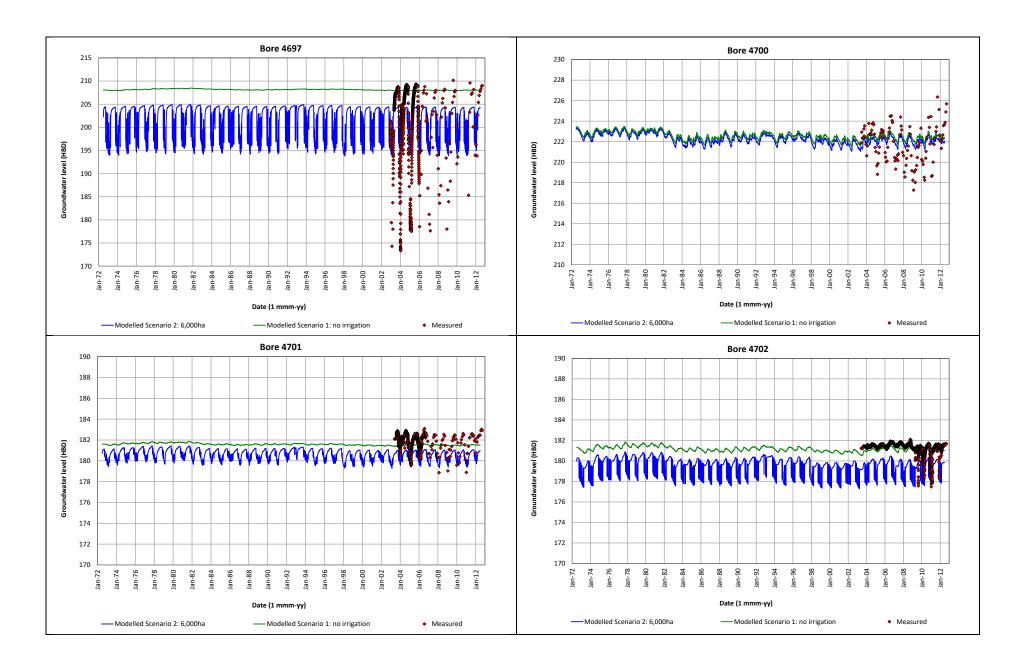


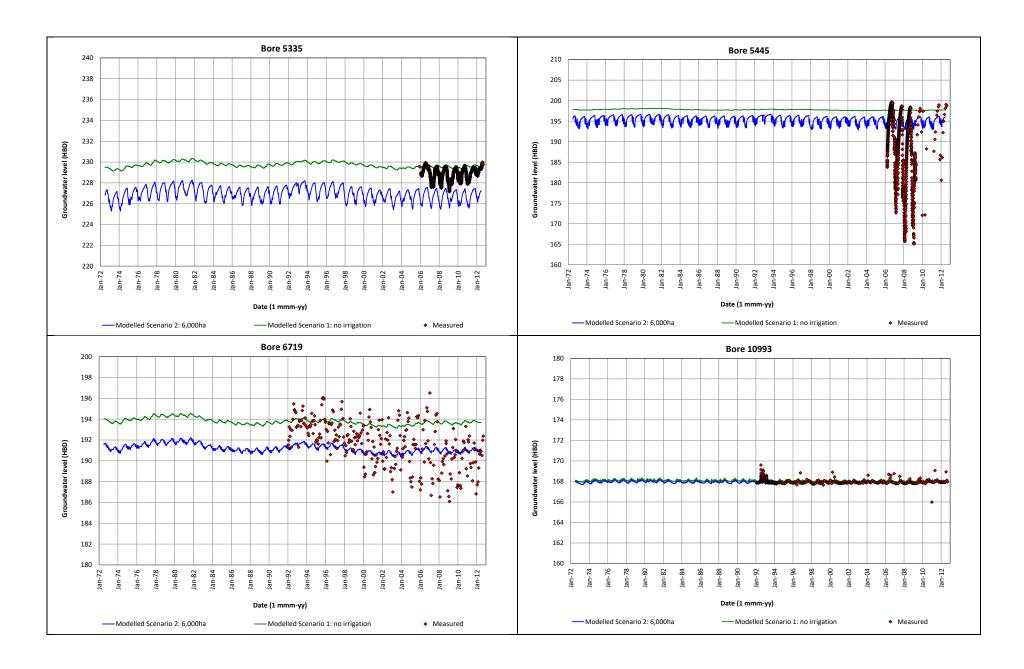


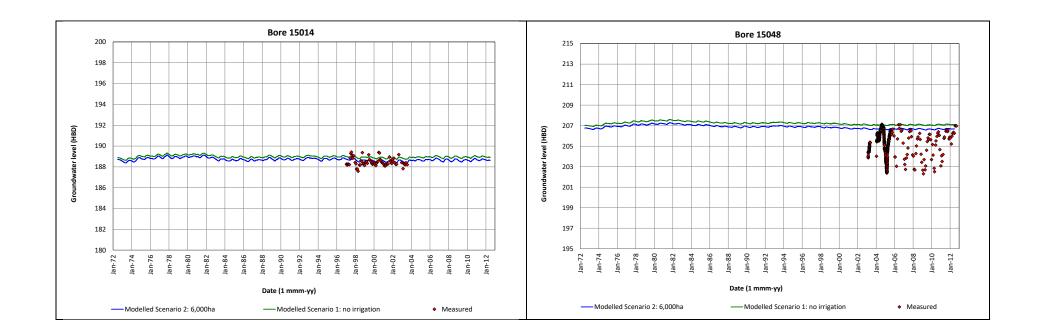




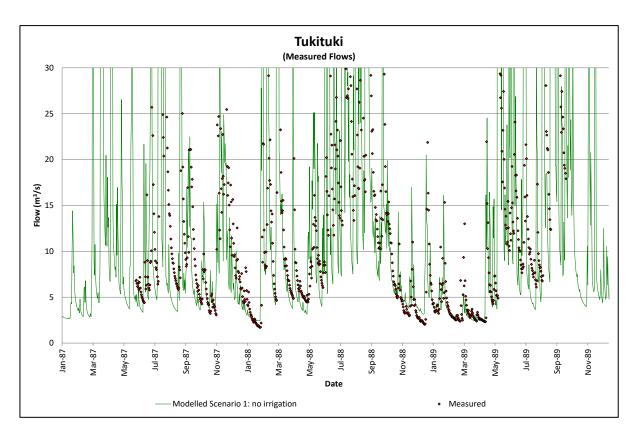


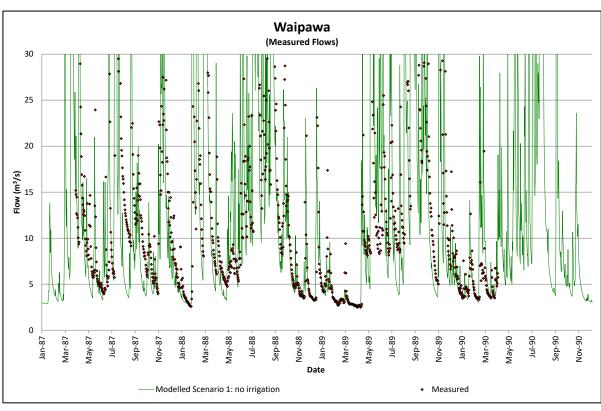




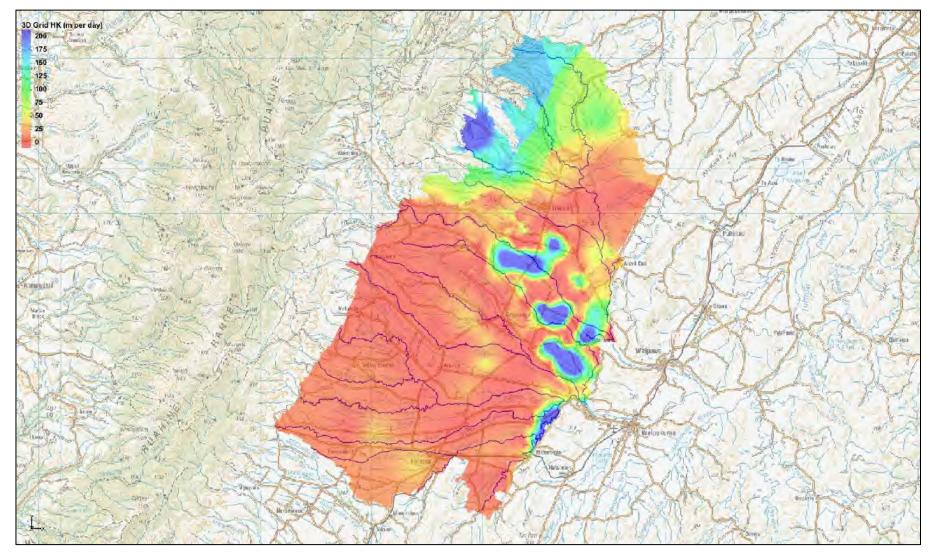


Appendix B: River flow calibration plots

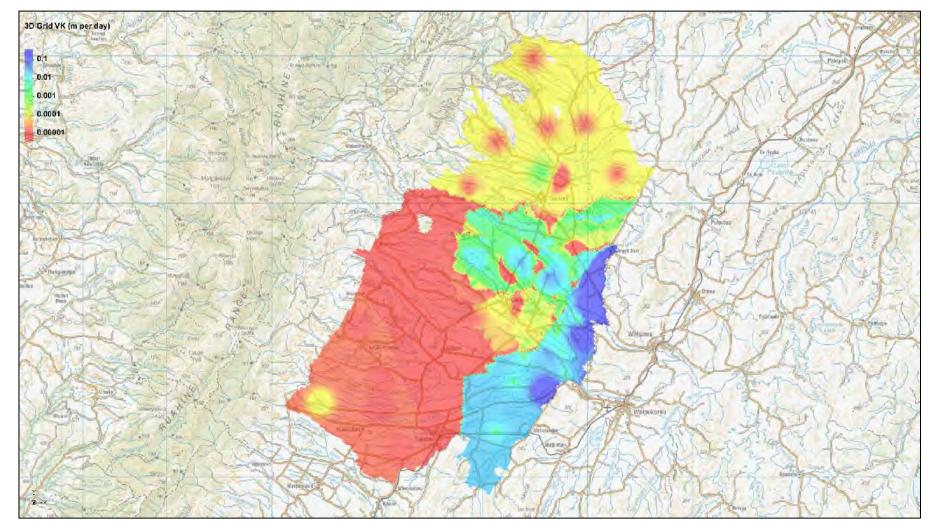




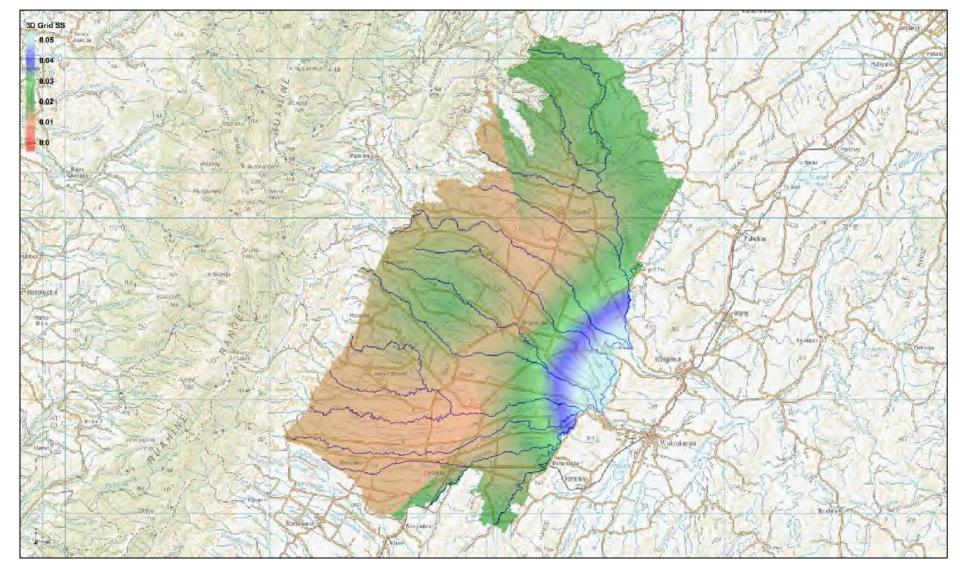
Appendix C: Calibrated Aquifer Parameters



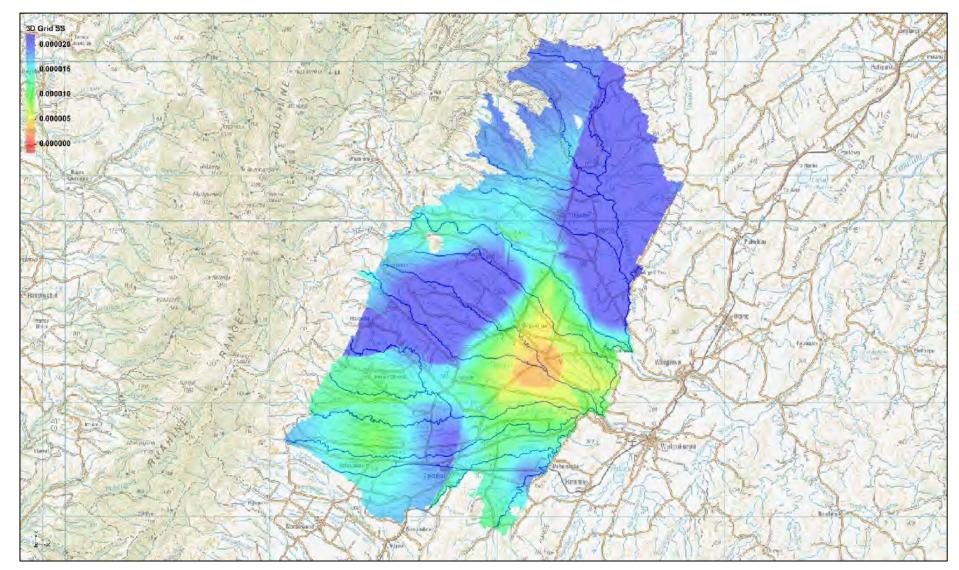
Horizontal hydraulic conductivities (for all layers)



Vertical hydraulic conductivities (for layers 2 and below)

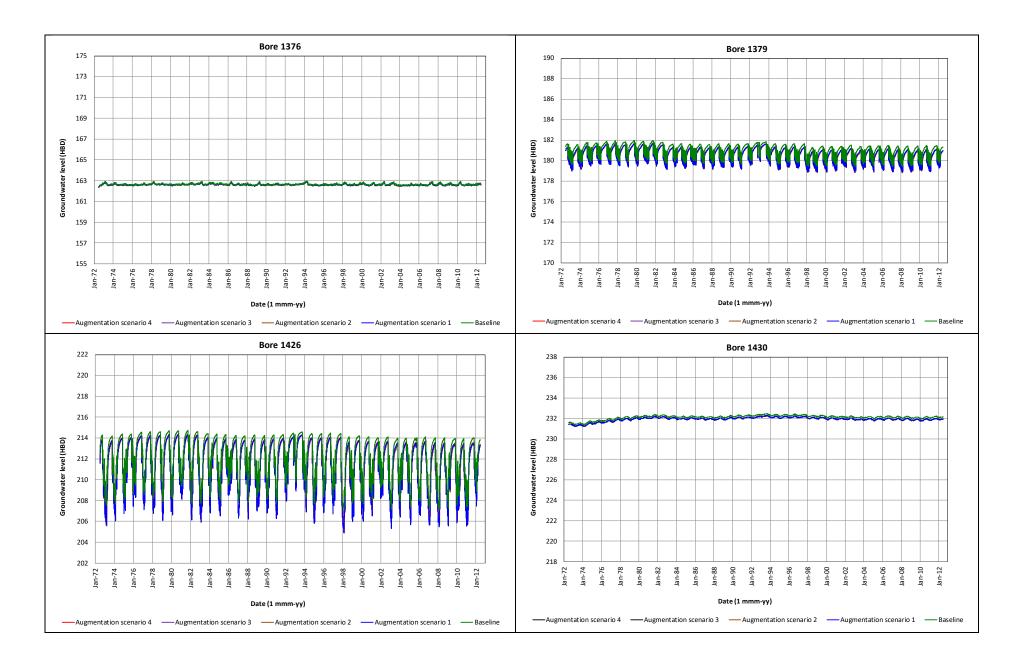


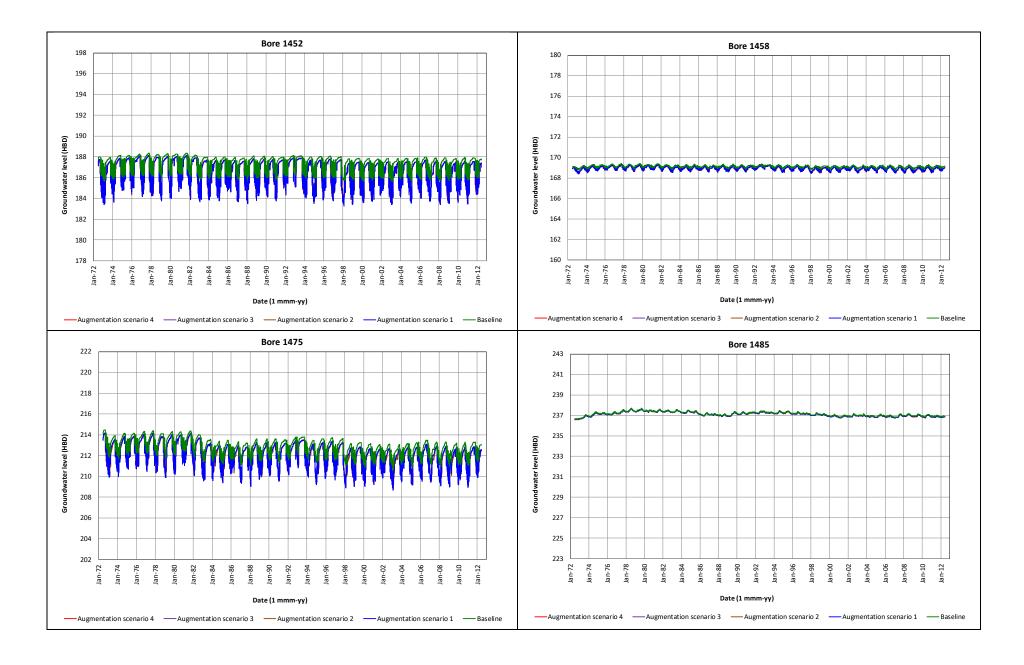
Specific storage for layer 1

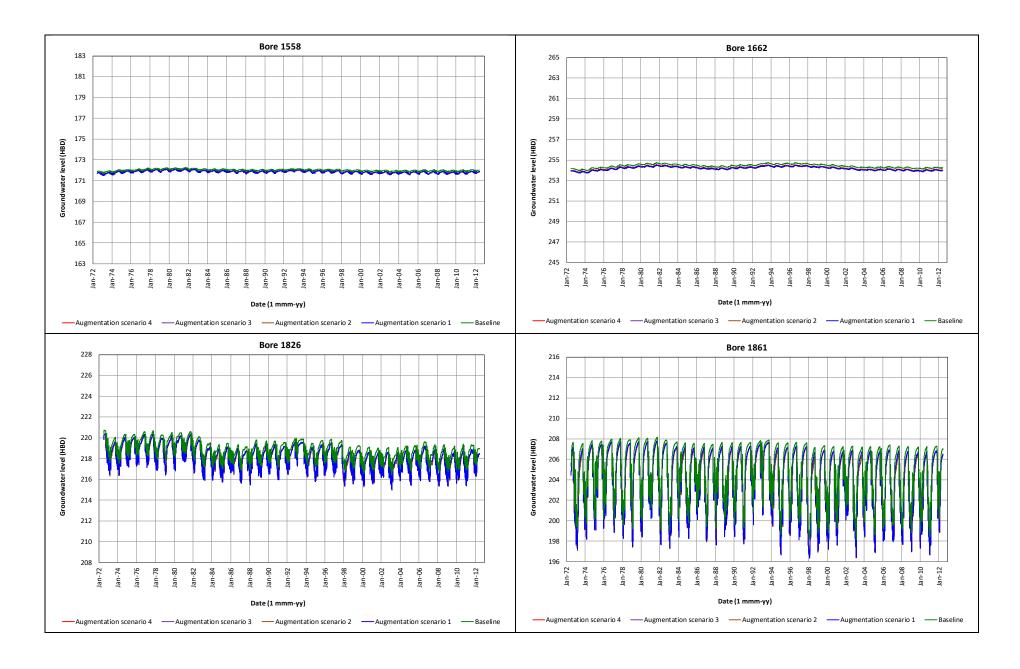


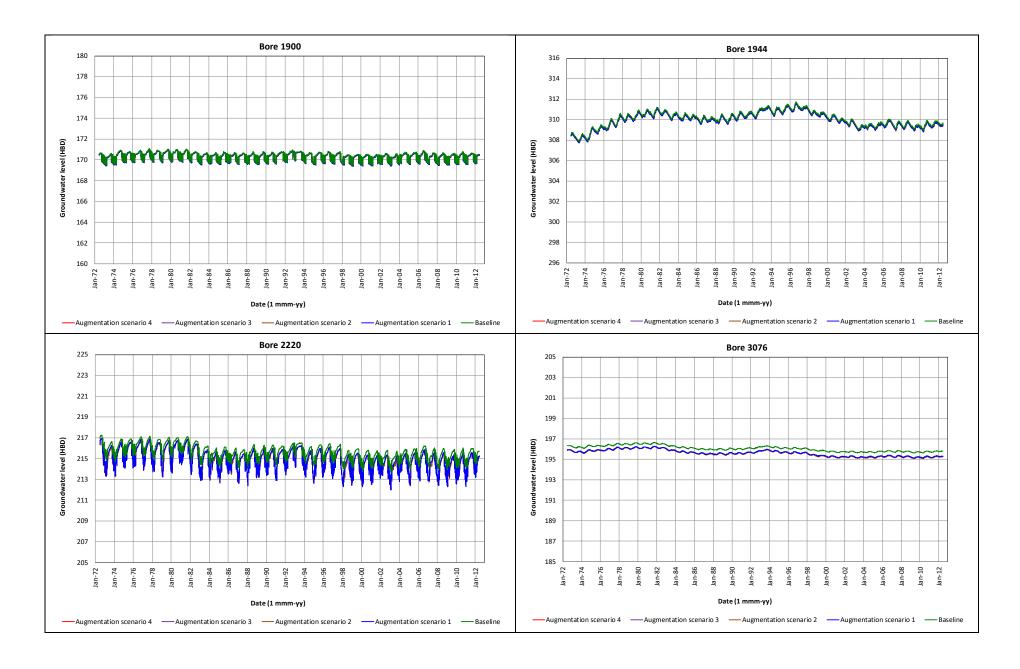
Specific storage for layers 2-10

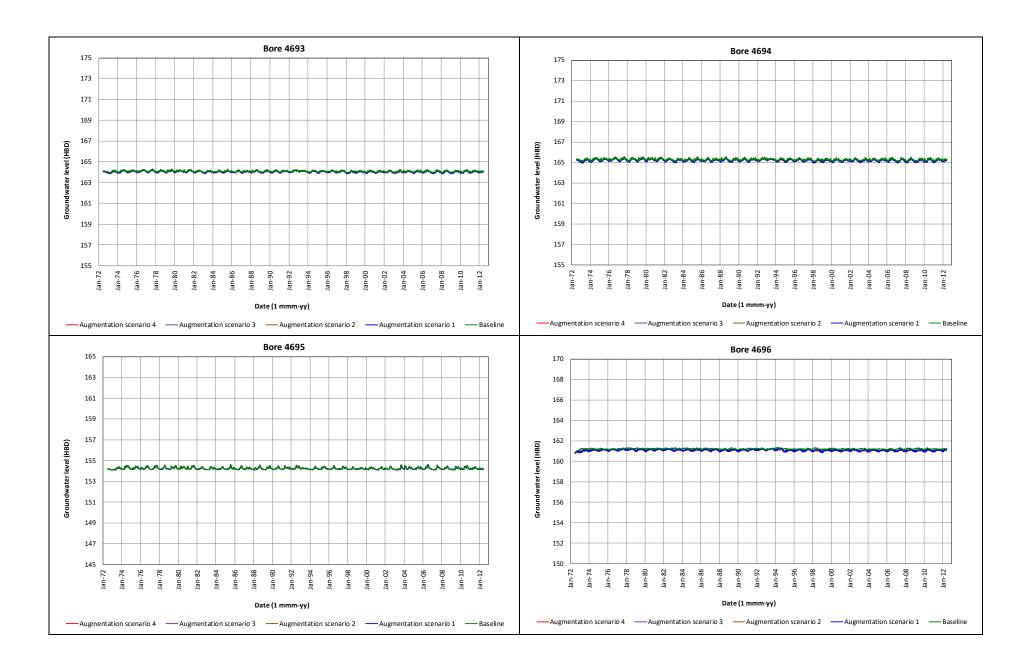
Appendix D: Groundwater level hydrographs for different scenarios

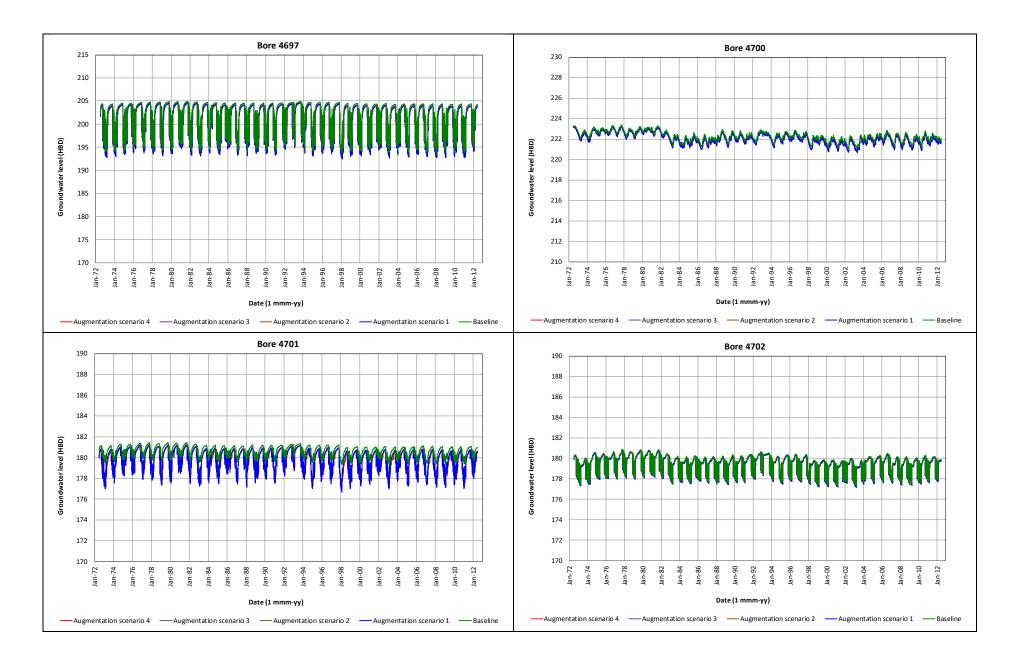


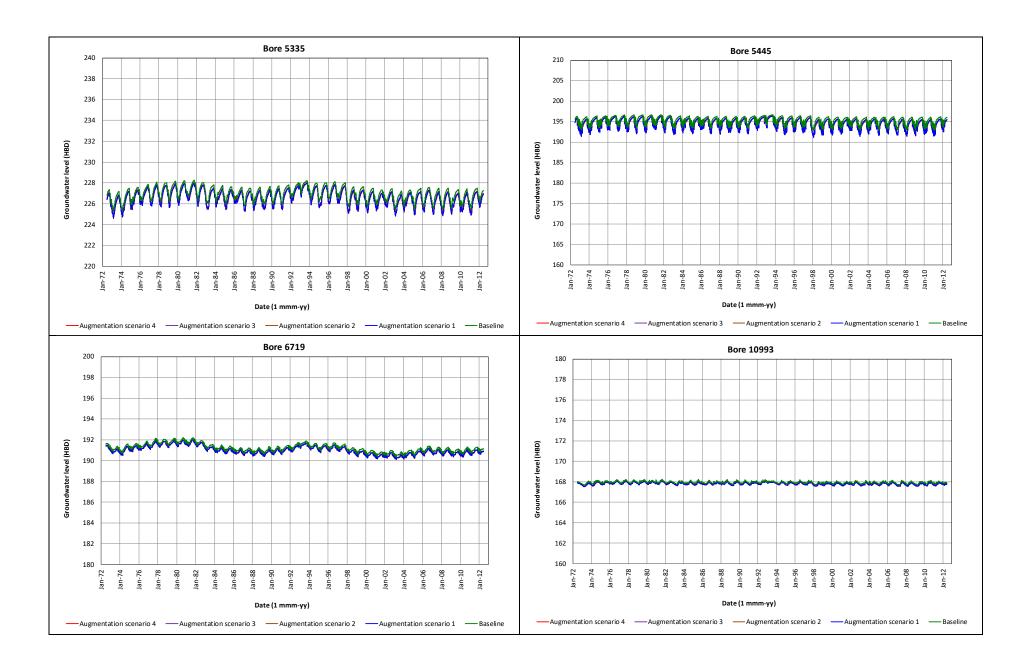


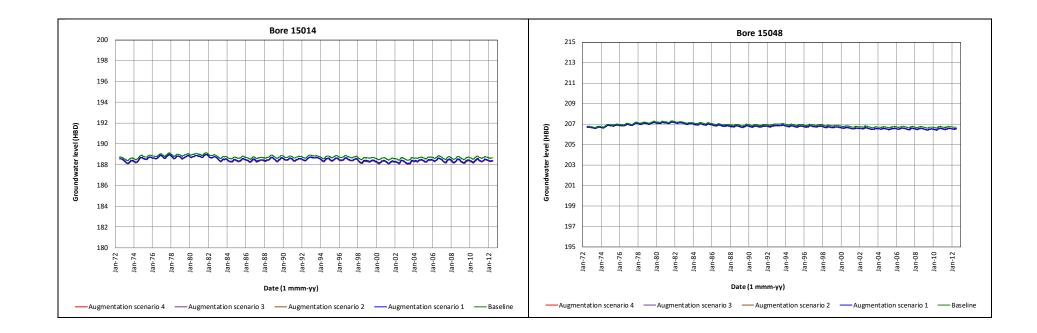












APPENDIX C – Wetlands and Surface Water Effects Assessment

Bay Geological Services Ltd

27 May, 2021 ref: BGS201_07

Consents Planner Hawkes Bay Regional Council Private Bag 6006 Napier 4110

Dear Sir/Madam,

RUATANIWHA BASIN TRANCHE 2 APPLICATION RESPONSE TO PDP'S PRELIMINARY REVIEW OF THE AQUALINC GROUNDWATER MODEL ITEMS 9 AND 10 ONLY

A review of the Aqualinc Ruataniwha Basin Tranche 2 groundwater modelling (Weir, 2020) by Pattle Delamore Partners Ltd (PDP) resulted in preliminary comments by PDP communicated by Julian Weir (Aqualinc) in a memorandum to the Tranche 2 Applicants dated 27 January, 2021. The following report provides additional information as a full response to Items 9 and 10 raised by PDP, which question potential effects on wetlands and minor waterbodies upstream of the augmentation sites as detailed below:

9. Effects on other river sites:

The model simulates river flows at many reaches within the basin, but only a few have measurements with which to calibrate. There are also several smaller streams and drains that are too small to model at the basin scale of the model. Furthermore, it is not feasible to augment into every stream, so there may be some reaches that are depleted and do not receive augmentation water until further downstream, below a confluence with another stream that does.

10. Effects on streams upstream of augmented rivers: This is the same as Item 9.

To assess the potential effects on wetlands and minor waterbodies, local knowledge (from Hawkes Bay Regional Council, Applicants and their Consultants) was sought to select representative stream and wetland sites for initial consideration. This was followed by a high-level assessment of the instream value of the waterbody as discussed in the following sections.

1. INTRODUCTION

The Ruataniwha Basin groundwater model (Weir, 2020) was developed by Aqualinc to predict effects on surface water flows as a result of the proposed Tranche 2 groundwater takes. However, the smaller river reaches and wetland areas were not included in the model due to the small scale of the features compared to the catchment scale of the model. Therefore, predicted effects on these waterbodies as a result of the takes were not specifically assessed.

There are multiple small-scale streams and drains in the catchment. In order to determine which sites could be considered significant and warrant further assessment, the Tranche 2 Applicants provided information regarding their local creeks, wetlands and streams to Bay Geological Services Ltd. These included reaches that typically dry up naturally in summer months and therefore present little instream value; to those that flow or are wet year-round. Further to this, local knowledge of the Ruataniwha Plains was used to inspect sites of interest, such as areas of recorded upwellings and spring flows. Not every reach or wetland was inspected as this was outside reasonable scope for the required outcome; however sites were selected in strategic parts of the basin, these being representative of effects in other reaches across the study area.

2. MAPPED SIGNIFICANT WETLAND AREAS

Guidance was also sought from Hawkes Bay Regional Council (HBRC) who were very helpful in providing information detailing wetland areas within southern and central Hawkes Bay. The council advised of the wetland State of the Environment (SOE) programme and the establishment (several years ago) of permanent water level recording sites installed to monitor potential effects of water abstraction on wetlands. Initially ten sites in the Tukituki River Catchment were selected with water-level loggers installed at these sites, and later other criteria were also selected in order to make the sites more suitable for the SOE programme. HBRC provided shapefiles of the SOE wetland data, from which Aqualinc generated a map included in Appendix A to show significant (SOE) wetlands.

The HBRC SOE wetlands are included in the Aqualinc map presented as Figure 1. The majority of the designated significant wetlands lie outside the study area, and are therefore outside of the influence of the proposed Tranche 2 groundwater takes. Only one significant wetland (near the upper reaches of the Mangatewai River) is identified within the project area, as displayed.

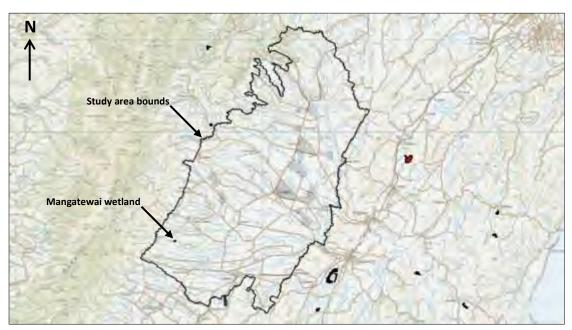


Figure 1. HBRC SOE wetland sites across central and southern Hawkes Bay (HBRC). Scale: 10 km grid.

3. REGIONAL GEOLOGY AND HYDROGEOLOGY

The NNE-SSW-oriented Ruataniwha Basin is fault-bound along the western Ranges, and lies adjacent to westward-dipping strata in the east which form the Raukawa Range. The development of the alluvial basin commenced with evacuation of the former inland sea as a result of tectonic uplift, followed by infilling of the basin with alluvial sediments. This resulted in a heterogeneous accumulation of sediments and multi-layered aquifer systems which comprise highly variable material ranging from gravels, sandstone, pumice and limestone (PDP 1999, Francis 2001).

Both groundwater-surface water interactions (as modelled by Baalousha, 2009a), and stream losses and gains (determined by Wood, 1998) as discussed in the 2012 Aqualinc report (Ballard, 2012) are presented in Figure 2. The boundary of the zero yield is observed to track alongside State Highway 50 (SH50) inferring that seepage from groundwater to rivers occurs east of the highway.

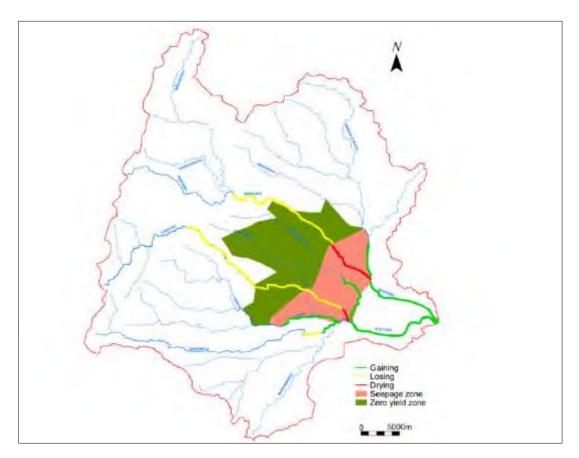


Figure 2. Aqualinc map displaying gain and loss patterns along with seepage zones in the Ruataniwha Basin (Ballard, 2012).

A review of published geological maps (Lee et al, 2011) reveals a series of active faults across the northeastern and southern periphery of the basin (refer Figure 3). The faults are typically orientated NNE with uplift to the west; however, they appear to terminate, or are not identified through the central part of the basin likely as a result of the extensive alluvial gravel cover sediments. If the faults were to track across the basin, it is likely that they would follow the regional NNE-SSW orientation. An overlay of the zero yield and seepage zones is added to the geological map as brown shading (zero yield) and blue shading (seepage) as presented in Figure 3.

The field sites are denoted as red dots on the geological map. The field inspection revealed several surface water sites which may be associated with deformation such as faulting or folding in response to tectonic stresses. These sites are denoted as blue circles on the geological map.

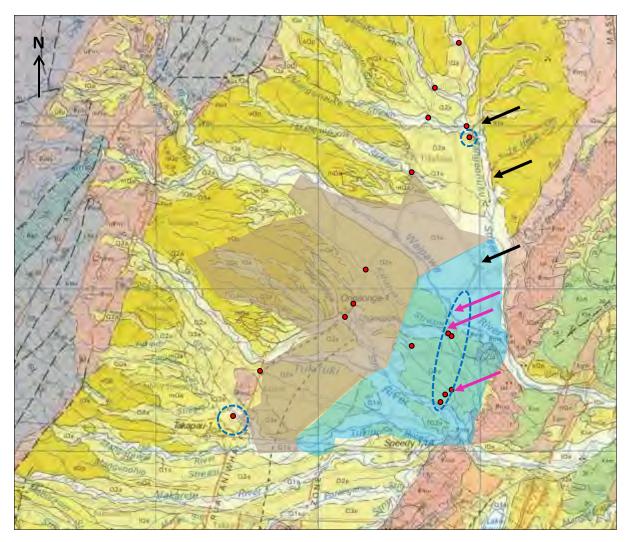


Figure 3. GNS 1:250,000 geological Qmap across Ruataniwha Plains (Lee et al, 2011). Scale 10 km grid. Red dots indicate field sites; black arrows indicate mapped faults/folds to the east. Pink arrows infer areas of possible deformation based on river/stream realignment; and blue circles denote wetland areas possibly associated with areas of tectonic deformation.

It could be inferred that the approximated NNE-SSW-oriented boundary between the zero yield and seepage zones follows the regional geologic trend. In addition, it is noted that a change in alignment of the Waipawa River and minor streams occurs in an area of inferred tectonic deformation (pink arrows).

4. FIELD INSPECTION OF WETLANDS & MINOR STREAMS

The Ruataniwha Basin wetland and minor streams/waterways were visited in March 2021 during a very dry period, in order to record the staus of the surface water features during a very dry period. These sites are shown as green circles, added to the Aqualinc (Weir, 2020) map as presented in Figure 4, which also displays the study area boundary and Applicants' property locations.

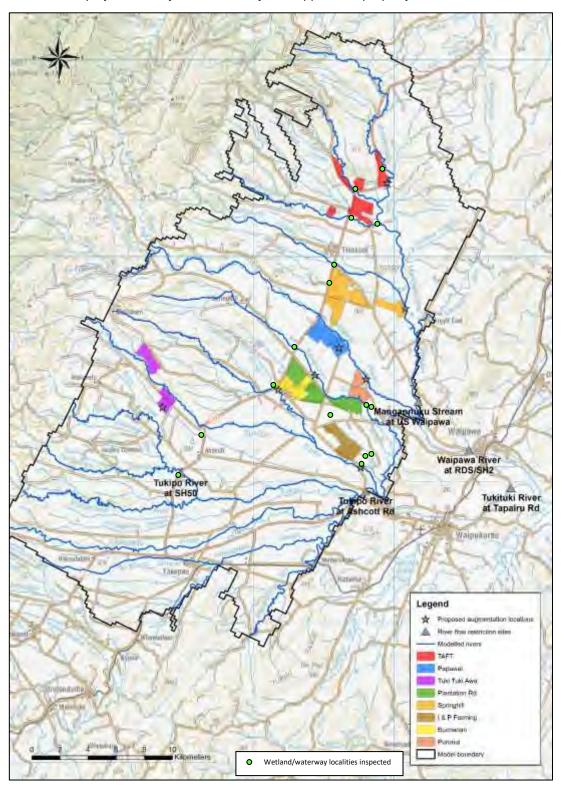


Figure 4. Surface water sites inspected (green circles) are displayed on the Aqualinc (Weir, 2020) map,

Details of surface water features inspected across the study area are listed in Table 1, which includes description and status of the waterbody as observed in the field by Bay Geological Services Ltd. The formation of the waterbody is also assessed, being either a natural feature or artificially formed such as a farm drain.

Table 1. Details of Ruataniwha Basin wetland and minor streams as inspected.

	Ru	ataniwha Basin We	etiands and water	ways	
Site	Grid ref (NZTM)	Description of Waterbody	Substrate/ Environment	Formation	Site Status (March 2021)
Manga-o-nuku Stream (03)	1898590 5593590	major stream	fine to med gravels	natural	flowing (very low rate)
Mangamate Stream (SH50)	1897330 5590690	major stream	fine to med gravels	natural	flowing (very low rate)
Mangamauku Stream (SH50)	1896750 5588935	major stream	fine to coarse gravels	natural	dry (no water evident)
Mangamauku Stream (Creek Rd)	1898650 5588265	major stream	fine to med gravels	natural	no obvious flow (pools of water)
Mangaoho Stream (SH50)	1895753 5585550	major stream	fine to med gravels	natural	dry (no water evident)
		I			
Chesterman bridge (trib Kahahakuri Stm)	1892945 5579505	minor stream	fine gravels	natural	dry (no water evident)
Kahahakuri Stm (SH50)	1892200 5577550	minor stream	fine gravels	natural	dry (no water evident)
Ongaonga Stream SH50, Ongaonga	1891630 5576620	major stream	fine to coarse gravels	natural	dry (no water evident)
Curama Dd	4007700				
Swamp Rd Wetland 01	1897700 5575900	medium size pond	unknown	man-made	high level
Swamp Rd drain (ex Waipawa River, trib Kahahakuri Stream)	1897940 5575841	very minor stream/drain	fine to med gravels	man-made	moderate flow rate (flows year-round)
Swamp Rd Wetland 02	1897910 5575600	medium size pond	unknown	natural	high level
Swamp Rd drain (trib Kahahakuri Stream)	1897625 5575875	very minor stream/drain	fine to med gravels	man-made	low flow rate (flows year-round)
Kahahakuri Stream (Swamp Rd)	1897590 5575810	minor stream	fine to med gravels	natural	mod flow rate (flows year-round)
Black Stream Source (Fairfield Rd)	1895370 5574535	swale	grassed paddock	natural	damp underfoot (rare ponding)
top Black Stream (pond off Fairfield Rd)	1895470 5574465	small pond	unknown	man-made	low level (wet all year)
,					, , ,
Woolshed pond, lower Black Stream	1898370 5571990	medium size pond	unknown	man-made	large pond
lower Black Stream (Hobin Road)	1898515 5571795	very minor stream	v fine to fine gravels/sand	natural	flows year-round (very low rate)
lower Black Stream Trib 01 (Hobin Road)	1897795 5571805	stream	v fine to fine gravels/sand	natural	flowing (moderate rate)
Swan pond, lower Black Stream Trib 01	1897710 5571690	medium size pond	silt and muds	man-made	high level
drain at lower Black Stream Trib (02)	1898030 5571370	very minor stream/drain	silt and muds minor pebbles	man-made	flowing (very low rate)
Ashaott Dridge	100075		fina		.d
Ashcott Bridge (unnamed stream)	1886375 5572620	minor stream	fine gravels	natural	dry (no water evident)
SH50 wetland (adjacent Tukipo Rv)	1884665 5570615	small pond	grassed paddock	natural	moderate level

The sites are described in detail as follows:

4.1 Manga-o-nuku Stream (03), SH50, Tikokino

The minor stream is located near the northern bounds of the study area. The stream flow was assessed as very low in March, 2021 (refer Figure 5).



Figure 5. Manga-o-nuku Stream (03), SH50, Tikokino (view eastward). Very low flow observed.

4.2 Mangamate Stream, SH50, Tikokino

The minor stream is located near the northern bounds of the study area. The stream flow was assessed as very low in March, 2021 (refer Figure 6).

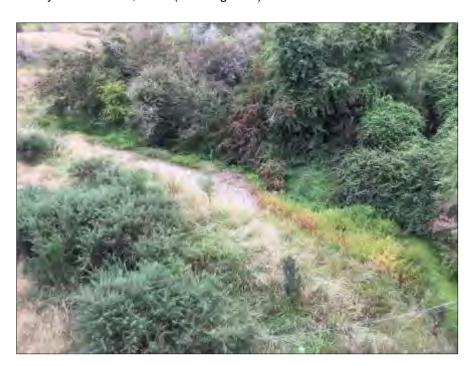


Figure 6. Mangamate Stream, SH50, Tikokino (view westward). Very low flow observed.

4.3 Mangamauku Stream, SH50, Tikokino

The minor stream is located near the northern bounds of the study area. No standing nor flowing surface water was observed along the stream bed in March, 2021 (refer Figure 7).



Figure 7. Mangamauku Stream, SH50, Tikokino (L = view west; R = view east). No surface water observed.

4.4 Swamp Road wetlands, Ongaonga

Two wetlands were inspected off Swamp Road, Ongaonga which are located centrally within the basin. Both wetlands appeared full, with Wetland 02 fed by moderately high flow rate, unnamed waterway (refer Figure 8).



Figure 8. Gunson Wetland 01 (L) and 02 (R), Swamp Road, Ongaonga.

An HBRC online wells map is presented as Figure 9, marked-up to illustrate waterways and the two wetlands south of the Waipawa River, and immediately north of the Kahahakuri Stream. The gaining reach of the Kahahakuri Stream lies to the south of the Waipawa River, from which river water is likely flowing subsurface and emanating across the property and draining into the stream.

The waterway draining into Wetland 02 is shown in Figure 10.



Figure 9. HBRC online map showing the Swamp Road wetlands and surface water features (HBRC).



Figure 10. Unnamed waterway flowing into Wetland 02, Swamp Road, Ongaonga.

4.5 Kahahakuri Stream, Swamp Road, Ongaonga

The Kahahakuri Steam flows east then southeast along the southern boundary of the Gunson property (refer Figures 11 and 12). The stream flows year-round across this reach, being fed in part by extensive drains and waterways that track across farm that lies to the south of the Waipawa River. The 2012 Aqualinc report (Ballard, 2012) indicates that the Kahahakuri Stream becomes a gaining reach at this point.



Figure 11. Kahahakuri Stream gaining reach, Swamp Road, Ongaonga.



Figure 12. Kahahakuri Stream, gaining reach, Swamp Road, Ongaonga.

4.6 Black Stream source, Fairfield Road, Ongaonga

The source of the Black Stream is located north off Fairfield Road, Ongaonga and is observed in the field as a grassed swale as displayed in Figure 13. The swale was damp underfoot, possibly due to the man-made dam located immediately to the south as shown in Figures 13 and 14 and discussed in Section 4.7.



Figure 13. HBRC Wells map (L) showing the Black Stream source off Fairfield Road; and photo of source (R), Fairfield Road, Ongaonga.



Figure 14. Black Stream pond near source, Fairfield Road, Ongaonga.

4.7 Black Stream, off Hobin Road, Ongaonga

A large pond exists on a property approximately 100 m west of Hobin Road which lies immediately adjacent to the Black Stream that flows year-round at this point.

4.8 Black Stream tributary 01, Hobin Road, Ongaonga

The tributary (01) of the Black Stream appears to originate south of Fairfield Road. The waterway is mapped as a drain at the head of a willow plantation on a property west off Hobin Road as shown in Figure 17, which was observed as a very slow to slow flow, to being damp underfoot within the willows.

Further west along an offshoot of the tributary, a moderately flow rate was observed, downstream of a large pond on the property. The flows are likely sourced from pond seepage.



Figure 17. Black Stream tributary, Hobin Road, Ongaonga.

4.9 Black Stream tributary 02, Ongaonga-Waipukurau Road, Ongaonga

The tributary (02) of the Black Stream appears to originate on land immediately east of Ongaonga-Waipukurau Road. The waterway was observed as a very slow flow with deep pools evident away from the culvert near the road.

4.10 State Highway 50 Wetland (south of the Tukipo River)

The wetland is located east of SH50 at the foot of a moderate embankment (refer Figure 18), observed as an abandoned meander, elevated several metres above the nearby Tukipo River. Groundwater seeps through the extensive gravel terraces and a minor waterway to the northwest.



Figure 18. SH50 wetland (south of the Tukipo River).

4.11 Streams originating west of SH50

It was noted during the field visit that the following streams originating west of SH50 and tracking eastward across the highway and into the Ruataniwha Plains were dry with no water ponding evident. Photos of the streams are presented in Figures 19 to 23.

- Mangaoho Stream (Tikokino)
- Tributary Kahahakuri Stream (Chestermans Bridge, Ongaonga);
- Kahahakuri Stream (Ongaonga);
- Ongaonga Stream (Ongaonga); and
- Unnamed Stream at Ashcott Bridge (Ashcott, Takapau).



Figure 19. Mangaoho Stream, Tikokino (view NW);

Figure 20. Chestermans Bridge. Ongaonga (view NW).



Figure 21. Kahahakuri Stream, Ongaonga (view NW);

Figure 22. Ongaonga Stream, Ongaonga (view SE).



Figure 23. Unnamed Stream at Ashcott bridge, Ashcott, Takapau (view SE).

4.12 Upper reaches of the Mangatewai River, Boyle Road, Ashley Clinton

The one significant wetland that occurs within the study area is identified near the upper reaches of the Mangatewai River, which is upstream and well outside of any area of influence of the proposed Tranche 2 takes (refer Appendix A, Map 3). It was noted at the time of field inspection, that the Mangatewai River at Boyle Road was almost dry (refer Figure 24); however, the wetland was not inspected due to limited access.



Figure 24. Upper reaches of the Mangatewai River, Boyle Road, Ashley Clinton.

5. PREDICTED EFFECTS ON WETLANDS AND MINIOR STREAMS

The Aqualinc model was developed to predict effects of the proposed Tranche 2 take on major river reaches only, and therefore the potential effects on wetlands and minor waterways were not directly predicted. However, in discussion with Aqualinc (J. Weir, 2021 pers comm), effects can be estimated by considering the change in shallow groundwater level predicted by the model at wetland locations and, with local knowledge, estimate a projected impact. This is relevant for wetlands, springs and surface water locations upstream of any reach receiving augmentation water. Surface water reaches downstream of augmentation sites will have the same or better flows during extreme dry periods (due to the added augmentation water), even if nearby groundwater levels are a little lower.

The modelled effect on shallow groundwater, as presented in the Aqualinc report (Weir, 2020), is presented in Figure 25. The locations inspected in the field were overlain onto the map, and the predicted changes in shallow groundwater levels were assigned. These predictions estimate a point in time during extreme dry conditions, being a very occasional event.

The predicted changes in shallow groundwater levels as modelled by Aqualinc (Weir, 2020) are included in Table 2, along with an estimate of how this may affect surface water bodies as inspected in the field. It is anticipated that this assessment reflects basin-wide changes.

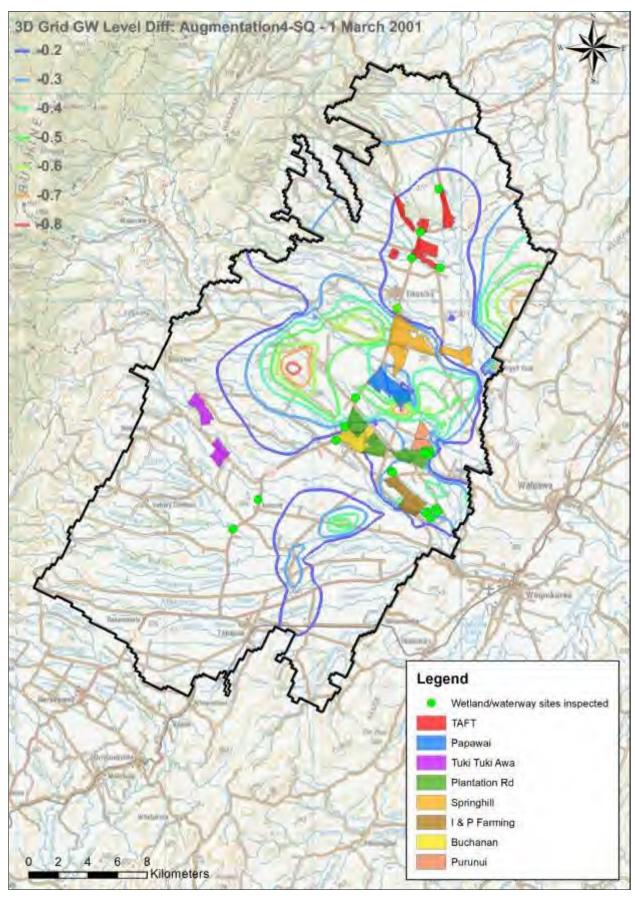


Figure 25. The Aqualinc map displaying modelled effect on shallow groundwater as an extreme event (Weir, 2020) showing Applicants' properties and wetland/waterway sites visited.

Table 2. Details of Ruataniwha Basin surface water features inspected and estimated effects on shallow groundwater applied to localised wetlands and minor streams.

Basin Location	Site	Up/downstream of augmentation	Site Status (March 2021)	Formation	Predicted Effect on shallow GW	Predicted change to SW feature
North	Manga-o-nuku Stream (03)	upstream	flowing (very low rate)	natural	-0.00	negligible
	Mangamate Stream (SH50)	upstream	flowing (very low rate)	natural	-0.00	negligible
	Mangamauku Stream (SH50)	upstream	dry (no water evident)	natural	-0.10	negligible
	Mangamauku Stream (Creek Rd)	downstream	no obvious flow (pools of water)	natural	-0.00	negligible
	Mangaoho Stream (SH50)	upstream	major stream	natural	-0.10	negligible
	Manga-o-nuku Stream (03)	upstream	flowing (very low rate)	natural	-0.00	negligible
West (SH50)	Chesterman bridge (trib Kahahakuri Stm)	upstream	dry (no water evident)	natural	-0.50	likely negligib (potential to d earlier)
	Kahahakuri Stm (SH50)	upstream	dry (no water evident)	natural	-0.30	likely negligib (potential to d earlier)
	Ongaonga Stream SH50, Ongaonga	upstream	dry (no water evident)	natural	-0.20	likely negligib (potential to d earlier)
	Swamp Rd					
	Wetland 01	downstream	high level	man-made	-0.40	negligible
	Swamp Rd drain (ex Waipawa River, trib Kahahakuri Stream)	downstream	moderate flow rate (flows all year)	man-made	-0.40	negligible
	Swamp Rd Wetland 02	downstream	high level	natural	-0.40	negligible
Central	Swamp Rd drain (trib Kahahakuri Stream)	downstream	low flow rate (flows year- round)	man-made	-0.40	likely measurable lowering of drain water level
	Kahahakuri Stream (Swamp Rd)	downstream	mod flow rate (flows all year)	natural	-0.30	possible mine lowering of water level
	Black Stream Source (Fairfield Rd)	upstream	damp underfoot (rare ponding)	natural	-0.30	potential to dry earlier
	top Black Stream (pond off Fairfield Rd)	upstream	low level (wet all year)	man-made	-0.40	likely measurable lowering of da water level
East	Woolshed pond, lower Black Stream	upstream	large pond	man-made	-0.40	minor lowering
	lower Black Stream (Hobin Road)	upstream	very low rate (flows all year)	natural	-0.30	potential to reduce flow
	lower Black Stream Trib 01 (Hobin Road)	upstream	flowing water (moderate rate)	natural	-0.30	minor lowering of water leve
	Swan pond, lower Black Stream Trib 01	upstream	high level	man-made	-0.30	minor lowering of water leve
	drain at lower Black Stream Trib (02)	downstream	flowing (very low rate)	man-made	-0.30	likely measurable lowering of water level

est	Ashcott Bridge (unnamed stream)	upstream	dry (no water evident)	natural	-0.10	negligible
Southw	SH50 wetland (adjacent Tukipo Rv)	upstream	moderate level	natural	-0.10	negligible

To assess what level of value the waterbody presents and whether the reach requires protecting, the formation of the water feature, whether artificially constructed or a naturally occurring stream/wetland is taken into consideration. Sites such as man-made drains provide little value other than collecting and redirecting rainfall runoff and shallow groundwater from across farmland, and therefore do not require protecting. Naturally occurring streams may have a greater value and the flows therefore may require protection.

In applying the estimated effects on sites inspected in the field, the following changes in water levels are predicted:

- North of study area: the predicted effect on surface water features is estimated to be negligible and not likely to affect minor streams and wetlands.
- West of SH50: the predicted effects on streams that typically dry out during the summer are
 thought to be negligible; however, it is possible that streams may experience dry conditions
 slightly earlier and/or resume flowing again a little later in the season (likely by a matter of a
 few days difference).
- Central and eastern areas: large wetlands and minor streams were observed in the field which appear to be fed by a possible subsurface divergent channel of the Waipawa River upwelling in areas of lower elevation, where changes in lithology result poor confinement. Modelled effects on shallow groundwater as a result of the proposed Tranche-2 takes are greatest in this area. However, anecdotal evidence and field inspections suggest that the majority of the surface water features flow year-round through formed drains.

Therefore, the estimated changes in surface water levels are unlikely to result in significant effects on the majority of the naturally occurring waterways; although in places the effects will be measurable. The estimated effect across the lower reaches of the Black Stream may be such that the low flow rate observed in March 2021 could be reduced to a trickle.

• Southwest area: the predicted effects on streams that typically dry out during the summer are thought to be negligible; however, flows may be a little less during dry periods for sites upstream of any augmentation site. It is possible that streams may experience dry conditions slightly earlier and/or start flowing a little later in the season (likely by a matter of a few days difference) along with minor drying effects on the wetland areas inspected.

6. SUMMARY

The recent modelling completed by Aqualinc (Weir, 2020) predicted effects of surface water flows across the major river reaches that flow through the Ruataniwha Basin project area, as a result of abstracting the Tranche 2 allocation. However, wetlands and minor streams were not included in the modelling due to the catchment scale of the model. Therefore, this study sought to estimate the effect on wetlands and minor waterways as a result of predicted changes in shallow groundwater from the abstraction of Tranche-2 volumes as modelled by Aqualinc (Weir, 2020).

Following discussions with HBRC, shapefiles of wetlands were provided by Council from which Aqualinc generated a map which displays the significant wetlands monitored by HBRC as part of the SOE programme. In addition, Tranche-2 Applicants were asked to provide information on surface water features across their properties. To assess size, formation (natural/man-made) and state (dry/wet) of the recorded surface water features, field inspections were completed in March 2021 during a very dry period to record locations and the state of wetlands and minor streams across the basin. Not all localities were inspected due to the extensive area and project scope constraints; however, a robust representation of surface water example sites is provided. Assessing the formation of the water feature is critical in evaluating the level of protection required, where sites such as man-made drains provide little value; however, naturally occurring streams may require protection.

Using the Aqualinc 2020 assessment which predicted effect on shallow groundwater, and applying the estimated changes to sites inspected in the field, the following effects on surface water levels are predicted. It must be noted that the modelled effect is a prediction at a point in time during an extreme dry event, not a typical irrigation year:

- North of study area: the predicted effect on surface water features is estimated to be negligible and not likely to affect wetlands and minor streams.
- West of SH50: the predicted effects on streams that typically dry out during the summer are
 thought to be negligible; however, it is possible that streams may experience dry conditions
 slightly earlier and/or resume flowing again a little later in the season (by a matter of a few
 days).
- Central and eastern areas: large wetlands and minor streams were observed in the field which appear to be fed by a possible subsurface divergent channel of the Waipawa River upwelling in areas of lower elevation, where changes in lithology result poor confinement. Modelled effects on shallow groundwater as a result of the proposed Tranche-2 takes are greatest in this area. However, anecdotal evidence and field inspections suggest that the majority of the surface water features flow year-round through formed drains. Therefore, the estimated changes in surface water levels are unlikely to result in significant effects on the majority of the naturally occurring waterways; although in places the effects will be measurable. The estimated effect across the lower reaches of the Black Stream may be such that the low flow rate observed in March 2021 could be reduced to a trickle.
- Southwest area: the predicted effects on streams that typically dry out during the summer are
 thought to be negligible; however, it is possible that streams may experience dry conditions
 slightly earlier and/or start flowing again a little later in the season (by a matter of a few days).
 Therefore, minor effects are anticipated on the wetland areas inspected.

7. REFERENCES

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Report Limitations

This letter report is written based on conditions as they existed at the time of the study, and there is no interpretation made on potential changes that may occur across the sites. Subsurface conditions may exist across the project area that are not able to be detected or revealed by the investigation within the scope of the study, and are therefore not taken into account.

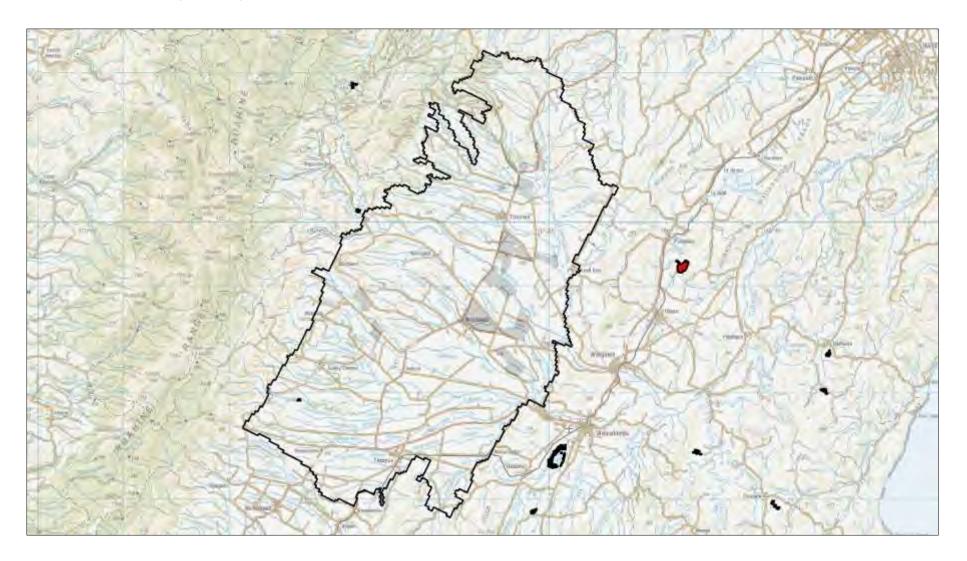
APPENDICES

APPENDIX A

Topo map illustrating SOE significant wetland areas

(data sourced from HBRC and map generated by Aqualinc)

A1.: Topo map illustrating SOE (significant) Wetland sites across the Central & Southern Hawkes Bay area (data sourced from HBRC)



APPENDIX D – Well Interference Assessment Report



RUATANIWHA BASIN TRANCHE 2 IRRIGATION WATER PERMIT CONSENT APPLICATION

-ASSESSMENT OF WELL INTERFERENCE EFFECTS

CENTRAL HAWKE'S BAY

PROJECT NO. J19220-REP-01 T2-WI

PREPARED FOR COLLABORATIVE GROUP

PREPARED BY SUSAN RABBITTE

DECEMBER 2020





Report prepared by:

Susan Rabbitte
Principal Geologist/Hydrogeologist

Sabatte

Report QA/QC review by:

Ivan Berra Geologist

PROJECT: J19220 REPORT STATUS: FINAL



EXECUTIVE SUMMARY

Lattey Group (Lattey) was engaged by the Tranche 2 (T2) applicants' collaborative group to prepare a basin wide well interference assessment associated with the proposed T2 ground water abstraction.

For this assessment all the wells recorded within the Ruataniwha Basin are considered. There are many wells, and these wells were divided into groups based on their recorded total well depths. The initial focus is on the wells within the 0m to 50m depth range and these are assessed against the numerical groundwater model shallow (layer 1) impact predictions. To assess the potential for impact on deeper wells, the modelled drawdown impact at a deeper level (layer 6) was reviewed separately against the deeper wells. It was not possible to fully consider wells of unknown depth but, if it is assumed that they are sufficiently deep to enable "efficient" taking of water then they are unlikely to be adversely impacted.

Consideration of aquifer parameters from existing well aquifer pump testing supports the outputs from the numerical model and increases confidence in the drawdown predictions made for the well locations assessed in this report.

A methodology for the assessment is presented. In some cases, gaps within the data provided by the Hawkes Bay Regional Council (HBRC) database are filled. Where this has occurred, the assumed value is identified with red text, for example Static Water Levels (SWL) and top screen heights. The assumed SWL are a numerical average of all the SWL available for each depth group and the top screen heights where absent are assigned a value equivalent to the total well depth.

The two contour maps provided from the numerical modelling represent water level differences associated with the preferred scenario for the T2 water abstraction, for both shallow and deep layers within the aquifer. Values were interpolated from these contour maps and applied to specific basin well locations.

Seasonal variation was predicted based on analysis of the HBRC State of the Environment (SOE) monitoring wells network. A maximum value range was calculated based on the last 5 years of data. Where sufficient datapoints were available this data was contoured, and an interpolation of the seasonal variation value was applied to the individual well locations. Where the data was insufficient the available value or an average was applied.

Wells were initially flagged on the assessment spreadsheet for each depth group. They were then separated into those that were flagged because they are potentially of insufficient depth or otherwise classified as "inefficient" wells and those that might genuinely be adversely impacted by the proposed abstraction. Some could be further discounted from likely impact based on their recorded use or information from other recorded details.

Of a total of 657 wells (this value excludes the 46 unknown depth wells) that were individually assessed 94 were initially flagged in the well groups and 24 remain flagged following some further consideration. Of these flagged wells 14 are in the depth range <50m and 10 are >50m deep.

Further investigation is required to assess whether they will be adversely affected or not, and this may involve contacting the well owners or visiting the well locations. However, given that almost all of the wells assessed (i.e. 96%) are not considered as adversely affected, it is likely that this will also be the case for the remaining wells.

PROJECT: J19220 REPORT STATUS: FINAL PAGE: |



TABLE OF CONTENTS

EXECUT	IVE SUMMARY	.1
1.0 1.1 1.2 1.3 1.4	INTRODUCTION	.1 .3 .4
2.0 2.1 2.2	RUATANIWHA BASIN WELLS All Wells DATA Seasonal Variation Values	.7
3.0 3.1 3.2 3.3 3.4 3.5 3.6	RESULTS OF WELL INTERFERENCE ASSESSMENT Results - Well Group 0 - 9.99m Results - Well Group 10 - 19.99m Results - Well Group 20 - 29.99m Results - Well Group 30 - 39.99m & 40 - 49.99m Results - Well Group +50m Summary - All Wells Groups	5 6 6 6
4.0	CONCLUSION	22
5.0	REFERENCES	22
Figure 2 Figure 3 Figure 4 Figure 7 Figure 7 Figure 7 Figure 1 Figure 1 Figure 1 Figure 1 Figure 1 Figure 1 Figure 1 Figure 1	: Tranche 2 Applicants' Farm Locations (Aqualinc, 2020) : Difference in Shallow (Layer 1) Groundwater Level - Scenario 4 (Aqualinc, 2020). : Difference in Deep (Layer 6) Groundwater Level - Scenario 4 (Aqualinc, 2020) : Key Features for Assessing Well Interference	3 .4 .6 .8 .9 .0 .1 .2 .1 .3 .4 .4 .8 .9 .0 .1 .2 .1 .3 .4 .4 .8 .9 .0 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1
Table 2 Table 3 Table 4	: Well Groups Assessed	3 7 7
Append	dices dix 1: Seasonal Variation Values	25



1.0 INTRODUCTION

Lattey Group (Lattey) was engaged by the Tranche 2 Collaborative Group (T2 Group) to prepare an assessment of the impact of the proposed Tranche 2 (T2) water take on existing wells within the Ruataniwha Basin area. This is essentially a well interference assessment, that combines both the shallow (layer 1) and deeper (layer 6) groundwater level difference outputs from the numerical groundwater model, with data for the existing basin wells and seasonal water level variations. The numerical model outputs selected relate to a specific water use scenario developed for the collaborative group to optimally manage the impacts of the taking on river flows via augmentation.

This report is required to support the application for groundwater abstraction under the Ruataniwha Basin Tranche 2 allocation regime. It identifies wells within the basin area that may be impacted by the T2 abstraction and that may require further consideration or limited notification in relation to the consent application process. The scope of works and information provided by this report includes:

- Summary of the consent application;
- Description of the assessment methodology applied;
- Review of numerical modelling outputs;
- Identification of the basin wells;
- Influence of seasonal variation: and
- Assessment of results.

1.1 CONSENT APPLICATION DETAILS

Hawke's Bay Regional Council (HBRC) completed the process to finalise a change to their Regional Resource Management Plan (RRMP) within the Tukituki River Catchment. This plan change is known as Plan Change 6 (PC6). One of the outcomes of the plan change was the inclusion of a Tranche 2 groundwater allocation limit for Zones 2 and 3 of the Ruataniwha Basin of 15M m³. The allocation is available primarily for irrigation use, however, to access it there is a requirement to augment river flows to off-set the impacts on them of the T2 taking. The abstraction must be sourced from wells screened at depths greater than 50m. The concept is to use aquifer storage as a buffer and to support river flows during times of low flow. However, this will still result in greater volumes of water being abstracted from the basin and over the longer term the water table will reach a new lower dynamic equilibrium.

As part of the PC6 decision making process a 3D numerical groundwater model was developed (Weir, 2013). This model was recently updated and used to test the hydraulic response of the groundwater and surface water system in the basin from multiple proposed T2 groundwater take applications (Aqualinc, 2020). This revised model then informed the augmentation requirements and was used to identify an optimised scenario that provides for mitigation of the adverse impacts on river flows whilst maximising irrigation use.

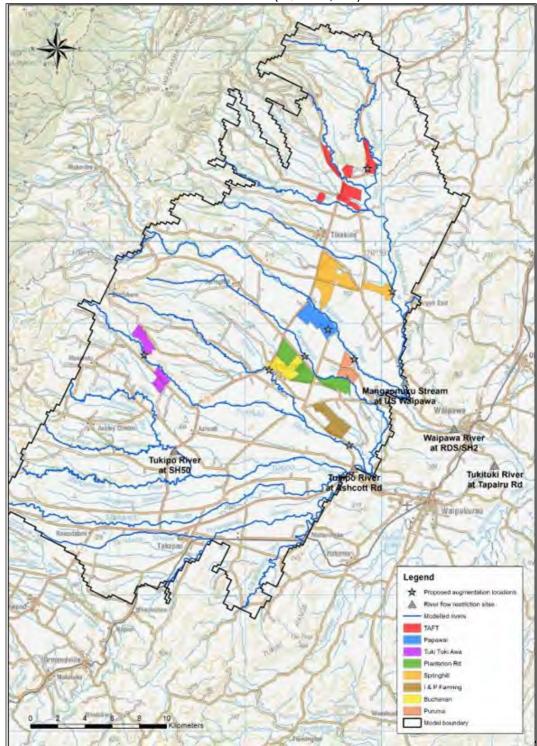
The effects of the T2 taking are basin wide and mitigation requires a distributed approach to augmentation locations. There is a total of eight applicants and to ensure effective mitigation means a collaborative approach is required. The applicants are:

- Te Awahohonu Forest Trust (TAFT);
- Papawai Partnership;
- Tukituki Awa;
- Plantation Road Dairies;
- Springhill Dairies;
- I & P Farming;
- Buchanan Trust No. 2; and
- Purunui Trust.



Together these applicants have applied for the full 15M m³ of groundwater allocation. The locations of the applicants' properties are shown on the map, Figure 1, along with the model boundary and augmentation locations.

FIGURE 1: TRANCHE 2 APPLICANTS' FARM LOCATIONS (AQUALINC, 2020)





1.2 RESULTS OF THE NUMERICAL MODELLING

The numerical groundwater modelling assessed the impact on river flows of each individual applicant in terms of change in average flows (L/s) and change in the mean 7-day annual low flow (MALF) (L/s). Following this, several scenarios were developed to consider the combined effects of all applicants together with augmentation. A total of four potential abstraction augmentation scenarios were developed and compared against a Status Quo.

For the purposes of this assessment Augmentation Scenario 4 is the preferred solution. This includes for Tuki Tuki Awa to abstract T2 water only to fill gaps in their surface water taking with subsequent discharge to the Tukituki River. This scenario represents the best optimisation of the volume of water taken for irrigation versus that taken for augmentation with a 15% reduction in augmentation discharge compared to Scenario 3, allowing a corresponding increase in the irrigated areas.

A map of the groundwater level difference between the Status Quo and Augmentation Scenario 4 was presented (Aqualinc, 2020), this map is shown here in Figure 2. The map shows how groundwater levels are predicted to change spatially during dry periods. The shallow groundwater levels are predicted to lower up to a maximum of 0.8m, and this is focussed in areas of greatest abstraction i.e. near the bulk of the applicant's properties and to the northwest of Ongaonga township. Elsewhere, shallow groundwater levels are predicted to change less than 0.3m (Aqualinc, 2020).

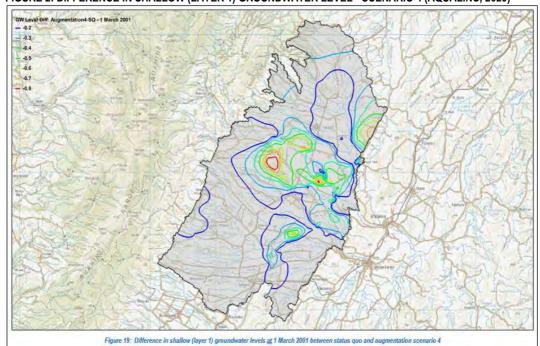


FIGURE 2: DIFFERENCE IN SHALLOW (LAYER 1) GROUNDWATER LEVEL - SCENARIO 4 (AQUALINC, 2020)

This predicted drawdown contour map for shallow (layer 1) groundwater levels can be used to make well specific predictions for the shallow (<50m) deep well locations within the Ruataniwha Basin. These individual well locations can then be assessed to understand if the magnitude of well interference predicted is likely to have an adverse impact on them or not.

In addition to the shallow (layer 1) map, information was also provided from the numerical groundwater model regarding deep (layer 6) drawdown predicted effects, this map is shown in Figure 3. The information provided by this output is assessed against the deep (>50m) well locations, to assess the potential impacts on them.

The layer 6 model output shows zero, or almost zero, difference in groundwater level in the southern portion of the Ruataniwha Basin. The water level difference increases northward to around 8m or more in the north-eastern most area.



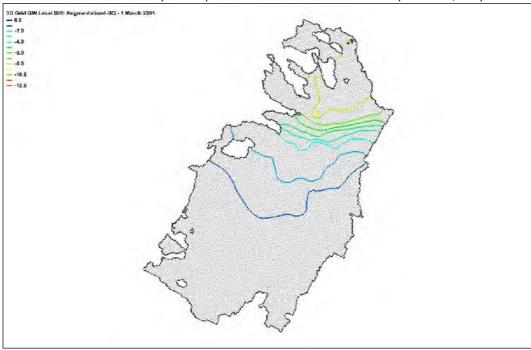


FIGURE 3: DIFFERENCE IN DEEP (LAYER 6) GROUNDWATER LEVEL - SCENARIO 4 (AQUALINC, 2020)

1.3 RUATANIWHA BASIN TRANSMISSIVITY VALUES

A review of the locally derived aquifer parameters i.e. those calculated from aquifer pump testing within the basin was made to aid understanding of the expected basin response to the proposed T2 abstraction. This review was carried out by Bay Geological Services Limited and a copy of their full report is provided as Appendix 2.

The report focuses on aquifer parameters calculated for wells >50m in depth as this represents the minimum top screen height for the existing and new T2 abstraction and augmentation points.

In summary, the Ruataniwha Basin comprises a series of spatially distributed and discontinuous alluvial aquifer deposits with variable properties. Meaning there are no discrete gravel aquifers present. High transmissivity (T) values >1,500m²/d occur at depths of 40m to 60m below ground level (bgl). There are 4 deeper wells (>75m) with equally high T values, located between the Kahahakuri Stream and the Waipawa River, probably a result of well-developed, clean gravel paleo-channel deposits adjacent to the river.

Significantly lower T values are reported south of the Tukituki River. Likely because of a less active depositional setting. Resulting in a meandering river with a channel off-set by tectonic activity and subsequently less well-developed gravel rich paleo-channels.

Storativity (S) values reduce with increasing depth and confinement. Values of 0.001 to 0.002 are common in the basin centre at 40m to 75m bgl, lowering to 0.0001 to 0.00001 in the north and west. At depths of >75m the S value is generally between 0.0005 to 0.00001.

This information is consistent with what might be expected in this geological setting where tectonic basin development results in alluvial in-filling. The bulk of the historically successful wells are focused in the basin centre with the more marginal performances reported toward the boundaries.

The bulk of the T2 abstraction is also focused in these higher performing areas targeting the deep paleo-channels. Where the T and S values are higher the aquifers can better sustain abstraction with lower surrounding impacts. This is because the transmission of effect is more rapidly and widely distributed within the basin at this level, more is gained from the higher storage component and recharge is equally rapid. This is evident in Figure 3 where despite the greater proportion of T2 abstraction occurring in the central area of the basin the water level difference is minimal. However, as the effects propagate northwards, and combined with the abstractions at the northern end of the basin, the lower aquifer parameters are less able to distribute effects and maintain recharge, a greater decline occurs. South of the Tukituki River there is no T2 abstraction and this is reflected in the absence of water level





difference effects at this deep level in this area. However, over time as the effects continue to spread and propagate to the surface the more basin wide impact is apparent and this is reflected in the water level difference contour map provided by the numerical model and shown in Figure 2.

1.4 WELL INTERFERENCE ASSESSMENT METHODOLOGY

When considering the potential for adverse impacts on an existing well it is necessary to understand the current availability of water within that well based on the water column. The amount of available water can then be compared with the magnitude of predicted effects associated with the new taking at that well location. In some cases, there will be water available and in others the existing situation may indicated that the well potentially already has issues with abstraction and is considered "inefficient".

If the predicted effect of the take represents only a small proportion of the water that remains available at the location of interest, then it is unlikely that a significant adverse effect will occur. However, if the predicted impact makes up a large proportion of the remaining water column of existing wells, then they may be impacted such that access to the water supply by the user is reduced.

In terms of the HBRC Regional Resource Management Plan (RRMP) Policy 77 (POL77) identifies a requirement for "efficient taking" of groundwater, as it specifies that the groundwater be managed so "that existing efficient groundwater takes are not disadvantages by new takes". Efficient taking is further defined in the RRMP as abstraction by a bore which penetrates the aquifer from which water is being drawn at a depth sufficient to enable water to be drawn all year (i.e. the bore depth is below the range of seasonal fluctuations in groundwater level), with the bore being adequately maintained, of sufficient diameter and screened to minimise drawdown, with a pump capable of drawing water from the base of the bore to the land surface". Further information on POL77 and how it relates to this assessment is provided in Section 3.0 of this report.

How water availability is assessed in a well is illustrated on the diagram in Figure 4, here the total water column is calculated and then divided up for different uses. The total water column (Available Head) is the height from the top of the well screen to the SWL. However, this calculation is conservative as POL77 states that the pump should be capable of drawing from the base of the bore to land surface. In deeper wells a submersible pump will be used. This is usually placed above the top of the well screen and will have an intake point that must remain submerged, it is considered reasonable to allow a pump length of 1m. Shallow wells may use surface pumps to draw water, or small pumps if the rate and volume of abstraction is low i.e. a domestic take, so it is reasonable to have zero or <1m pump length allowance. For shallow wells <10m in total depth, it is reasonable to consider the total well depth rather than the top screen to determine available head because they may use surface pumps, foot valves and / or shrouds to draw from the base of the well. Where there is no information on the top screen height for a well even if it is deeper than 10m, it is also reasonable, in line with POL77, to use the total well depth value.

Seasonal water level fluctuations can affect water availability within a well. The groundwater levels are usually higher in the winter and lower in the summer, this change in level is termed seasonal variation. The amount of variation can be different from year to year depending on whether it is a particularly dry year or not. HBRC have a series of wells within the region that are monitored, including within the Ruataniwha Basin, to aid with understanding of the magnitude of this variation and winter recovery levels (i.e. is the gap widening significantly and is the winter rainfall topping up the aquifer each year, or is there a risk of long term water level decline that could adversely impact the overall aquifer sustainability). The magnitude of seasonal variation can vary dependant on the well depth and proximity to pumping wells. How this value is calculated for this assessment and applied to the basin wells is discussed in more detail in Section 2.2 of this report.



SWL
Seasonal Variation

Predicted Drawdown

Remaining
Head

Pump Length

FIGURE 4: KEY FEATURES FOR ASSESSING WELL INTERFERENCE

Once the pump length and seasonal variation values are removed from the Available Head the water column that is left is known as the Remaining Head. This section of water column is the part that is used by the well owner when they pump their well. It may also be impacted by other nearby pumping wells, though some of this effect is included in the seasonal variation value. It is this section that is compared against the drawdown prediction. As a rule of thumb if the predicted drawdown value at the well is greater than 20% of the remaining head then there is a possibility that the well in question may be adversely impacted by this additional drawdown.

Well Screen

Often when making this sort of assessment there are many surrounding wells that must be considered. This method allows consideration of every well location to happen quickly and wells that are not likely to be impacted can rapidly be discounted and those where an effect is more likely can be flagged for further, more detailed, assessment. For example, a well that is identified as potentially being impacted may later be identified as of inadequate depth making it "inefficient", unused or of very low use. It may also have a real risk of potentially adverse effect that requires further consultation or mitigation.



2.0 RUATANIWHA BASIN WELLS

There are a large number of wells situated within the Ruataniwha Basin area. HBRC holds a database of all the recorded well locations and other details such as well depth, water level, lithology, date drilled etc. that are pertinent to that individual well. It is noted that there are likely other well locations that are not included in this database, or some in the database that have very little information, it is not possible to consider these at this point because of the absence of information.

2.1 ALL WELLS DATA

A request was made to HBRC to provide a spread sheet with their database information for all of the wells within the Ruataniwha Basin area. A total of 703 wells locations with associated data were identified. These wells represent a large range of different depths. It was noted that the numerical model drawdown predictions represent the change within the shallow (layer 1) and deeper (layer 6) groundwater levels. Within PC6 POL TT11 for managing groundwater takes hydraulically connected to surface water bodies there is an identified depth of 50m (top screen) where wells are excluded from assessment under this policy. Therefore, to assist with managing the large number of wells identified within the basin area they were divided into arbitrary groups based on their depths. The wells <50m in total depth are assessed against the shallow (layer 1) model output and the deeper wells >50m are assessed against the deep (layer 6) output. The well groups and number of wells associated with each group are shown in Table 1.

TABLE 1: WELL GROUPS ASSESSED

Group	Number Wells		
0 -9.99m	109		
10 – 19.99m	75		
20 – 29.99m	75		
30 – 39.99m	82		
40 – 49.99m	79		
+50m	236		
Unknown	46		
Total	703		

The locations of wells within the basin area are illustrated on the map in Figure 5, with the pink area representing the Ruataniwha Basin North Waipawa Catchment and the blue area the Basin South Tukituki Catchment.



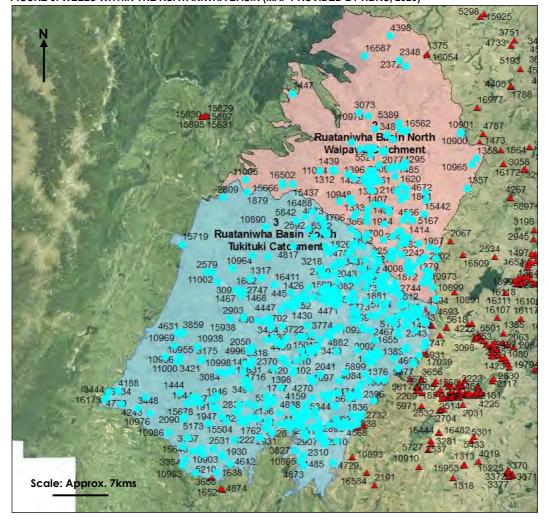


FIGURE 5: WELLS WITHIN THE RUATANIWHA BASIN (MAP PROVIDED BY HBRC, 2020)

These well locations were divided into the groups identified in Table 1 and applied to contour maps created from data provided by Aqualinc of the shallow (layer 1) and deep (layer 6) groundwater level changes for Scenario 4, Figures 6 and 7. The well groups in Figure 6 are colour coded with the 0-9.99m layer as red circles, 10-19.99m layer blue, 20-29.99m dark green, 30-39.99m magenta, 40-49.99m dark blue.

In Figure 7, the points shown represent all the deep wells in the Ruataniwha Basin and some proposed deep wells that will be constructed for T2 irrigation or augmentation purposes. The T2 abstraction and augmentation points are shown as solid blue circles, the other deep wells (>50m) are represented by the purple/blue open circles.

There are 46 wells identified that have unknown depths, the locations of these are shown in Figure 6 as black circles. These well locations are spread across the basin area but seem to have a larger number to the south.

The separate well groups were then analysed using specialised software to generate point sample grid values for the individual well locations. These point sample values represent the numerical model predicted drawdown at the individual well locations. This information was applied to a spread sheet and converted to an absolute value to represent predicted drawdown at each well location. This information is shown on the spreadsheets in Appendix 3.



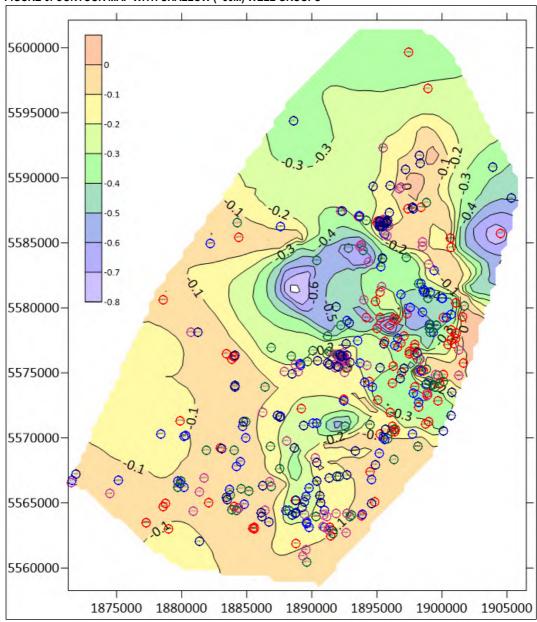


FIGURE 6: CONTOUR MAP WITH SHALLOW (<50M) WELL GROUPS

There is some data missing from the information provided in relation to the historic SWLs, top screen heights and in some cases also the total well depth. Where relevant data is missing values are assigned based on the information from the well in question or other nearby wells. For example, for the SWLs, an average of the existing data in each depth group was made and this average value was applied to wells missing this information. The assigned values are denoted in red text in the spreadsheets. Where the top screen height data is missing the total well depth value was assigned, again denoted in red text on the appended spreadsheets.



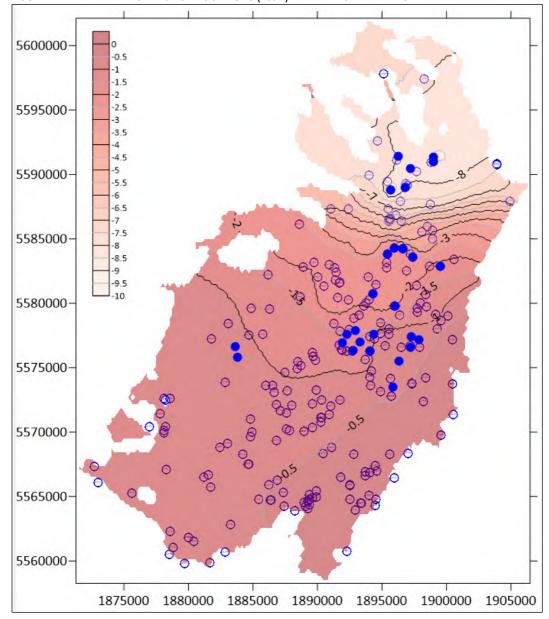
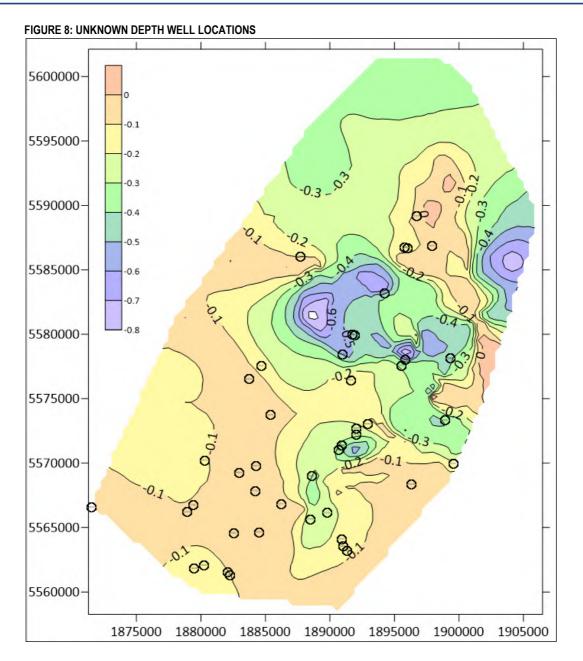


FIGURE 7: DEEP WELL ABSTRACTION LOCATIONS (>50M) AND TRANCHE 2 WELLS

The locations of the wells with no depth information are shown in Figure 8, but they are not included within the later well interference analysis spreadsheets. There is just not enough information to be able to consider them such that an estimate of the impact or otherwise can be reliably assessed using the methodology outlined here. It is likely that the majority are old or unused and that is why the information is so sparse. To consider them further, more well specific information for each location would need to be gathered.





2.2 SEASONAL VARIATION VALUES

As noted in Section 1.3 of this report, seasonal water level fluctuations can affect water availability within a well. The groundwater levels are usually higher in the winter and lower in the summer and the magnitude of this change can also vary year on year.

HBRC have a series of wells within the region that are monitored, the locations of these State of the Environment (SOE) wells within the Ruataniwha Basin are shown in Figure 9. Some of these wells are monitored for only groundwater levels, others for water quality and some are for both. Within the basin area there are a total of 40 water level wells monitored.

The magnitude of seasonal variation can vary dependant on the well depth and proximity to pumping wells. The water level data for each SOE well location can be accessed via the HBRC website, with a 5-year water level record from which the maximum and minimum level can be obtained as shown in Figure 10. The wells within the basin were identified and a summary of the location, depth, maximum and minimum water levels was tabulated, Appendix 1.



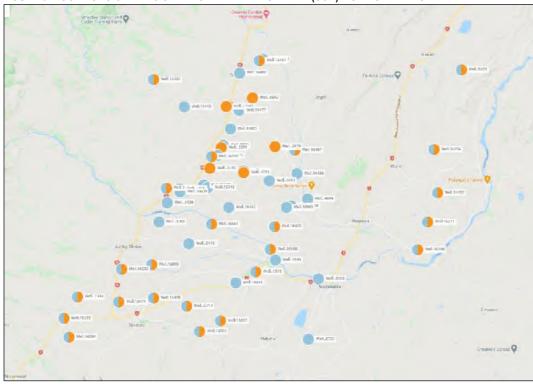
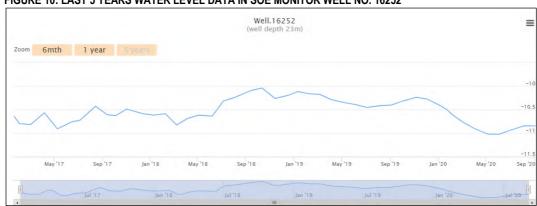


FIGURE 9: LOCATIONS OF HBRC STATE OF THE ENVIRONMENT (SOE) MONITOR WELLS





This allowed calculation of the maximum seasonal variation at each well. The SOE wells were separated into the same groups based on depth as the basin wells. A summary of the number of wells in each depth group is provided as Table 2. There are shallow and deep well locations, but there is only one well between 30 - 39.99m depth and two that are 40 - 49.99m deep. Where there are enough SOE wells in a depth range to provide data points, it is possible to generate a contour map and then interpolate a value for the other specific well locations. Where there are not enough wells the value, or an average of the two values, is adopted and applied across all the wells at that level. For example, the 10 - 19.99m well group well locations and SOE well points, with the variations, are shown in red in Figure 11. A contour map of the seasonal variation values was generated, Figure 12, all of the wells in that depth range were plotted and point sample grid values were obtained for each well location. This same methodology was used for the wells in the >50m well group as there are 14 SOE wells at this level.

The contour map for the >50m depth range wells is shown in Figure 13, it is noticeable that Well No. 5445 has a much larger recorded variation than the other wells at 55.48m. The next deepest is Well No. 1475 with 21.49m. These large variations may be a direct result of the influence of nearby pumping and have resulted in the contour plot estimating negative values for wells located in the south west of the basin area. It is not realistic that these values



would be negative so a conservative value of 2m for seasonal variation is used in the later well interference assessment for wells where the contour plot returned negative values. The locations of these wells, outside of the zero-contour line, are identified in Figure 13.

TABLE 2: SUMMARY OF SOE WELLS

Group	Number SOE Wells
0 -9.99m	6
10 – 19.99m	8
20 – 29.99m	9
30 – 39.99m	1
40 – 49.99m	2
+50m	14
Total	40

FIGURE 11: SOE WELL DATA AND WELL LOCATIONS FOR 10 - 19.99M WELL GROUP

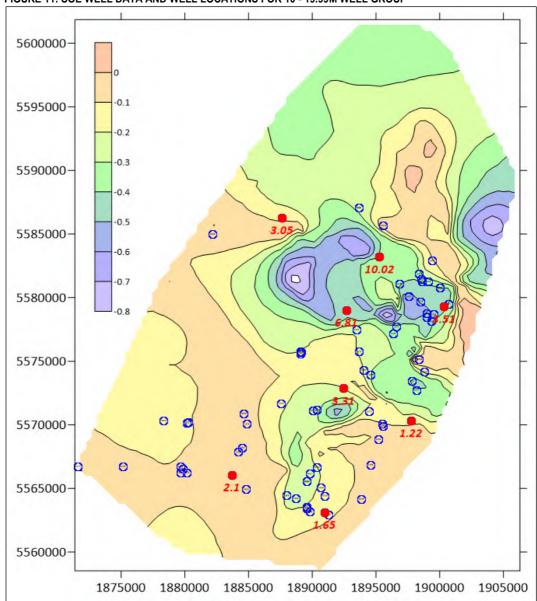




FIGURE 12: CONTOUR MAP - SOE WELL SEASONAL VARIATION DATA FOR 10 - 19.99M GROUP

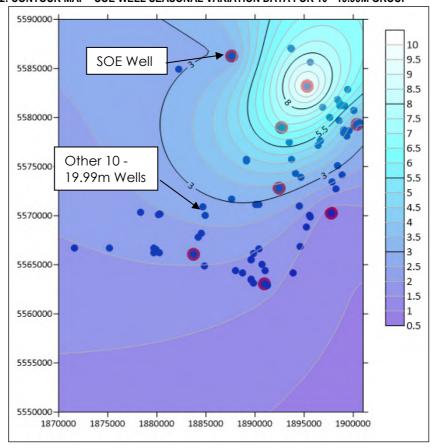
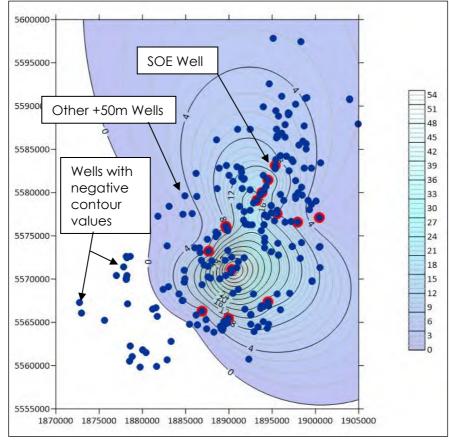


FIGURE 13: CONTOUR MAP - SOE WELL SEASONAL VARIATION DATA FOR +50M GROUP





3.0 RESULTS OF WELL INTERFERENCE ASSESSMENT

Spreadsheets were created for each well group and these show the locations where information was inferred from other nearby wells and calculates the available head, and the remaining head values for each well location. The predicted drawdown value is displayed, and this is calculated as a percentage of the remaining head value for each well. The values that are in excess of 20% or are a negative percentage value are highlighted. The highlighted wells are then assessed based on their potential for being categorised as "inefficient", their recorded use or other factors that may impact the outcome such as an unusually high value for SWL or very shallow top screen when compared with other similar depth wells.

The RRMP in Policy 77 provides guidance for assessing groundwater abstraction:

POL 77 ENVIRONMENTAL GUIDELINES - GROUNDWATER QUANTITY

- (a) To manage takes of groundwater to ensure abstraction does not exceed the rate of recharge.
- (b) To manage the available groundwater resource to ensure supplies of good quality groundwater.
- (c) To manage the groundwater resource in such a manner that existing efficient groundwater takes²¹ are not disadvantaged by new takes.
- (d) To manage takes of groundwater to ensure abstraction does not have an adverse effect on rivers, lakes, springs, or wetlands.

Of particular interest to this assessment is POL77 (c) with regard to the management of groundwater such that existing efficient groundwater takes are not disadvantaged by new takes. There is a definition for "efficient taking" that is:

"21 For the purposes of this Plan "efficient taking" of groundwater means abstraction by a bore which penetrates the aquifer from which water is being drawn at a depth sufficient to enable water to be drawn all year (i.e. the bore depth is below the range of seasonal fluctuations in groundwater level), with the bore being adequately maintained, of sufficient diameter and screened to minimise drawdown, with a pump capable of drawing water from the base of the bore to the land surface".

This definition allows the identification of wells through this process that could be considered as "inefficient" and that should therefore not prevent the abstraction proposed under this T2 take. A potentially inefficient well can be identified as one where the calculated value for remaining head on the well interference assessment spreadsheet returns a negative value. This negative value indicates that when the SWL and top screen (or total well depth in some cases) are considered along with the seasonal variation that the water column in the well may already be insufficient to provide for abstraction. This is before any other interference associated with additional drawdown predictions are considered. This tends to be most prevalent within shallow wells as they have minimal available head as a direct result of their depths. Copies of the spreadsheet assessments are provided in Appendix 3. It is also noted that the definition in POL77 refers to a pump capable of drawing water from the base of the bore to land surface. The 0m-9.99m group of wells is assessed based on the total well depth; the other well groups are more conservatively assessed based on top well screen values, or where this is absent the total well depth.

3.1 RESULTS - WELL GROUP 0 - 9.99M

There are 110 wells within the Ruataniwha Basin that are in the depth range 0m - 9.99m. The well interference assessment has initially flagged 28 wells for which the predicted drawdown as a percentage of remaining head is either a negative value or greater than 20%. Where the value is negative this represents a well that could be considered as "inefficient" and there are 14 negative values, 13 of which are wells that are 3.6m deep or less. The other negative value is from a well with a recorded SWL that is deeper than the recorded well depths, so could be an error in the database.

The other 14 wells identified have values greater than 20%. Of these 1 is less than 2m deep, 3 have recorded uses that are stated as unused or groundwater monitoring and 3 others are identified as groundwater sampling/monitoring. Wells that are used only for groundwater sampling/monitoring have a very low use requirement and will only have small amounts of

TRANCHE 2 CONSENT – WELL INTERFERENCE ASSESSMENT



water drawn at intermittent periods. It is unlikely that they will be adversely impacted, such that a sample cannot be obtained from them.

The result of this assessment is that a total of 7 wells remain flagged because they have a greater than 20% value and their use is not specified, or they are identified as providing domestic or stock water supplies. This means that from this assessment an adverse impact on them cannot be ruled out. Further consideration of these individual wells is therefore required, and this may take the form of an on-the-ground inspection to confirm if they are still operational and in use.

3.2 RESULTS – WELL GROUP 10 – 19.99M

There are 75 wells in this depth range and of these 17 were flagged initially as being less than 0% or greater than 20%.

Of these, 13 were negative values and therefore potentially "inefficient" wells. Closer inspection of the well details indicates that they are flagged because they have particularly shallow top screen heights or deep SWLs. Four of these wells are also identified as being groundwater sampling or monitoring wells.

The other four wells that are identified as being greater than 20% either do not have an identified use, or have exploratory, domestic and irrigation uses. Further consideration of these wells is recommended to confirm their uses and potential for impact.

3.3 RESULTS - WELL GROUP 20 - 29.99M

There are 75 wells in this depth range and of these 10 were flagged initially as being less than 0% or greater than 20%.

Of the 10 wells flagged 7 are negative values and therefore potentially inefficient, again these tend to be flagged because they have either deep SWLs or shallow top screens compared with other wells in this depth range.

There are 3 wells that have values of 20% or greater and these are retained as wells that need further consideration, their uses are identified as unknown, domestic supply and irrigation.

3.4 RESULTS - WELL GROUP 30 - 39.99M & 40 - 49.99M

The low numbers flagged by these groups means they are discussed together in this section. There is a total of 82 and 79 wells respectively in these depth groups. Both groups have 9 wells flagged initially. All but one of these was because of a negative percentage value suggesting potentially inefficient wells. It was noted that the flagged wells tended to have very shallow top screens or very deep SWLs compared to other wells in the depth groups. These could represent errors in the database or reflect SWL measurements that represent summer lows or are impacted by other nearby pumping wells. If the SWLs are already representing low stands then adding the seasonal variation to this makes for a very conservative assessment.

The one well in this group that was flagged as being greater than 20% was Well No. 2730 in the 40-49.99m depth range. This well records a SWL of 23.6m below ground level (bgl) which is considerably more than the average of the available SWL values of 9.01m bgl. With a total depth of 48.15m and a predicted interference value of 0.48m it is considered unlikely that this well will be adversely impacted.

3.5 RESULTS - WELL GROUP +50M

The greater than 50m deep wells comprises a large group of 236 wells. A different and deeper numerical model predicted drawdown layer, than for the shallower wells, was used to assess the potential impact at this level. Using the same methodology for assessment as for the shallow well interference of the 236 deep wells, a total of 21 were flagged initially. Of these 11 were removed as they have either very shallow top screen heights or very deep SWL recorded, this means that even without considering the impact of the proposed T2 abstraction these wells could be considered as "inefficient" in terms of the HBRC policy 77. The total number of wells that remain flagged at this depth range is 10.



3.6 SUMMARY - ALL WELLS GROUPS

The results of the previous well group discussions are summarised in Table 3. Of a total of 703 wells (including unknown depth wells) 94 were initially flagged in these well groups and 24 remain flagged following some further consideration.

There are 46 unknown depth wells. While it is not possible to specifically consider the impact on them, if it is assumed that they are still in use and that they, as noted in POL77, "penetrate the aquifer from which water is being drawn at a depth sufficient to enable water to be drawn all year (i.e. the bore depth is below the range of seasonal fluctuations in groundwater level)" and they are considered efficient wells, it is unlikely they will be adversely impacted. Of the 657 wells individually assessed only 24 are considered to require some further assessment.

TABLE 3: SUMMARY OF ALL FLAGGED WELLS

Well Group	Total No. Wells	Initially Flagged	Reduced	Comments
0-9.99m	110	28	7	"Inefficient" and monitor wells removed
10-19.99m	75	17	4	"Inefficient" and monitor wells removed
20-29.99m	75	10	3	"Inefficient" and monitor wells removed
30-39.99m	82	9	0	"inefficient" wells removed
40-49.99m	79	9	0	"inefficient" wells removed +1 deep SWL
+50m	236	21	10	"inefficient" wells removed, some with very deep SWL or shallow top screens
Unknown	46			
Totals	703	94	24	

The locations of the shallow (<50m) initially flagged, and the reduced number of flagged wells are shown in Figures 14 and 15. The grey and black areas in these figures are the locations of the applicants' properties. In Figure 14 only a few of the flagged wells are on any of these properties. There is no specific pattern to the locations of these wells, they are distributed across the basin with some clustering towards the eastern margin. A summary of the well interference assessment for these wells is provided in Table 4.

TABLE 4: DETAILS OF REDUCED FLAGGED WELLS 0 - 50M DEPTH RANGE

Well No.	Depth	H_SWL	Top Screen	Avail. head (m)	Seas. variati on (m)	Pump length (m)	Remai ning Head (m)	Predict ed DD (m)	as % remain ing head
15866	4	-1.92	4	2.08	1.95	0	0.13	0.08	66%
2902	4.11	-1.45	2.11	2.66	1.80	0	0.86	0.36	42%
5532	5	-1.92	5	3.08	1.96	0	1.12	0.28	25%
1357	5.18	-2.13	0.00	3.05	1.94	0	1.11	0.72	65%
16346	6	-1.92	6	4.08	1.83	0	2.25	0.51	23%
3590	7.3	-5.50	7.3	1.80	1.55	0	0.25	0.13	52%
3690	7.5	-4.30	7.5	3.20	1.75	0	1.45	0.47	32%
10968	13.1	-2.37	10.36	7.99	6.76	0	1.23	0.33	27%
5662	15.5	-3.9	11.5	7.60	7.30	0	0.30	0.28	91%
2773	18	-7.1	12	4.90	4.65	0	0.25	0.21	84%
5211	18.5	-3.9	9	5.10	3.63	0	1.47	0.39	27%
3664	25.6	-8.02	14.03	6.01	4.88	1	0.13	0.11	82%
3843	28	-4.17	10.50	6.33	5.09	1	0.24	0.08	35%
10978	28.35	-9.44	21.88	12.44	10.58	1	0.86	0.57	67%



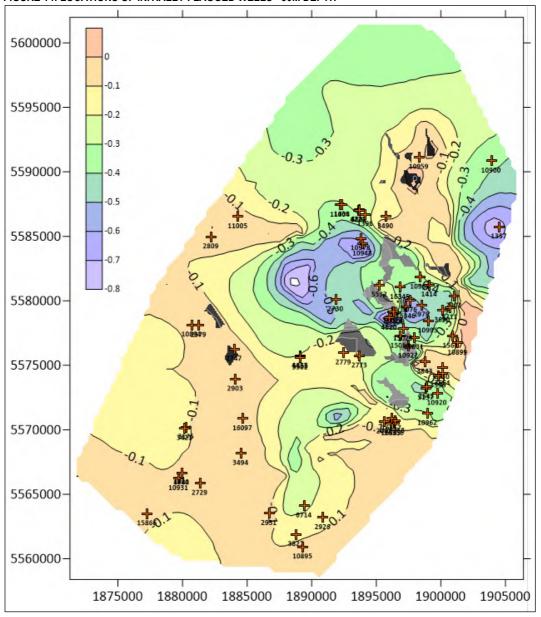


FIGURE 14: LOCATIONS OF INITIALLY FLAGGED WELLS <50M DEPTH

The well locations identified in Figure 15 are predominantly within the central area of the basin, where the predicted impact of the T2 taking at this shallow level is expected to be greatest. The exception is Well No. 15866 located in the south western corner. It is the well locations identified in Figure 15 for which some further consideration is recommended.



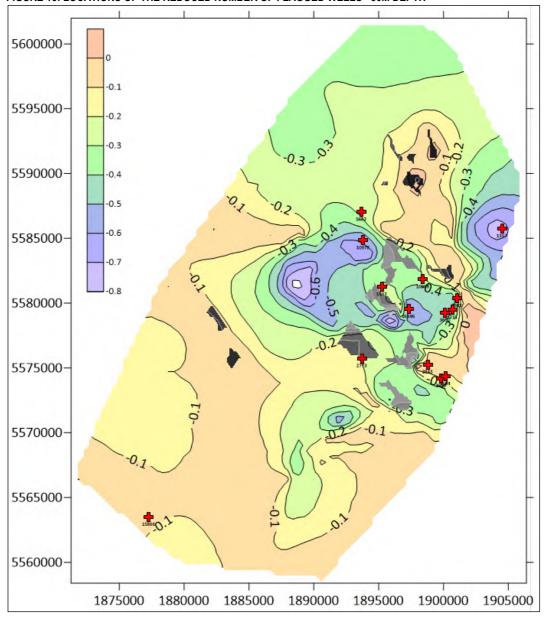


FIGURE 15: LOCATIONS OF THE REDUCED NUMBER OF FLAGGED WELLS <50M DEPTH

A summary table of reduced flagged well interference assessment results for the deep wells (>50m) is provided as Table 5. Maps showing the locations of the identified flagged and reduced flagged wells at this deeper level are shown as Figures 16 and 17. The initially flagged wells at this deeper level are widespread across the basin area. However, it is notable that the reduced number of flagged wells shows a cluster at the northern end of the basin. This corresponds with the higher level of water level difference predicted in this northern area by the numerical model.



TABLE 5: DETAILS OF REDUCED FLAGGED WELLS >50M DEPTH RANGE

Well No.	Depth	H_SWL	Top Scree n	Avail. head (m)	Seas. variati on (m)	Pump length (m)	Remai ning Head (m)	Predic ted DD (m)	as % remai ning head
3437	50	-10.5	46.0	35.5	6.23	1	28.27	6.45	22.81
10945	51.8	-17.86	24.6	6.7	2.30	1	3.41	0.99	29.09
3882	52.5	-8.3	44.5	36.2	4.97	1	30.23	8.04	26.59
5435	54	-12.17	23.6	11.5	9.37	1	1.08	0.27	25.00
16483	54.28	-12.17	52.3	40.2	4.97	1	34.19	7.99	23.36
16033	56.36	-12.17	53.5	41.3	5.53	1	34.76	7.56	21.75
10927	59.3	-12.17	59.3	47.1	5.40	1	40.73	8.20	20.14
16739	59.5	-12.17	57.6	45.4	5.04	1	39.39	8.01	20.33
1429	65.86	-3.65	42.0	38.4	34.50	1	2.89	0.74	25.60
5515	66	-12.17	54.0	41.8	5.56	1	35.27	8.97	25.42

FIGURE 16: LOCATIONS OF THE INITIALLY FLAGGED WELLS >50M DEPTH

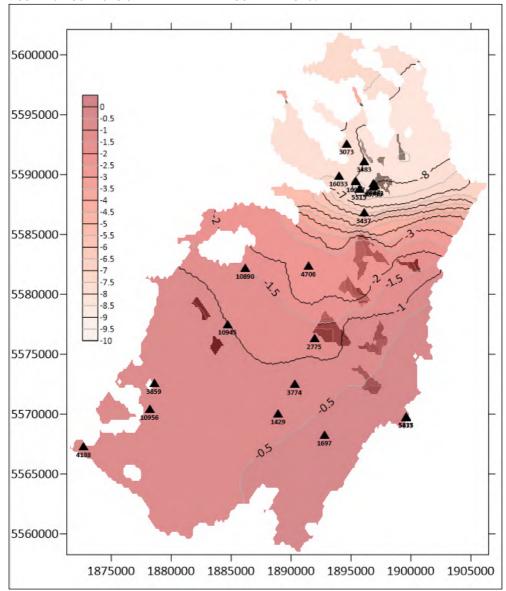




FIGURE 17: LOCATIONS OF THE REDUCED NUMBER OF FLAGGED WELLS >50M DEPTH 5600000 5595000--0.5 -2.5 5590000--3.5 -4.5 -5 5585000--5.5 -6 -6.5 -7.5 5580000--8 -8.5 -9 -9.5 -10 5575000-05 5570000-1429 5435 5565000-5560000 1875000 1880000 1885000 1890000 1895000 1900000 1905000



4.0 CONCLUSION

For this assessment all the wells recorded within the Ruataniwha Basin are considered. There are many wells, and these wells were divided into groups based on their recorded total well depths. The initial focus is on the wells within the 0m to 50m depth range and these are assessed against the numerical groundwater model shallow (layer 1) impact predictions. In order to assess the potential for impact on deeper wells, the modelled drawdown impact at a deeper level (layer 6) was reviewed separately against the deeper wells. It was not possible to fully consider wells of unknown depth but, if it is assumed that they are sufficiently deep to enable "efficient" taking of water then they are unlikely to be adversely impacted.

Consideration of aquifer parameters from existing well aquifer pump testing supports the outputs from the numerical model and increases confidence in the drawdown predictions made for the well locations assessed in this report.

A methodology for the assessment is presented. In some cases, gaps within the data provided by the Hawkes Bay Regional Council (HBRC) database are filled. Where this has occurred, the assumed value is identified with red text, for example Static Water Levels (SWL) and top screen heights. The assumed SWL are a numerical average of all the SWL available for each depth group and the top screen heights where absent are assigned a value equivalent to the total well depth.

The two contour maps provided from numerical modelling represent water level difference associated with the preferred scenario for the T2 water abstraction, for both shallow and deep layers within the aquifer. Values were interpolated from these contour maps and applied to specific basin well locations.

Seasonal variation was predicted based on analysis of the HBRC SOE monitoring wells network. A maximum value range was calculated based on the last 5 years of data. Where sufficient datapoints were available this data was contoured, and an interpolation of the seasonal variation value was applied to the individual well locations. Where the data was insufficient the available value or an average was applied.

Wells were initially flagged on the assessment spreadsheet for each depth group. They were then separated into those that were flagged because they are potentially of insufficient depth or otherwise classified as "inefficient" wells and those that might genuinely be adversely impacted by the proposed abstraction. Some could be further discounted from likely impact based on their recorded use or information from other recorded details.

Of a total of 657 wells (this value excludes the 46 unknown depth wells) that were individually assessed 94 were initially flagged in the well groups and 24 remain flagged following some further consideration. Of these flagged wells 14 are in the depth range <50m and 10 are >50m deep.

Further investigation is required to assess whether they will be adversely affected or not, and this may involve contacting the well owners or visiting the well locations. However, given that almost all of the wells assessed (i.e. 96%) are not considered as adversely affected, it is likely that this will also be the case for the remaining wells.

5.0 REFERENCES

Weir J. J. (2013): Statement of Evidence of Julian James Weir for Ruataniwha Water Users Group (Groundwater Modelling). Expert evidence presented before a Board of Inquiry for the proposed Tukituki Catchment plan change 6.7 October 2013.

Aqualinc Research Limited (2020) Ruataniwha Basin Tranche 2 Groundwater Modelling ref: WL18045 14/05/2020 Prepared by Julian Weir.

Board of Inquiry (2014) Final Report and Decisions Tukituki Catchment Proposal Volume 3 of 3 Plan Change and Conditions



APPENDIX 1

SEASONAL VARIATION VALUES



TRANCHE 2 CONSENT - WELL INTERFERENCE ASSESSMENT

				Actual Max	Actual Min	Variation
Easting	Northing	Well No.	Total depth	WL (m)	WL (m)	(m)
1899373	5574074	10993	5	-0.6	-1.99	1.39
1879008	5562996	16504	5	-0.58	-1.93	1.35
1878608	5564741	16253	6	-0.67	-3.23	2.56
1897409	5587588	16484	7	-0.88	-3	2.12
1898231	5572395	16499	8	-1.58	-3.4	1.82
1899808	5574191	16250	9	-2.82	-4.41	1.59
1887629	5586252	16502	11	-1.94	-4.99	3.05
1900352	5579269	16487	11	-2.54	-6.05	3.51
1897758	5570302	16500	11	-0.77	-1.99	1.22
1883705	5566043	16479	11	-2.89	-4.99	2.1
1890974	5563088	16503	11	-0.85	-2.5	1.65
1892688	5578991	16249	14	-2.6	-9.41	6.81
1892460	5572835	16501	16	-2.25	-5.56	3.31
1895306	5583180	16248	19	-8.28	-18.3	10.02
1879938	5566652	1944	20	-2.65	-14.23	11.58
1888442	5576277	16251	21	-7.7	-11.45	3.75
1884059	5569027	16252	23	-10.04	-11.02	0.98
1896318	5568312	1376	24	-0.12	-1.76	1.64
1898169	5569335	4696	25	1.47	-1.79	
1890368	5583649	16488	26	-12.37	-24.06	11.69
1891886	5576425	16880	26	-2.46	-7.4	4.94
1886853	5569343	16095	26	-2.06	-7.68	5.62
1892933	5563971	16507	28	-8.51	-12.6	4.09
1892218	5576195	15048	33	-1.86	-9.28	7.42
1897746	5587727	1485	45	-8.68	-13.92	5.24
1888469	5574925	1430	46	-6.03	-34.11	28.08
1893206	5579093	1475	52	-12.76	-34.25	21.49
1895573	5577578	1452	55	-19.84	-27.59	7.75
1886887	5566285	16478	58	-5.36	-8.33	2.97
1894468	5567403	16492	73	-1.41	-15.34	13.93
1889640	5576143	1426	75	-8.19	-15.93	7.74
1889691	5575889	4685	77	-12.04	-20.07	8.03
1900477	5577147	16486	79	0.36	-1.02	1.38
1895377	5583147	16477	82	-2.87	-11.74	8.87
1889909	5565432	6719	88	-5.98	-14.64	8.66
1893783	5579993	2220	110	-10.14	-22.73	12.59
1897911	5576587	4701	111	-1.28	-7.89	6.61
1887644	5573215	3702	123	-30.53	-38.46	7.93
1890301	5571106	5445	130	-6.01	-61.49	
1894517	5581470	15021	148	0.65	-15.35	16



APPENDIX 2

REVIEW OF TRANSMISSIVITY VALUES



Bay Geological Services Ltd

Bay Geological Services Ltd RD6 Napier 4186

mobile: +64 275 014 984 email: baygeological@xtra.co.nz

3 December, 2020 ref: BGS201_05

Consents Planner Hawkes Bay Regional Council Private Bag 6006 Napier 4110

Dear Sir/Madam,

DISCUSSION ON AQUIFER PARAMETERS ACROSS THE RUATANIWHA BASIN TRANCHE 2 RESOURCE CONSENT APPLICATION

1. BACKGROUND

The collective Application for Tranche 2 groundwater allocation provides detailed investigations into groundwater resources across the Ruataniwha Basin, including comprehensive numerical modelling of existing parameters to ascertain augmentation volumes along with calculations to determine potential adverse effects as a result of the proposed activity (Weir, 2020).

A long-term well interference assessment has been completed on the shallow (<50 m depth) aquifer wells, along with an assessment on potential effects on deep (>50 m depth) wells as part of the Tranche 2 Application.

This letter report was prepared to better understand aquifer parameters across the basin, particularly within the vicinity of the proposed Tranche 2 water takes which lie predominantly in the west of the Ruataniwha Plains.

2. SITE INFORMATION

The majority of the proposed Tranche 2 water takes are generally concentrated centrally and across the west of the gently sloping Ruataniwha Plains, immediately east of State Highway 50. Two Applicants are located on the western periphery, bound by rolling hills, and a third Applicant is located near the eastern edge of the basin. The extent of the study area across the Ruataniwha Basin is illustrated in Figure 1.



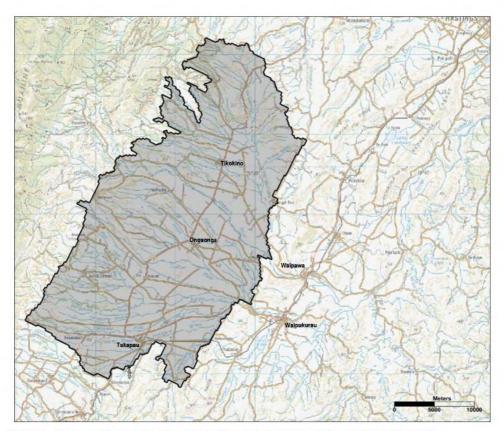


Figure 1. Topographic map showing extent of Ruataniwha Basin study area (Aqualinc, 2020)

3. HYDROGEOLOGY

The Ruataniwha Plains alluvial basin has been comprehensively studied in the past, resulting in interpretations of the geology and hydrology by PDP (1999, 2018), Francis (2001), and Morgenstern et al (2012). These reports describe the NNE-SSW-oriented sedimentary basin as being fault-bound at the western Ranges, and the eastern edge defined by westward-dipping strata which forms the Raukawa Range. The development of the basin commenced approximately 2 Ma years ago, with gradual evacuation of the former inland sea as a result of continual tectonic uplift, followed by infilling with alluvial sediments. This resulted in accumulations of material ranging from gravels, sandstone, silts and clays along with pumice and peat forming multi-layered aquifer systems with highly variable parameters (PDP 1999, Francis 2001). The alluvial sediments associated with the basin and relevant to hydrology are divided into two main units, oldest to youngest as follows (Francis, 2001):

- Salisbury Gravel (Upukororo) Formation: A unit comprising lower Quaternary, slightly to unconsolidated, typically blue-grey gravels, ignimbrite, clays and minor peat. The formation is observed in outcrop along hills to the north and northeast/west of the Plains, particularly east of the Mangaonuku Stream. Overlies marine sandstone and pumiceous siltstone; and,
- "Young Gravels" unit (including the Recent Terrace Aquifer Group and Old Terrace Aquifer Group). An upper Quaternary unit, observed as typically red, free and clay-bound gravels, silts and clays generally deposited basin-wide in a dominantly alluvial environment with minor lacustrine and swamp deposits.

2



The majority of the wells drilled across the Ruataniwha Plains intercept red-brown, poorly sorted Young Gravels that extend to approximately 40 m depth. The Young Gravels typically overlie significant thicknesses (250 m observed in outcrop) of blue grey, well sorted gravels and associated sediments attributed to the Salisbury Gravel Formation (Francis, 2001). The gravels overlie bedrock that forms the base of the alluvial basin including top Pliocene-aged siltstone, sandstone and mudstone.

4. AQUIFER PARAMETERS ACROSS THE RUATANIWHA BASIN

Information provided by HBRC staff and via the HBRC website associated with bore logs and aquifer test reports were reviewed in order to analyse aquifer parameters across the Ruataniwha Basin. Furthermore, a report was completed for HBRC by PDP (2018) which investigated basin aquifer properties. The Tranche 2 Well Interference report discussed the potential for long-term well interference across the shallow (<50m depth) wells as a result of the proposed activity. However, in line with the HBRC Plan Change 6 (PC6), the Applicants' wells will be drilled and screened to depths greater than 50m below ground level (bgl) in order to avoid stream depletion effects. Therefore, the aquifer parameters discussed in this report are from pump tested wells screened > 50 m depth.

4.1 Transmissivity values

The transmissivity (T) values determined from wells as included in the PDP (2018) study are included in Appendix A. The report findings indicate that high transmissivity values greater than T = 1500 m²/day are noted at well screen depths between about 40 and 60 m bgl (PDP, 2018); however, there are exceptions to this observation. Zones of transmissivity values from all wells were plotted by PDP (2018) as presented in Figure 2, recognising that the wide distribution of T values make a detailed analysis impractical. It is also noted that there are no discrete gravel aquifers within the basin, rather a series of discontinuous alluvial deposits with varying properties dependent upon depth and location (PDP 1999, PDP 2018). This results in a wide range of T values distributed spatially across the basin at varying depths.

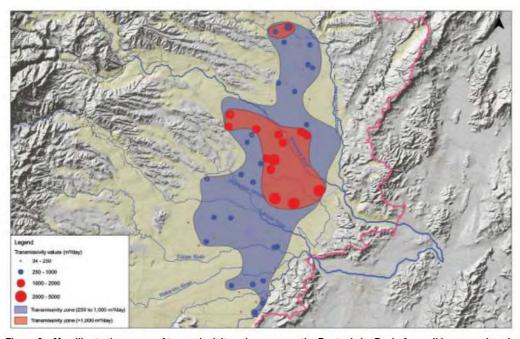


Figure 2. Map illustrating range of transmissivity values across the Ruataniwha Basin from all bores analysed (PDP, 2018).

3



The locations of the eight Applicants' properties are displayed on the Aqualinc map (Weir, 2020) as presented in Figure 3, along with annotations of aquifer transmissivities. The T values are denoted as being from Intermediate (I) depth wells (50 to 75 m bgl screen) or Deep (D) wells (screen > 75 m bgl).

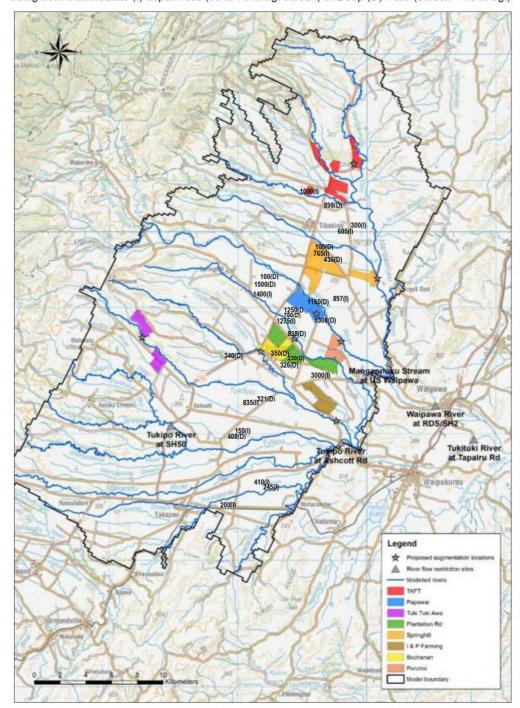


Figure 3. Aqualinc (Weir, 2020) map illustrating Applicant farm locations, with transmissivity values annotated defining the Intermediate (I) gravel (50 to 75 m bgl) and Deep (D) gravel (>75 m bgl) aquifers.

4



Figure 4 shows that the highest T values (>1000 m²/day) for the deep wells (>75 m bgl) are clustered north of the Kahahakuri Stream and generally south of the Waipawa River, likely inferring development of a wide, gravel-rich paleo-channel subparallel to the existing waterways. One intermediate depth well within the same area also displays a high T value of 1275 m²/day.

To the south of the Kahahakuri Stream, T values determined from deep wells reduce significantly to around 200 to 400 m²/day as far south as the Tukipo River, although it is noted that the number of pump-tested wells also decreases. Intermediate depth wells across this area provide a wide range of transmissivities from 150 to 850 m²/day, with test data available as far south as the Porangahau Stream.

To the north of the Waipawa River, limited aquifer parameter data is available for deep wells, with recorded transmissivities also lower, ranging from about 100 to 900 m 2 /day. Intermediate depth wells report T values from 300 to almost 900 m 2 /day, with a value of 1000 m 2 /day from a well immediately adjacent to the Mangamauku Stream near the northern bounds of the basin.

The details of T values within wells screened from 40 to 75 m bgl are shown in Figure 4, and clearly display the clustering of aquifer pump tested wells with significant gaps in the record. It is noted that zones of higher transmissivities (1000 to 2000 m^2 /day) are concentrated across the central part of the basin, with an outlier value (1400 m^2 /day) further to the west.

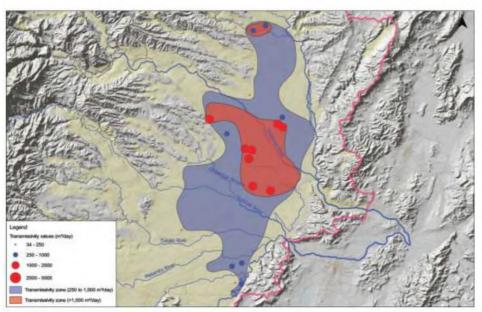


Figure 4. Plotted transmissivity values for screen depths from 40 to 75 m bgl (PDP, 2018).

The transmissivity values for wells screened >75 m depth were also plotted (PDP, 2018) and are displayed in Figure 5 which indicate that four wells recorded high T values (1000 to 2000 m^2 /day) in the central-western part of the basin, clustered between the Kahahakuri Stream to the south and the Waipawa River to the north. Lower transmissivities (250 to 1000 m^2 /day) are mapped to the north and south of this zone.

5



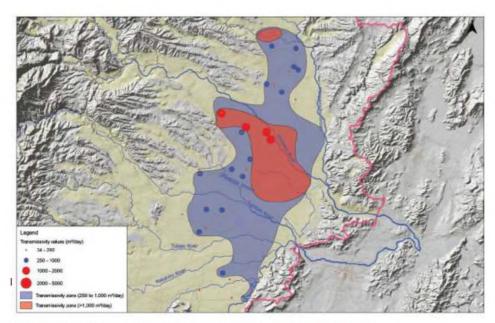


Figure 5. Plotted transmissivity values for screen depths >75 m bgl (PDP, 2018).

The available aquifer pump test data indicate that transmissivity values are generally highest north of the Kahahakuri Stream and south of the Waipawa River, although there are exceptions to this. The higher values lie adjacent to the meandering Waipawa River, which suggest that the river hydraulics provided consistent supply of clean, coarse material. The river is more linear than the Tukituki River, as illustrated on the GNS 1:250,000 geological map (Lee et al) presented as Figure 6.

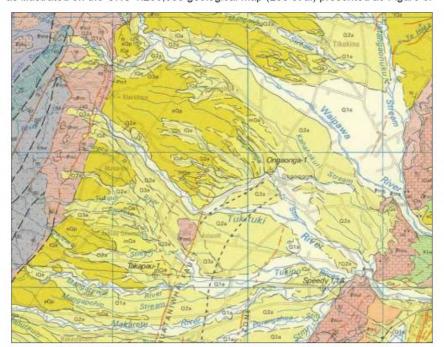


Figure 6. GNS 1:250,000 geological map across the Ruataniwha Plains (Lee et al, 2011).

6



The recorded transmissivities across the vicinity of the Tukituki River are variable and significantly lower than those further north. This reflects an increase in fine grained material within the gravel aquifer with possibly less well-developed paleo-channels. It is considered that velocities within the Tukituki River were lower compared to that in the Waipawa River.

In addition, tectonic activity along the active Ruataniwha Fault offset the main stem of the Tukituki River at the point where it crosses the river, likely affecting flow direction and velocity. This may have resulted in a lower velocity system meandering across the near level topography, re-working over-bank deposits with gravel deposits. The Waipawa River does not appear to be significantly offset by tectonic activity due to the fault pinching out north of the Tukituki River (Lee et al, 2011).

4.2 Storativity values

A review of pump test data and bore logs revealed a reduction in availability of storativity values in comparison to transmissivity, particularly across the older wells, which is likely due to more comprehensive pump testing being required. The storativity values determined from wells as included in the PDP (2018) study are included in Appendix A. Generally, storativity values decrease with greater depth, as a higher level of confinement is typically provided by layers of low permeability strata. The storativity data collected from wells screened from 40 to 75 m bgl (PDP, 2018) are presented in Figure 7

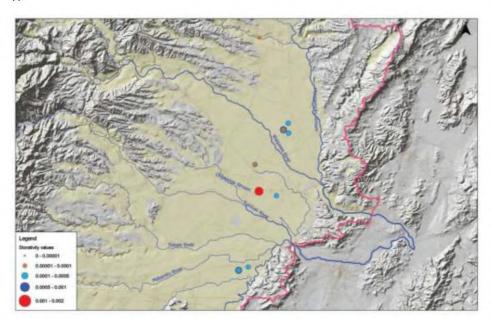


Figure 7 Storativity values for well screen depths 40 to 75 m bgl (PDP, 2018).

The available data is relatively variable, with storativity values of 0.001 to 0.002 mapped across the central part of the basin, and a greater level of confinement (storativity of 0.00001 to 0.0001) recorded in wells to the north and to the west of the basin.

The image presented as Figure 8 maps storativity values gathered from wells screened greater than 75 m depth (PDP, 2018). As expected for deeper wells, the values determined range below 0.001, where a higher level of confinement is afforded by the greater depth and likely inclusion of additional low permeability layers.

7



Although there are more points in the dataset, the storativity values for wells screened > 75 m bgl are lower, with the majority ranging from 0.0005 to 0.00001.

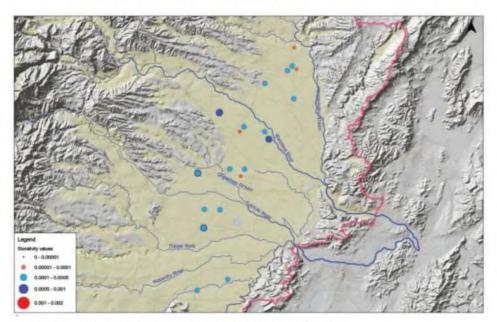


Figure 8 Storativity values for well screen depths >75 m bgl (PDP, 2018).

5 SUMMARY

The following points are made following a review of the available aquifer parameters in Intermediate depth (screened 50 to 75 m bgl) and Deep (screened >75 m bgl) wells across the Ruataniwha Basin:

- The aquifer pump test data indicate that no discrete gravel aquifers are present within the basin, rather a series of discontinuous alluvial deposits with varying properties dependent upon depth and location, which results in a wide range of T values distributed spatially across the basin at varying depths;
- High transmissivity values greater than T = 1500 m²/day are generally noted at well screen depths between about 40 and 60 m bgl;
- The transmissivity values for wells screened >75 m depth indicate that four wells recorded high
 T values (1000 to 2000 m²/day) in the central-western part of the basin, clustered between the
 Kahahakuri Stream to the south and the Waipawa River to the north likely where welldeveloped, clean gravel paleo-channels lie adjacent to the river;
- Lower transmissivities (250 to 1000 m²/day) are mapped south of the Tukituki River;
- The ancient Tukituki River likely meandered across the plains moreso than the Waipawa River, partially as a result of tectonic activity offsetting river channels, resulting in less well-developed gravel paleo-channels, infilled with re-worked fine grained material from over-bank deposits;
- Storativity values across the basin are lower with increasing depth likely due to the presence of additional layers of low permeability clays and silts.
- In the majority of wells screened from 40 to 75 m bgl, storativity values of 0.001 to 0.002 are mapped across the central part of the basin, with a greater level of confinement (storativity of 0.00001 to 0.0001) recorded in wells to the north and to the west of the basin;
- The storativity for wells screened > 75 m bgl generally ranges from 0.0005 to 0.00001 for the majority of the bores.

8



6 REFERENCES

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Lee, J.M., Bland, K.J., Townsend, D.B., Kamp, P.J.J. (compilers), 2011: Geology of the Hawkes Bay area. Institute of Geological & Nuclear Sciences 1:250,000 Geological Map 8. GNS Science, Lower Hutt, New Zealand.

Morgenstern, U., van der Raaij, R., Baalousha, H., 2010: Groundwater flow pattern in the Ruataniwha Plains as derived from the isotope and chemistry signature on the water. GNS Science Report 2012/23. Wellington, New Zealand.

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Pattle Delamore Partners Ltd, 2018: Ruataniwha Aquifer Properties Analysis and Mapping. Client Report No. C02591543 for Hawkes Bay Regional Council, prepared by Cameron Jasper, Pattle Delamore Partners Ltd., Christchurch, New Zealand.

Weir, J., 2020: Ruataniwha Basin: Tranche 2 Groundwater Modelling. Client Report Project No. WL18045. Aqualinc Research Limited, Christchurch, New Zealand.

Report Limitations

This letter report is written based on conditions as they existed at the time of the study, and there is no interpretation made on potential changes that may occur across the site. Subsurface conditions may exist across the site that are not able to be detected or revealed by the investigation within the scope of the desktop project, and are therefore not taken into account.

7



APPENDICES



APPENDIX A

AQUIFER PROPERTY DATA FOR
RUATANIWHA BASIN BORES
PDP (2018)



Final K.	in/all	2 2	96	41	16	24	18	188	20	2	23	36	417	22	15	24	14	43	6		. 0	. 00	17	4	1	~		89	10	303	44	36	0	34	2	43	62	23	41	77	1 2	9	3.	33	19	2%	**
Screen length	(m)	53.9	20.0	16.0	6.1	19.6	0'9	2.0	12.0	7.0	7.0	19	0.0	8.0	14.3	14,3	10.0	24.0	1.0	101	35.0	25.0	0.6	17.4	29.0	23.6	0.0	17,0	6.7	5 4	20	20.0	39.5	0'9	7.1	14.0	14.0	2.9	3.0	0.0	0'6	0.6	9.0	10.0	10.0	6.0	4.5
Final K, (Matched)	(m/m)		1.61E-03						1176-09								6.586-04	6.86E-02												3,000,000	6,9905.98	1,405-06				6.66E-02	1.48E-02			5, 205,03	1.06E-01	2.65E-02	1.06£-02			8.01E-03	
Final K, (Max)	(m/a)	1,621-03	20.7407		2,605-04		1075-04						4.00E-03	2.04E-03	2225-04	2,225-03				1 000 04	3 705-03	1.855-03	3.605-04		2,105-03				1.386-02	3.805-02									2.80E-02	3.308-02				5.00E-02	4.00E-02		
Final 6	full tues	100	23	911	36	61.45	107	118	319	55.5	85.5	142.5	9	88	74	74	8	86		63	37	37	36	92.5	30	33			\$ 5	6 5	44.3	53.8	584	112.51	28.8	74	24	0	Q x	20	R	R	53	8	8	88	-
Total Depth	(m)	119.0	80.0	150.0	77.1	81.0	117.0	162.5	0999	139.7	139.7	157.1	OTTO	77.0	123.8	123.8	137.0	123.0	77.0	0000	74.8	74.8	580	134.0	111.0	342.0		39.4	81.2	95.0	5000	77.4	121.0	123.0	97.5	90.0	006	31.7	0.80	0.89	66.0	66.0	66.0	61.7	61.7	98.0	100
Screen	(do w)	110.0	60.0	150.0	1.19	61.0	117.0	135.0	0.00	139.7	139.7	157.1	000	74.0	123.8	123.8	135.0	120.0	77.0	000	74.8	74.8	58.0	129.0	77.0	72.0		39.4	80.4	96.0	515	77.4	1000	121.0	75.5	89.3	89.3	31.7	52.6	0.70	66.0	66.0	66.0	60.0	60.0	945	1
Screen	(Maga)	20.08	40	134	61.56	61.45	111	133	24	137.7	132.7	1 13	35	69	109.5	109.5	125	96	2 7	607	40.8	49.8	49	111.6	48	48.4		22.4	70.67	10.01	44.5	57.39	60.5	115	68.4	75.3	75.3	28.8	49.0	62.0	i tr	25	25	90	05	88.5	100
Final K/8' (Matched)	(App)		7.00F-05						3.00E-05								7,00E-06	7.00E-04												5 000 03	K-900K-903	7.00E-05				9,00E-04	2.00E-04			1.005.04	2.00€-03	5.00E-04	2.00E-04			9.00E-05	11 11 11 11
Final K/S' (Max)	T deal	\$.00E-05	Section Section		1.005-05		1.006-06						1.00E-04	3.00E-05	3.00E-06	3.00E-05		-	1,000.04	3.000,00	1,005-04	5.00F-05	1.006-05		7,00E-05				2.005-04	3,301.04									8.00E-04	1,001-03				1.00E-03	8.DDE-04		
a lead	- Final S	1,305-04	1.80F-04		2,805-04	1.55E-04	1,405-04	5.00E-04	5,906-05		4.00E-05		5.00F.04	1.05E-04	1,15E-04	2,705-04	3.146-04	5,00E-04	6,508-04	A. AUT. OF	5,005.04	1.705-04	3.40E-04		1,27E-04		9.60E-05	1.025-04	6.20E-04	4.0015-04	1110000	2,00E-04			3.00E-04	3.60E-04	1.70E-04		1.50E-04	1.82E-04	7.205-04	2.23E-04	8,28E-05	4,00E-04	1,40F-04	3,508-05	
Final T	(Mea/m)	1150	2500	868	200	509	210	1500	1000	326	300	430	250	245	300	480	230	1300	360	000	370	260	320	100	52	8	400	1109	9 5	877	525	765	410	408	835	960	1250	207	305	1585	533	168	1412	520	006	700	-
Morehime	NOTES AND ADDRESS OF THE PARTY	5576763	5573747	5587889	5564890	5585936	5585510	5581548	5588811	5575604	5575604	5583557	5581505	5565918	5572459	5572459	5576273	5579075	5575889	23/3000	5566631	5566611	5556947	5584217	5582866	5572055	5587673	5589256	5570750	05/0/55	5580364	5584310	5566526	5570382	5572513	5580255	5580255	5564205	5579717	5579717 SSROTHE	5580036	5580036	5580036	5280655	5580655	5579768	
		1892989	1897306	1896453	1889635	1898543	1898101	1891706	1895692	1893721	1893721	1897384	1891736	1892498	1891769	1891769	1894084	1896353	1889691	1600000	1893704	1893704	1894618	1896642	1898683	1888029	1898731	1896856	\$890247	1890690	1805858	1895977	1891828	1889611	1890300	1894023	1894023	1892678	1898466	1898450	1897978	3897978	1897978	1898424	1898424	1893617	
Roce cumber Earling	Se number	16508	15755	15431	6721	5723	5707	1655	5515	758	5498	5497	5392	5344	4882	4882	4830	4764	4685	4083	4656	4656	4654	4593	4566	64489	4295	4271	4110	4110	2883	3870	3854	3852	3774	2933	2933	2911	2278	2278	2277	7777	22277	2246	2246	2219	2000



Final P.	(m/d)	26	63	44	7	133	121	109	30	101	2	78	42	15	1	52	9 :	4.6	10	122	mags in
Screen length	(m)	11.6	1.8	0.0	10.0	6.0	6.0	0'9	6.1	2.0	27.8	12.0	4.0	4,0	9	0	1.4	100	3.6	11	ot shown on the
Final K, (Matched)	(p/w)					4,505.04	4.50E-04													1.35F-04	Note: Bores with multiple-values have values from multiple observation bore concern to a four bore 16705 was provided via data from a concent application after the main dataset from HBAC was received. This bore is not shown on the maps in
Final K, [Max]	(p/m)																				set from HBRC was
Final B'	conf (m)			71.3	17.4	45	45	17.3		8.54	33.86	40.23	0	33,23	38.7	45,105	*****	35.01	33.98	53	e main data
Total Depth	(m)	38.4	29.9	83.5	080	51.0	51.0	55.5	11.9	45.0	73.2	75.0	19.8	65.8	45.7	52.4	25.7	26.4	58.4	76	of collection after the
Screen	(m bg()	35.7	15.5	82.6	68.6	50.0	50.0	55.5	8.8	45.0	61.6	68.7	18.0	55.2	43.6	976	ď:	44.0	43.0	76	
Screen	(m bg()	24.1	13.7	76.6	58.6	44	4	49.5	2.74	43	33.86	86.69	14	51.16	39.31	46.33	47.85	181	39.36	99	
Final K/B' (Matched)	(day)					1.00E-05	1.00E-05													1.00F-05	
Ŷ	[day]																				
	Final S					1.106-04	4,666-05													4.666-05	
FinalT	(/up/, m)	458	495	227	34	1600	1456	1306	357	806	19	1400	418	147	R	572	2715	177	92	1275	
	Northing					5576725	5276725	5577675	5564137	5564971	5565080	5580465					23/13/8				
	Essting	1896736	18953868	1894411	1895358	1895271	1895271	1894914	1893885	1894591	1893998	1891572	1895591	1896641	1897746	1893206	18255/3	10000333	1887435	1893210	
	Sore number Easting	2011	2044	2043	1914	1881	1881	1880	1838	1837	1836	1826	1656	1518	1485	1475	1452	1436	1381	16765	

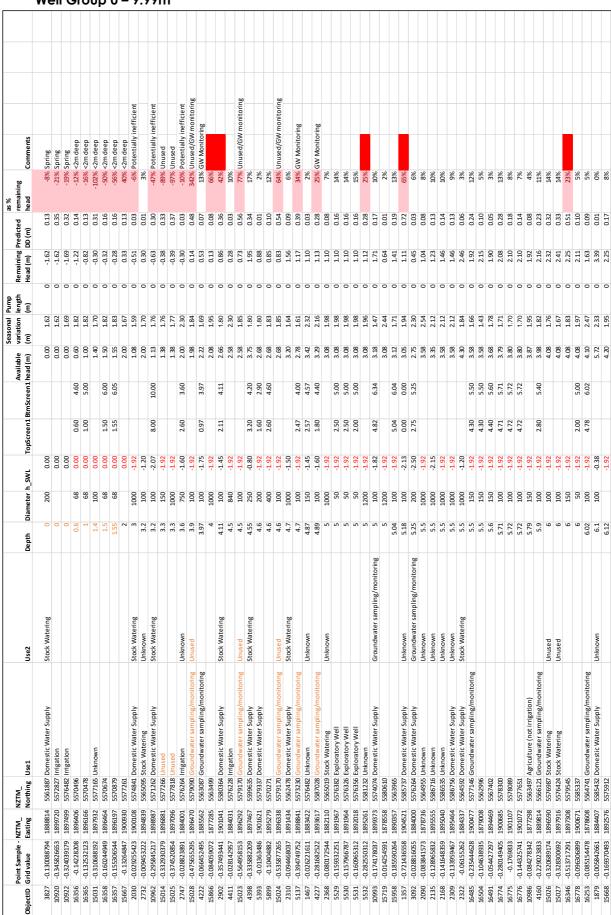


APPENDIX 3

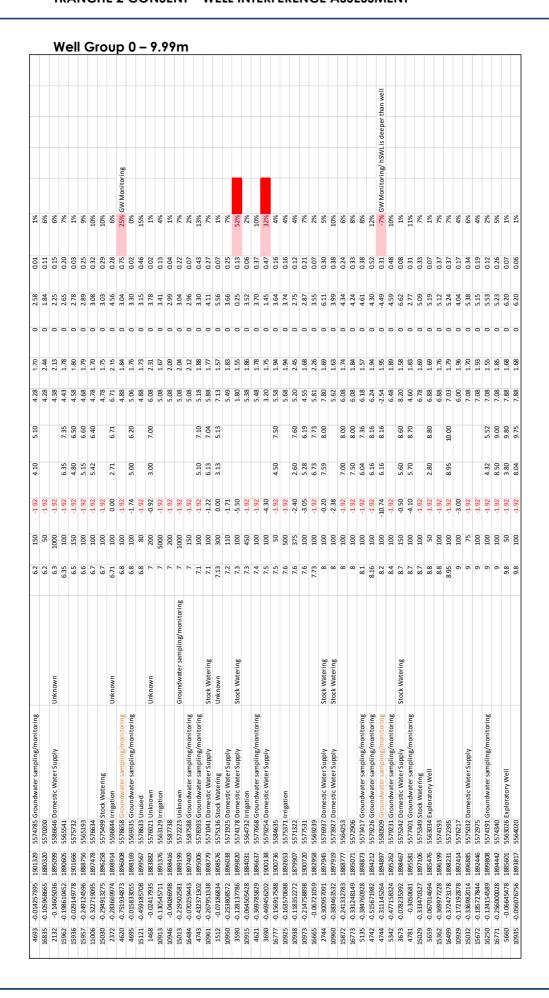
WELL INTERFERENCE ASSESSMENT



Well Group 0 - 9.99m







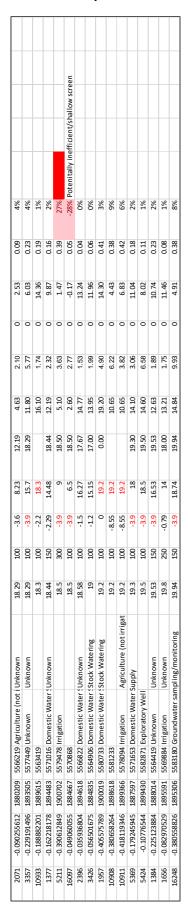


Well Group 10 – 19.99m

	_ ~	Point Sample -								-	Available	Variation	length	Remaining P	Predicted r	remaining		
Company Series Stroke Stro	jectID G		NZTM_Eas 1871605	NZTM_No Use1 5566711		10	Diameter F	StcWtrLv 7	opScreer Btr.	0	0	(E)	Œ .	Head (m) 4.23	0		Comments	
Controller Con	3175	-0.106420037		5570202 Stock Waterin	g Domestic Water Supply	10.05	150	-3.2	4	10.05	0.80	2		ľ	0.11	%9-	very shallow top screen/potentially	/ inefficient
Compression Systems	16770	-0.297310488	1894635	5573902		10.11	100	-3.9	9.61	10.11	5.71				0.30	13%		
0.000000000000000000000000000000000000	4477	-0.196723963	1889093	5575720 Groundwater	sampling/monitoring	10.2	100	-6.3	6.2	10.20	-0.10				0.20	-4%	8	
Companies parameter para	6353	-0.031337623	1875164	5566712		10.2	125	-3.9	10.2	1	6.30				0.03	1%		
Company Comp	6489	-0.204911392	1894088	55/4285		10.2	150	-3.9	10.2	11.50	6.30				0.20	% %		100000
Control No.	3474	-0.0640603	188451/	SSS 1/9 Domestic was	er : Stock watering	10.5	027	-3.2	5.35	10.50	CT.7				0.00	%67-	very snallow top screen/potentially	Inerricient
Control Cont	5343	-0.321225291	1896407	5577135 Groundwater	sampling/monitoring	10.5	100	9.5	10.5	4	0.60				0.32	18%		
Control No. 1985 State Control No. 1985	2961	-0.200602856	1890381	5566647		10.5	100	-3.9	9.6	10.50	5.70				0.20	2%		
OF TOWARDS STATE AND TOWARDS	302	-0.090456011	1895518	5570024		10.7	100	-1.04	8.2	9.78	7.16				0.09	5%		
A LIANCATION INSINSTANT SERVINGE CONTRACTOR TO MINISTANT MANAGEMEN 1956666 (2015) (201	205	-0.130360983	1887629	5586252		1	120	-3.9	9.8	11.00	2.90				0.13	2%		
QUESTION STATES STATES STATES 1144 100 349 1037 1144 637 1147 6 37 104 107 640 255 0.04 404 0.04	1228	-0.278047815	1893666	5587059 Groundwater	sampling/monitoring	11.18	100	-10.35	8.4	11.20	-1.95				0.28	-3%	8	t/very deep h
0.0259029 190325 2012 2012 2012 2012 2012 2012 2012 20	200	-0.147572216	1897758	5570302		11.44	100	-3.9	10.27	11.44	6.37				0.15	3%		
0.00258929 187939 573427 Stock Watering Unknown 115 150 349 113 1156 640 1650 040 0402664892 187939 573427 Stock Watering Unknown 115 115 100 349 10134 1879 050 050 050 050 050 050 050 050 050 05	487	-0.438999299	1900352	5579269		11.5	120	-3.9	10.3	11.50	6.40				0.44	16%		
0.005581214 189818 189519 190520 100050 100500 100050 100500 100050 100050 100050 100050 100050 100050 1005050 1000500 1000500 1000500 1000500 100050 100050	203	-0.144757907	1890974	5563088		11.5	150	-3.9	10.3	11.50	6.40				0.14	3%		
OFFICIATION STATES	467	-0.402846492	1897819	5573427 Stock Waterin		11.57	100	-2.28	8.96	10.48	99.9				0.40	10%		
O.075501505 SSSSESS SSSESS OF CONTRICTATION CO	356	-0.195483124	1889144	5575689 Groundwater	sampling/monitoring	11.8	20	-3.9	2.8	11.80	1.90				0.20	-8%	Potenitally inefficient	
OCYSINATIONS 1889 18 58 5543 CANDURANE SERVING 11.98 11.09 11.89	838	-0.050367857	1893885	5564137 Domestic Wat	er Stock Watering	11.88	150	0.35	2.74	8.84	3.09				0.05	3%		
CASTACTAN STANDARD SIRREDOR UNIFORM Grandwater simpling 11.99 15.9 19.9 11.9	479	-0.073292063	1883705	5566043 Groundwater	sampling/monitoring	11.98	100	-3.9	10.74	11.48	6.84				0.07	2%		
Q.0579518871 SSSSSS SSSSSSSSSSSSSSSSSSSSSSSSSSSSSS	920	-0.524701387	1897594	5580050 Unknown		11.99	120	-7.63	11.99		4.36				0.52	-30%	GW Sampling/deep hSWL	
OFFORMSISSISSISSISSISSISSISSISSISSISSISSISSIS	345	-0.387877748	1896860	5581052		12	150	-3.9	11	12.00	7.10				0.39	-138%	Potenitally inefficient	
O-05791554 is 188922 SS-559CP Unexter (water from the control of	999	-0.075591897	1895218	5568823 Domestic Wat	er ! Unknown	12.3	120	-1.22	9.14	11.58	7.92				0.08	1%		
0.48995188 PR SSST STORY Unknown Countrol extension in 12.4 s 19.9 s 18.6 s 12.4 s 18.0 s 18.0 s 19.0	143	-0.067531462	1884224	5567824 Domestic Wat	er Stock Watering	12.3	120	Ļ	9	10.00	2.00				0.07	3%		
0.1099861516 1898213 SSPSTADE Plotanoval Configuration (Marchage) 12.4 59 6.5 12.4 6.0 4.30 0.0 7.90 0.0 0.109861516 1898213 SSPSTAD Exploratory Watering (March 1 12.4) 12.7 12.7 12.4 9.7 12.7 12.7 0.0 1.0 0.0 </td <td>979</td> <td>-0.497913534</td> <td>1898532</td> <td>5579677 Unknown</td> <td></td> <td>12.4</td> <td>150</td> <td>-9.35</td> <td>11</td> <td>12.00</td> <td>1.65</td> <td></td> <td></td> <td>Ċ</td> <td>0.50</td> <td>-14%</td> <td>GW Sampling</td> <td></td>	979	-0.497913534	1898532	5579677 Unknown		12.4	150	-9.35	11	12.00	1.65			Ċ	0.50	-14%	GW Sampling	
0.089581791 1892009 057000 Domestic Water Stock Watering 125 159 2.74 125 100 222 222 0.00 0.01 0.01 0.01 0.01 0.	205	-0.193495417	1889128	5575579 Exploratory M	(ell	12.4	20	-3.9	6.5	12.40	2.60				0.19	-11%	Potenitally inefficient	
0.238541795 189818 557113 Domestic Water' Supky Attenting 126 127 100 2456 127 108 0 249 0 229 0 0 612 0 0 612 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	655	-0.389951807	1898211	5572708 Domestic Wat	er Stock Watering	12.5	120	-2.74	12.5		9.76				0.39	2%		
0.0285714785 18895818 581138 6818 6	421	-0.108815145	1880180	5570103 Domestic Wat	er Stock Watering	12.6	120	-5.95	8.27	10.80	2.32				0.11	%98-	Potenitally inefficient	
0.938824918 SB89282 SSESS133 Connected Water (Unknown 1.28 1.03 1.173 1.28 0.59 6.86 0.34 0.34 0.9238249438 SB892812 SSESS13 Gondwater sampling/monitoring 13.1 1.0 -3.7 1.05 1.15 1.0 9.9 1.0 0.93 0.9 0.0 4.17 0.03 0.0	923	-0.260568774	1890080	5571104 Domestic Wat	er Supply	12.7	100	-3.66	12.7		9.04				0.26	4%		
0.232803444 1889822 SSSSSSI Groundwater sampling/monitoring 131 150 -237 1318 1138 150 6 10 1310 6 10 1338 0 6 179 6 179 0 233 0 0.45284344 188981 SSSSSI Groundwater sampling/monitoring 131 1130 -31 131 1130 1130 1130 1130 11	414	-0.338541795	1899118	5581233 Domestic Wat	er (Unknown	12.8	100	-9.14	11.73	12.80	2.59				0.34	-10%	Potenitally inefficient	
2.3.2580444 ISSNB33 CONSTITEMENT OF A CONTRICT	637	-0.229714785	1889582	5565513 Groundwater	sampling/monitoring	13		-3.9	10	13.00	6.10				0.23	%9		
OLOSSIGNSES SSSIGNSES	896	-0.325803444	1898417	5581833		13.1	120	-2.37	10.36	11.58	7.99				0.33	27%		
0.0293773645 1889814 5565106 Groundwater sampling/monitoring 13.3 100 -3.9 13.3 14.0 9.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	872	-0.466350846	1899019	5578431 Agriculture (n	ot i Unknown	13.11	100	Ţ	12.2	13.11	11.20				0.47	2%		
O.059673645 1889218 5537105 Colored Marker Sampling/monitoring 13.3 10.7 3.9 13.5 13.5 1.9 0.1 0.2 0.2 0.1 0.1 0.1 0.1 0.2 0.1 0.1 0.2 <t< td=""><td>960</td><td>-0.191477827</td><td>1890678</td><td>5565026</td><td></td><td>13.2</td><td>100</td><td>-3.9</td><td>13.2</td><td>14.20</td><td>9.30</td><td></td><td></td><td></td><td>0.19</td><td>3%</td><td></td><td></td></t<>	960	-0.191477827	1890678	5565026		13.2	100	-3.9	13.2	14.20	9.30				0.19	3%		
O.0283407121 18892143 5555113 Stock Watering 1341 300 -207 1.24 1.24 1.24 1.24 1.24 1.24 1.24 1.24 1.24 1.24 1.24 1.25 1.25 0.0 0.22 0.0 0.28 0.0283407121 1889324 5555134 Industrial (Commercial) 14 150 -20 1.25 0.0 0.22 0.0 0.28 0.0283407121 1889220 555524 Industrial (Commercial) 14 100 -3 1245 139 0.56 0 2.28 0.05 0.018164407 1882712 558454 Industrial (Commercial) 14 100 -3 1245 139 0.0 0.2 0.0 0.2 0.018164407 1882712 5582454 Industrial (Commercial) 14 100 14 15 14 15 14 15 14 15 14 15 14 15 14 15 14 15 14 15 <td>770</td> <td>-0.3/0319819</td> <td>1896639</td> <td>55//6/2 Groundwater</td> <td>sampling/monitoring</td> <td>13.3</td> <td>001</td> <td>-3.9</td> <td>13.3</td> <td></td> <td>9.40</td> <td></td> <td></td> <td></td> <td>0.37</td> <td>%6</td> <td></td> <td></td>	770	-0.3/0319819	1896639	55//6/2 Groundwater	sampling/monitoring	13.3	001	-3.9	13.3		9.40				0.37	%6		
CODS9729491 1889282 556514 Industrial (Commercial) 13.9 13.9 14.9 15.0 2.0 2.0 0.0 0.0 0.239377261 1889382 556514 Industrial (Commercial) 14.0 13.9 14.4 15.0 1.3 0.0 2.6 0.0 0.2377761 1889382 556514 Industrial (Commercial) 14.2 15.0 -3 14.4 15.0 1.0 1.75 0.0 2.4 0.0 0.03177448 1882520 558697 Domestic Water (Stock Watering 14.24 10.0 -3 10.0 1.4 1.0 1.75 0.0 2.41 0.6 0.449388.86 578643 Irrigation 41.24 10.0 -3 13.9 14.2 1.0 2.4 0.0 2.41 0.45 0.440388.86 578643 Irrigation 41.44 10.0 -3.5 11.5 2.9 1.0 2.4 0.5 0.728707978 18.0 1.0 -3.5 1.0 -3.5 1.5 1.0 -3.5 1.5 <td>934</td> <td>-0.1/5038005</td> <td>1889814</td> <td>5563119 Stock Waterin</td> <td></td> <td>13.41</td> <td>9</td> <td>-4.2</td> <td>12.12</td> <td>13.41</td> <td>7.97</td> <td></td> <td></td> <td></td> <td>0.18</td> <td>3%</td> <td></td> <td></td>	934	-0.1/5038005	1889814	5563119 Stock Waterin		13.41	9	-4.2	12.12	13.41	7.97				0.18	3%		
Q.29272601 BR898265 S565362 Indicatorial (Commercial) 1.5 1.0 -3.9 1.4 1.5 0.5 2.3 0.5 0.5 0.5 0.5 0.5 0.5 0.6	3,70	-0.0596/3645	100001	5570025 UNKNOWN	UNKHOWII	13.0	000	70.7-	, 1,	13.33	2.4 c				0.00	5,07		
0.012175-448 1899282 5565054 Industrial (Lommercial) 14 150 -35 14 150 -35 14 150 -35 14 150 -35 14 150 -35 14 150 -35 14 150 -35 14 150 151	747	-0.28340/122	1890384	55/1135		13.95	001	-3.9	12.45	13.95	8.55				0.28	2%		
O.018104783 SSSS SSS SSS SSS SSS SSS SSS SSS SSS S	007	-0.229727601	10090001	5562502	lillercial)	1 5	150	0.0	4 5	5	10.10				0.70	2%		
OLA99382847 SSEASON Domestic Water Stock Watering Included in Control Marketing Includ	200	-0.1321/3440	10093390	2303302	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	14.7	3 5	0,0	- F	13.00	10.10				0.13	707	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
O.4612310283 SSSP5043 Ingition Agriculture (not irrigat 15.14 10.0 -3.5 15.19 13.49	5 5	-0.018164407	1882212	5584957 Domestic Wal	er; stock watering	14.2599	150	ט ל	10.59	14.25	I.59				0.02	-1%	Potenitally inemiclent/deep nswL	
O.05217043 1893933 Stocked impatrice motiting in the finite motiting in the constitution of the constitution	243	-0.449385862	1892688	25/8991	A continuity of the Continuity	14.34	9 5	ט ני	13.09	14.34	9. L9				C4.0	%AT		
L. 128010283 138384343 1383843 138384343 13838443	200	-0.461/3140/		5578545 Iffigation	Agriculture (not irrigat	15.19	700	0.5	15.19	5	11.09				0.40	020		
COMPOSITION DESCRIPTION STATE ADDRESS CONTRICTION STATE ADDRESS STATE ADDRESS<	t 8	-0.1/300/020		33/4134 DOINES IIC Wal	er ; stock watering	15.24	3 5	27.72	12.03	15.99	TO.70				0.10	707		
CLAGSS1307 SESSESS SECTION WATER STOCK Watering 1.5.3 9.0 7.50 7.50 7.50 0.20 0.1.408S3107 SESSESS SECTION WATER STOCK Watering 1.5.4 1.5	\$ 5	-0.055217045	1000419	02/2120	-	13.24	3 5	00.0-	12.17	15.00	0.01				0.00	T20		
0.240833107 18872 5554100 Groundwater sampling/monitoring 15.54 12.04 15.54 15.04 15.04 15.10 0.0 6.03 15.24 15.04 15.04 15.01 0.0 6.03 15.04	790	-0.278210958	1893683	558/022 Exploratory w	/ell	15.5	3 5	5.5	11.5	15.50	7.60				0.28	%T6		
0.0240833107 1898725 5564319 Groundwater sampling/monitoring 156 156 126 126 1560 6.04 1.84 0 4.20 0.24 0.176557311 1898028 5564319 Groundwater sampling/monitoring 16 -3.9 12.6 12.6 12.6 12.6 12.6 12.6 12.6 12.6	À :	-0.124958333	1891286	5562909 Domestic Wal	er Stock Watering	15.54	120	4-	13.64	15.54	9.04				0.12	%7		
0.2070/S0573 11 1899082 S566395 Groundwater sampling/monitoring 16 -3.9 13 16.00 9.10 174 0 7.36 0.18 -0.2070/S0573 11 1899082 5562395 Groundwater sampling/monitoring 16 100 -3.9 1.48 16.00 10.93 3.31 0 7.36 0.18 -0.1283751 1878380 5570304 Ukrknown 16.2 100 -2.13 16.2 14.07 2.38 0 1.1.69 0.13 -0.28836207 188599 5570304 Ukrknown 17.67 100 -2.13 16.2 14.07 2.38 0 11.69 0.13 -0.28862217 1889599 556686 Ukrknown Ukrknown 17.67 100 -3.2 14.02 5.28 2.13 0 6.22 0.03 -0.28862237 1898599 55567218 Domestic Water Grick Watering 18 150 -7.1 12 18.00 4.90 4.65 0 6.29 0.02 -0.2865290 189770	426	-0.240853107	1888752	5564190 Groundwater	sampling/monitoring	15.6	120	-6.56	12.6	15.60	6.04				0.24	%9		
0.0200708074 1892466 5572835 16 100 -3.9 14.83 16.00 10.33 3.31 0 7.62 0.20 -0.0239707954 1895615 5586363 Unknown Unknown 16.2 100 -1.52 16.2 0 14.68 8.46 0 6.22 0.04 -0.128373 187890 5570304 Unknown 17.06 300 -2.74 8.02 14.07 2.38 0 11.59 0.13 -0.38362607 188989901 5578734 Unknown 17.67 100 -3.2 16.45 17.67 13.25 6.36 0 6.90 0.36 -0.496352379 1899019 5578738 Domestic Water \$1000000000000000000000000000000000000	938	-0.176557311		5564395 Groundwater	sampling/monitoring	16		-3.9	13	16.00	9.10				0.18	7%		
0.039707954 1886615 Sissess Unknown Unknown 162 100 -1.52 162 162 0.04 0.1283751 1878380 570304 10.02 1.62 100 -2.13 16.2 10.0 11.69 0.13 0.09380651 1878390 556668 Unknown 17.06 10.0 -2.13 14.07 2.38 0 11.5 0.03 0.28032507 1888990 5586686 Unknown 17.06 10.0 -3.2 16.48 17.67 13.25 6.36 0 6.36 0.36 0.280325701 1893715 5578728 Domestic Water (Stock Watering) 18 10 -7.1 12 18.00 4.90 4.65 0 7.29 0.50 0.090526909 1879708 5566219 18.14 10 -3.9 18.4 19.0 7.29 0.21 0 2.2 0.405405499 1900000 556219 18.14 10 -3.9 18.4 10 2.2	201	-0.200708074		5572835		16	100	-3.9	14.83	16.00	10.93				0.20	3%		
-0-01283751 1878380 5570304	940	-0.039707954	1895615	5585635 Unknown	Unknown	16.2	100	-1.52	16.2		14.68				0.04	1%		
0.03836621 1879700 5566666 Unknown Unknown 17.06 300 -2.74 8.02 14.02 5.28 2.13 0 3.15 0.09 -0.38363627 1888599 55564402 Domestic Water funknown 17.7 100 -3.2 16.45 17.67 13.25 6.35 0 6.59 0.36 0.36 -0.208025701 1893715 5578712 Domestic Water fished Watering 18 150 -7.1 12 18.00 4.90 4.65 0 7.29 0.20 -0.09052609 1879708 5566219 18 10 -3.9 18 16.44 18.14 12.00 4.90 0 7.29 0.20 -0.09052609 1879708 5566219 18 10 -3.9 18 16.44 18.14 12.00 0 7.29 0 0 7.0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 <td>922</td> <td>-0.1283751</td> <td>1878380</td> <td>5570304</td> <td></td> <td>16.2</td> <td>100</td> <td>-2.13</td> <td>16.2</td> <td></td> <td>14.07</td> <td></td> <td></td> <td></td> <td>0.13</td> <td>1%</td> <td></td> <td></td>	922	-0.1283751	1878380	5570304		16.2	100	-2.13	16.2		14.07				0.13	1%		
-0.382836267 1898599 5581402 Domestic Water (Unknown 17.67 100 -3.2 16.45 17.67 13.25 6.35 0 6.90 0.36 -0.049332379 1899019 5787731 4.29 4.29 0 7.99 0.50 -0.049332379 1899019 5787728 4.29 4.29 0 7.99 0.50 -0.09052690 1879708 5556219 4.80 4.65 0 0 0.50 0 0.01 -0.04054091 19000 5560219 18 10 -39 16.84 18.14 4.90 4.65 0 0 1.0 0 0 0.02 0 1.0 0	839	-0.093806621	1879700	5566686 Unknown	Unknown	17.06	300	-2.74	8.02	14.02	5.28				0.09	3%		
-0.496352379 1889019 55/8731 Local Section 17.7 100 -3.2 15.48 16.76 12.28 4.29 0 7.99 0.50 -0.202035701 1893715 5577728 Domestic Watering 18 10 -7.1 12 18.0 4.90 4.65 0 0.25 0.21 8.0 -0.090526909 1879708 5566219 18 18 10 -3.9 18.1 10.9 -3.9 18.1 0 0.29 0 12.01 0.09 -0.0956090 5580732 18 10 -39 16.84 18.14 10.9 4.90 0 8.04 0.41 -0.22903833 1889314 5560121 Industrial (Commercial) 18.2 150 15 15 6.37 15 18.0 19.9 0 6.64 0.3	583	-0.358362607	1898599	5581402 Domestic Wat	er ! Unknown	17.67	100	-3.2	16.45	17.67					0.36	2%		
-0.089025701 1893715 5575728 Domestic Watering 18 150 -771 12 18.00 4.90 4.65 0 0.25 0.21 0.25 0.21 0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	6060	-0.496352379	1899019	5578731		17.7	100	-3.2	15.48	16.76					0.50	%9		
-0.0090526909 1879708 5566219 18 10 -3.9 18 14.10 2.09 0 12.01 0.09 -0.040540945 1900030 5580732 18.14 10 -3.9 16.84 18.14 12.94 4.90 0 8.04 0.41 -0.229023833 1889814 5566121 Industrial (Commercial) 18.2 150 -6.37 15 18.0 8.63 1.9 0 6.64 0.23	173	-0.208025701	1893715	5575728 Domestic Wat	er Stock Watering	18	120	-7.1	12	18.00	4.90				0.21	84%		
-0.40540945 1900030 5580732 -0.405400481 18.00 5580732 -0.405400482 18.00 5580732 -0.405400482 18.00 5580732 18.00 56.00 0.41 0.22 0.22 0.22 0.23 0.23 0.22 0.22 0.23 0.23	932	-0.090526909	1879708	5566219		18	100	-3.9	18		14.10				0.09	1%		
-0.229023833 1889814 556121 Industrial (Commercial) 18.2 150 -6.37 15 18.00 8.63 1.99 0 6.64 0.23	373	-0.40540945	1900030	5580732		18.14	100	-3.9	16.84	18.14	12.94	4.90			0.41	2%		
	161	-0.229023833	1889814	5566121 Industrial (Co.	mmercial)	18.2	120	-6.37	12	18.00	8 63	1 99		u	ccc	/oc		



Well Group 10 - 19.99m







Well Group 20 – 29.99m

Grid Value		N N						⋖	Available Variation	ariation Length		ing Predicted	remaining	
0100000	Easting	Northing Use1	Use2		Diameter h_SWL	- !	TopScreen1	BtmScr	lead (m) (t	8	٠,	DD (m)	head	Comments
0.0000000000000000000000000000000000000	0720201 14	-0.439655841 1899126 5578096 Domestic Water Sup	stic Water Sup	02 22	8 8	71.7	17.50	19.50	13.33	6.32	- ·	6.01	%/	Dotoutial lain in the contract of
C790202000			-in-	20.5	8 6	-2.13	71.07		50.7	74.11		o c		Potentially mellicient
-0.623032674			o do	2.07	3	-4.17	21.00		16.83	10.89	1 -			
-0.224564622				21.21	100	-4.17	19.51	21.21	15.34	4.65	-			
-0.466350846			ture (not irrig	21.34	100	-10.5	16.10		5.6	6.47	=		-25%	Potentially inefficient/ very deep hSWL
-0.21342182	32 1888442			21.48	100	-4.17	20.24		16.07	3.83	1 1:	11.24 0.21	. 2%	
-0.461334957	57 1899561	. 5578644 Irrigation Groundwa	on Groundwa	21.8	250	-3.4	15.80	21.80	12.4	92.9	1	4.84 0.46	10%	
-0.241109482			itic Stock Wat	22.06	20	-2	14.73	20.45	12.73	5.24	н			
-0.049141982			stic Stock Wat	22.25		9.0-	22.25		21.65	10.00	1			
-0.140402704				22.3	100	-5.79	15.24	16.46	9.45	3.63	-			
-0.331146673			stic Stock Wat	22.49	100	-4.9	22.49		17.59	9.16	-			
-0.162218178			stic Groundwa	22.5	120	-5.35	22.50		17.15	3.13	T .			
-0.602859567				22.6	100	-17.06	20.09	71	3.03	10.39	ਜ -			7% Potentially inefficient/very deep hSWL
-0.229971679				22.88	100	-4.17	16.89		12.72	4.90	- ;			
-0.062/84589			dwater sampli	23.51	100	-4.17	22.27	23.51	18.1	1.30				
-0.0278501.0-	COE00901 CL	53/4410 EE69213 Irrigation Grounday	and and	24.2	3 6	1	20.05		10.03	0 7 7	1 7	16.36	7%0	
-0.0446477			× × × × × × × × × × × × × × × × × × ×	24.75	3 5	0.0	20.02		20.55	6.79	1 -			
-0.082443358				27.52	3 5	717	27.72		20.00	30. 7	1 -			
-0.062443330			tic Stock Wat	24.23	3 5	-0.914	27.42	24.38	21.02	0.30	1 7			
-0.201990795			Disposal (Coo	24.4	150	-4.17	24.40		20.23	4.65	1 7			
-0,064428694			tic Stock Wat	24.7	100	-6.4	22.80	24.70	16.4	6.72	1 4			
-0.035125629			on	24.7	300	-0.35	7.00		6.65	4.99				
-0.192287756			tic Stock Wat	24.75	100	-1.65	23.25		21.6	4.09	1 1			
-0.09303065			Natering	25	150	-2.4	3.65		1.25	11.34	1 -1:		ľ	Very shallow top screen?
-0.060673106				25	100	-1.8	19.00		17.2	3.39	1 1.	12.81 0.06		
-0.150263323			on Unknown	25.1	300	-2.78	19.00	25.10	16.22	2.98	1			
-0.448279653			stic Stock Wat	25.2	100	-4.17	25.20		21.03	7.54	1 1.			
-0.015833055			dwater sampli	25.3	100	4	22.80		26.8	3.20	1 2.			
-0.387362223				25.5	100	2.44	19.87		22.31	9.61	r '			
-0.108824622			stic Stock Wat	25.6	100	-8.02	14.03	25.60	6.01	4.88	- ;		ω	
-0.302032403	23 188/13/17	5586533		25.6	3 5	-4.41	18.76		70.07	0.00	 	-5.04		2% -1% Dotantially inefficient/deen hCWI
-0.04-3007			tic Groundwin	25.0	3 5	-053	33.37		32.84	4 70	1 1			r Otenidany memorany deep mayor
-0.069620355				26	125	-4.17	23.45		19.28	6.32	1 11			
-0.553538332			awater sampli	79	150	-4.17	23.40		19.23	11.55	1			
-0.154539677				26.1	100	-4.17	25.10		20.93	5.37	1 14			
-0.494032326	26 1899092	5578730 Domestic Stock Wat	tic Stock Wat	26.13	100	7.94	24.13	26.13	32.07	6.61	1 2			
-0.16761384	34 1892756			26.2	100	-4.17	26.20		22.03	4.58	1 14	16.45 0.17		
-0.18572389	39 1890351	5563964		26.2	100	-4.17	26.20		22.03	4.77	1 14	16.26 0.19	1%	
-0.05522002	22 1885192	5565918		26.2	100	-4.17	23.30	26.20	19.13	5.26	1 1.	12.87 0.06	9%	
-0.169606832			tic Unknown	26.5	100	-1.2	23.50		22.3	4.88	1 1/			
-0.156843053				26.5	100	-4.17	25.20		21.03	4.95	1			
-0.342754454				26.52	100	-11.28	25.00		13.72	8.11	1			
-0.088187508				26.64	125	-4.17	26.64		22.47	4.02	T .			
-0.114839866			stic Water Sup	/7	001	-4.1/	77.30	73.60	18.13	10.52	- ;			
-0.16/061421	71 1892114	55/6428 Domestic Unknown	stic Unknown	27.13	9 5	-2.44	27.13	ŗ	24.69	66.40	A .	18.70 0.17	%1	
0.055250170			tic Water Sup	27.4	3 5	, <u>, , , , , , , , , , , , , , , , , , </u>	77.62	27.45	21 72	24.01	1 5			
-0.44430363			Vatering	27.4	8 6	-417	25.00		21.77	3.44	1 -			
0.00011067			tic Ctock Wot	27.42	3 5	77	2000		7.77	5 5	1 +			
-0.084032535			on Haknown	2 %	30.	-417	10.50		6 33	20.4	1 -		ď	
-0.195718894			5	78	100	-4.17	28.00		23.83	11.00	1 1			
-0.250109614	1894170	5586699 Domestic Agricultur	tic Agricultur	28.35	400	-19.51	26.67	28.35	7.16	10.91	7	-4.75 0.25	-2%	Potentially inefficient/deep hSWL
-0.57346657	57 1893814			28.35	100	-9.44	21.88	22.82	12.44	10.58		0.86 0.57	%29	
-0.067103441				28.5	100	-3.05	28.50		25.45	6.94	1 1.			
-0.447459938		1892214 5578630	:	28.5	100	-6.71	21.77	22.86	15.06	6.59	.	7.47 0.45	%9	



Well Group 20 – 29.99m

1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	%0	%0	2%	1%	1%	7%
0.14	0.09	0.16	0.16	0.17	0.17	0.04	0.12	0.16	0.20	0.04	0.04	90.0	0.18	0.09	0.11
19.48	18.32	20.34	16.69	17.26	14.93	5.99	12.85	19.06	19.87	28.02	18.59	3.91	22.86	8.64	7.80
1	1	1	1	1	1	1	1	1	1	1	1	1	Т	Т	+
4.80	4.07	4.76	4.77	4.91	4.98	2.67	10.48	4.94	4.66	7.00	2.00	5.31	4.95	2.61	10.38
25.28	23.39	26.1	22.46	23.17	20.91	12.66	24.33	22	25.53	36.02	24.59	10.22	28.81	12.25	19.18
28.80	28.80	28.90	28.95	28.96	25.56	28.83	29.50	28.90		29.80	29.76	18.96	0.00	15.50	28.08
28.50	27.56	25.90	26.63	27.34	23.71	16.83	28.50	28.23	29.70	28.40	28.76	17.53	29.87	13.70	26.19
-3.22	-4.17	0.2	-4.17	-4.17	-2.8	-4.17	-4.17	-3.23	-4.17	7.62	-4.17	-7.31	-1.06	-1.45	-7.01
100	100	100	100	100	100	150	100	100	100	100	150	100	150	200	100
28.8	28.8	28.9	28.95	59	59	53	29.5	29.6	29.7	29.8	29.8	29.82	29.87	29.9	29.9
3441 -0.144717178 1891139 5576463 Domestic Stock Wat	5563971 Groundwater sampli	5575949 Domestic Stock Wat	5575903 Irrigation Unknown	5576168 Domestic Unknown	5576330 Domestic Stock Wat	5560454 Stock Watering	5586561	5576628	5576705	5580133 Irrigation Unknown	5566276 Groundwater sampli	5566100 Domestic Stock Wat	-0.184662539 1887518 5567599 Domestic Unknown	5569942 Irrigation Unknown	-0.109682466 1895855 5586496 Domestic Stock Wat
1891139	1892933	1892238	1892456	1892454	1892338	1889627	1895561	1891314	1890086	1901720	1886888	1884573	1887518	1895868	1895855
-0.144717178	-0.092910743 1892933	-0.157589718	-0.163709287	-0.172051808 1892454	-0.173761932 1892338	-0.042124876	-0.118862234	-0.164029804	-0.204764169 1890086	-0.044457472 1901720	-0.036409181	-0.064315661	-0.184662539	-0.091864239 1895868	-0.109682466
3441	16507	4625	3707	2728	3714	4873	16882	10951	15394	2454	5335	2837	1398	2044	2289



Well Group 30 – 39.99m

ints		allow top screen level?				Shallow top screen, potentially inefficient			Deep SWL and shallow topscreen			-4% very shallow top screen level?													-	-22% very snallow top screen level?														Shallow top screen, potentially inefficient														
Comments		very shallow				Shallov						very sh													-	very sn																												
as % Remaining Head		ľ			%0	•							1%														1%								1%						7%			%0					1%					
Predicted DD (m)	0.61	0.15	0.16	0.13	0.07	0.09	0.04	0.57	0.06	0.14	0.20	0.17	0.16	0.00	0.18	0.09	0.11	0.11	0.16	0.16	0.17	0.16	0.17	0.14	0.13	0.39	0.15	0.10	0.33	0.17	0.13	0.17	0.13	0.17	0.06	0.18	0.17	0.13	0.16	0.06	U.15	0.00	0.17	0.05	0.16	0.17	0.12	0.05	0.08	0.16	0.07	0.00	0.18	
Remaining Pr	17.98	-21.04		12.91	15.69	-2.23	9.72	11.36	-6.52	5.46	14.91	-4.45	19.93	17 67	15.89	10.08	8.71	10.48	9.98	21.11	16.01	16.63	20.14	18.21	16.93	-1.78	18.31	10.02	17.99	13.63	19.10	19.50	15.97	14.01	5.48	20.53	22.98	14.71	18.31	-4.43	17.84	23.38	15.21	13.10	11.93	19.26	23.55	15.21	8.28	19.81	19.71	20.22	28.03	
Pump Length (m)	,	1	1	1	1	1	7	1	1	-	7	-	-	-		1	1	1	1	П	1	1	1	д.	-				H	1	1	1	П	-	A A	H	1	Т	1		7		1	1	1	1	1	₽,			H	П	1	
Seasonal P		7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	1
	8	-12.62	26.50	21.33	24.11	6.19	18.14	19.78	1.90	13.88	23.33	3.97	28.35	26.02	24.31	18.50	17.13	18.90	18.40	29.53	24.43	22.05	28.56	26.63	25.35	0.04	20.73	18.50	26.41	22.05	27.52	27.92	24.39	22.43	13.90	28.95	31.40	23.13	26.73	3.99	26.26	31.80	23.63	21.52	20.35	27.68	31.97	23.63	16.70	28.23	28.13	28.64	36.45	
Available TopScreen1 Head (m)	34.17	28.03	30.17				28.65	27.55	30.90	31.00	31.00	25.90	27.86	20.21	31.41	23.00	31.60	31.70	32.00	30.75	32.60		29.57	1	33.02	10.20		33.21		32.50		27.92	30.48	34.10	34.10	34.44		30.80		11	33.93	04.70	31.50	29.26	26.00	35.95		36.00	36.00	36.10	36.40	36.41		
opScreen1	32.77	1.53	28.17	30.48	30.48	9.45	27.58	26.15	16.50	28.18	29.70	5.18	26.22	28.41	29.61	21.50	23.50	28.80	24.00	28.92	30.80	31.42	28.56	33.00	31.72	8.00	33.10	31.81	32.78	30.60	33.89	25.92	28.96	28.80	31.40	32.76	34.44	29.50	33.10	10.36	32.03	35.00	30.00	27.89	23.00	34.05	35.97	30.00	24.00	34.60	34.50	35.01	36.57	
	37	-14.15	-1.67	-9.15	-6.37	-3.26	-9.44	-6.37	-14.6	-14.3	-6.37	-1.21	2.13	-2.33	-5.3	6-	-6.37	-9.9	-5.6	0.61	-6.37	-6.37	0	-6.37	-6.37	-1.30	-6.3/	-6.37	-6.37	-8.55	-6.37	2	-4.57	-6.37	-11.43	-3.81	-3.04	-6.37	-6.37	-6.37	-0.37	-9.3/	-6.37	-6.37	-2.65	-6.37	4-	-6.37	-7.3	-6.37	-6.37	-6.37	-0.12	
Diameter h SWL	100	100	100	100	100	100	100		75	120	100		100	2 5	100	100	300	200	150	100	100	100	100	200	100	300	3 5	001	100	150	100	100	100	150	100	100	100	100	100	100	3 5	061	100	100	100	100	150	300	150	150	100	125	100	00,
Depth	8	30.02	30.17	30.48	30.48	30.4899	30.5	30.5	30.9	31	31	31.08	31.1	31.4	31.41	31.5	31.6	31.7	32	32.3	32.6	32.62	32.9	33	33	33.1	23.1	33.3	33.38	33.53	33.89	34	34.1	34.1	34.1	34.44	34.44	34.5	34.6	34.7	7.4.7	35.75	32	35.05	35.85	35.95	35.97	36	36	36.1	36.4	36.41	36.57	1
Use2	Stock Wat	5563198 Domestic Stock Wat	5576245 Domestic Unknown	5586536 Unknown Unknown	5562700 Domestic Stock Wat	5565851 Domestic Stock Wat				5564189 Domestic Stock Wat	5575621 Domestic Stock Wat	5575978 Domestic Unknown	5576008 5564739 Prigation Habbourn		5575096 Domestic Stock Wat			5564205 Domestic Stock Wat	5563937 Domestic Stock Wat					5568426 Irrigation Unknown	5586611 Domestic Water Sup	55/3322 Exploratol Unknown	55/6195 Groundwater sampli	55/5545 5586336 Stock Wat Domestic		5564089 Domestic Stock Wat		5576193 Domestic Stock Wat			5564619 Public Water Supply 5584734 Agricultur Unknown	5576228 Domestic Unknown		5586766 Domestic Water Sup				5576102 Domestic Stock Wat	5575926 Domestic Water Sup	5585535 Domestic Unknown	Domestic		5562909 Domestic Stock Wat	5574873 Domestic Stock Wat	5561392 Domestic Stock Wat 5581613				5576528 Domestic Unknown	
Use1	Irrigation	Domestic	Domestic	Unknown	Domestic	Domestic				Domestic	Domestic	Domestic	rrigation	IIIgarioii	Domestic			Domestic	Domestic					Irrigation	Domestic	Explorator	Groundwa	Stock Wat		Domestic		Domestic		5576319 Irrigation	Public Wa	Domestic		Domestic				Domestic	Domestic	Domestic	5576429 Fire-fighti Domestic		Domestic	Domestic	Domestic				Domestic	1 1 1 1 1 1 1
NZTM_ Northing Use1	5583516	5563198	5576245		5562700	5565851	5585036	5584855	5560917	5564189	5575621	5575978	5576008	5584735	5575096	5566920		5564205	5563937	9609255	5576269	5576253	5576228	5568426	5586611	55/3322	25/0195			5564089	5586444	5576193	5583334	5576319	5564619	5576228	5575908	2286766	5575054	5578127	22/07/2	5576102	5575926	5585535	5576429		2262909			5575650	5564423	5566540	5576528	
	4.253	-0.15162 1890871.587	-0.16443 1892172.285	-0.13110 1895214.141	-0.07396 1892618.788	-0.08726 1881351.262	-0.04450 1898516.857	-0.57263 1893796.546	-0.06095 1889314.887	-0.13830 1891854.092	1888754.134	-0.16728 1892474.542	0.16345 1892354.455	-0.00133 1880134.441	-0.18145 1887910.688	1881709.3		-0.10557 1892677.629	-0.16226 1891148.667	-0.15697 1892081.253	-0.16783 1892239.326	-0.16470 1892173.284	-0.16949 1892314.385	-0.14448 1890245.411	0.12504 1895424.271	0.46474 1808302.471	-0.16474 1892218.32b	-0.22/12 1894219.814 -0.10059 1895848.639	-0.33406 1898363.606	-0.17397 1890900.485	-0.12918 1895127.101	-0.17208 1892433.472	-0.12937 1899017.601	-0.17499 1892407.429	-0.06048 1884411.318 -0.58954 1893713.517	-0.17537 1892514.52	-0.16823 1890426.181	-0.13383 1895494.283	-0.15940 1892505.742	0.05748 1880706.467	-0.1518/ 1891569.8/8	-0.15807 1892108.27	-0.16754 1892523.586	-0.05420 1895814.806	-0.15722 1891898.064	-0.16542 1892305.401	-0.12496 1891285.893	-0.05104 1899528.643	-0.08442 1889540.97 -0.29852 1895379 386	-0.15751 1891384.872	-0.06715 1886429.597	1871520.464	-0.17966 1892414.393	
Point Sample NZTM_ - Grid Value Easting	-0.60587	-0.15162	-0.16443	-0.13110	-0.07396	-0.08726	-0.04450	-0.57263	-0.06095	-0.13830	-0.19839	-0.16728	-0.16345	-0.00133	-0.18145	-0.09041	-0.10551	-0.10557	-0.16226	-0.15697	-0.16783	-0.16470	-0.16949	-0.14448	-0.12504	-0.38856	-0.164/4	-0.10059	-0.33406	-0.17397	-0.12918	-0.17208	-0.12937	-0.17499	-0.58954	-0.17537	-0.16823	-0.13383	-0.15940	-0.05748	-0.1518/	-0.15807	-0.16754	-0.05420	-0.15722	-0.16542	-0.12496	-0.05104	-0.08442	-0.15751	-0.06715		-0.17966	00000
ObjectID		2926	1599	1961	3596	2729	10898	10979	10895	2900	4623	2779	10906	10057	4452	10943	16836	2911	2901	11003	15445	17003	10892	2102	5206	2543	15044	4154	5829	2898	16806	3349	10897	10917	10924	1395	10992	5319	5361	10894	1,032	4765	5188	1407	4712	2898	2910	4049	15479	15997	2987	16434	1847	OFFIC



-0.31256 189228.383 55873436 O.0. -0.34126 28.71 -0.85 27.80 28.71 -0.85 7.42 1 -9.27 0.31 -0.15571 189228.83 5576139 Domestic Stock Wat 37 100 -0.3 37.20 29.43 7.42 1 2.120 0.17 -0.15673 189228.83 5576139 Domestic Mater Sup 37.5 100 -6.37 36.00 37.50 29.63 7.42 1 21.13 0.16 -0.15673 189228.31 557610 Unrigation Unknown 37.5 100 -6.37 36.00 37.50 29.63 7.42 1 21.13 0.16 -0.0271 188021.05 556977 Unknown 37.5 100 -5.36 37.50 29.63 7.42 1 21.11 0.16 -0.1866 1890230.44 556977 Unknown 10.40 -7.2 32.00 37.50 23.80 7.42 1 21.11 0.16 -0.1866 189944.35 5564119 Unknown 1		N	el	1 (GI	0	UĮ	0	3() -	- ;	39	9.9	79	m	1							
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0.31256 1899211.815 588436 36.6 100 -28.65 27.80 28.71 -0.85 7.42 1 -9.27 0.1326 189221.815 558739 bonnestic Srock Wat 37.5 100 -0.9 30.32 31.52 29.42 7.42 1 5.10 0.15673 1892288 613 557610 Inrigation Unknown 37.5 100 -6.37 36.00 37.50 29.63 7.42 1 26.03 0.15673 189223.045 557863 Domestic Water Sup 37.5 100 -6.37 35.90 37.50 29.63 7.42 1 26.11 0.01567 189294 686 557863 Domestic Water Sup 37.5 100 -1 1.37 36.90 37.50 29.63 7.42 1 26.11 0.02061 189294 686 5586336 37.6 100 -1 1.37 32.90 37.5 1 25.42 1 26.37 3.60 37.5 1 26.33 3.69 37.5 1 26.31 3.69 37.50 29.63 3.42<	0.31	0.17	0.17	0.16	0.16	90.0	0.24	0.10	0.19	0.22	0.20	90.0	0.15	0.13	0.01	0.13	0.16	60.0	0.13	0.41	0.05	0.11	0.05
-0.31256 1899211.815 5587436 36.6 100 -28.65 27.80 28.71 -0.08 -0.1573 1 1892220.341 5567010 Ingineating Indictional Unknown 37.5 100 -6.37 35.20 38.60 34.80 -0.15672 1 1892220.341 5576126 Ingineating Unknown 37.5 100 -6.37 35.90 37.50 29.42 -0.16673 1 1892220.341 5576126 Ingineating Water Sup 37.5 100 -6.37 35.90 37.50 29.63 -0.16673 1 1901320.52 5575863 Domestic Water Sup 37.5 100 -6.37 35.90 37.50 29.63 -0.16673 1 1901320.52 556712 Unknown 37.5 100 -1.36 37.50 33.84 -0.18695 1 188931.4 556712 Unknown 10.00 -1.2 32.00 33.60 24.80 -0.18695 1 189941.4 55 564119 Unknown 10.00 -1.2 32.00 33.60 24.80 -0.1561 1 189671.4 5564119 Unknown						-8.05															1.98	4.00	15.91
-0.31256 1899211.815 5587436 36.6 100 -28.65 27.80 28.71 -0.08 -0.1573 1 1892220.341 5567010 Ingineating Indictional Unknown 37.5 100 -6.37 35.20 38.60 34.80 -0.15672 1 1892220.341 5576126 Ingineating Unknown 37.5 100 -6.37 35.90 37.50 29.42 -0.16673 1 1892220.341 5576126 Ingineating Water Sup 37.5 100 -6.37 35.90 37.50 29.63 -0.16673 1 1901320.52 5575863 Domestic Water Sup 37.5 100 -6.37 35.90 37.50 29.63 -0.16673 1 1901320.52 556712 Unknown 37.5 100 -1.36 37.50 33.84 -0.18695 1 188931.4 556712 Unknown 10.00 -1.2 32.00 33.60 24.80 -0.18695 1 189941.4 55 564119 Unknown 10.00 -1.2 32.00 33.60 24.80 -0.1561 1 189671.4 5564119 Unknown	н	1	1	1	н	1	1	1	П	1	1	1	1	1	1	1	1	1	1	1	7	1	₩
0.13255 1892221.1815 5587436 36.6 100 -28.65 27.80 28.71 0.1326 1892228.838 5576139 Domestic Stock Wat 37.5 100 -0.9 30.32 31.52 0.1531 189228.861 5576126 Domestic Water Sup 37.5 100 -6.37 35.00 37.50 0.1637 189228.041 5576126 Domestic Water Sup 37.5 100 -6.37 35.00 37.50 0.0606 1910320.592 5576730 Domestic Water Sup 37.5 100 -6.37 35.00 37.50 0.0607 191330.992 558631 Linknown 37.5 100 -7.2 35.00 37.50 0.0007 198691 1888914.855 5569711 Unknown Unknown 38 100 -7.2 32.00 38.00 0.18695 1888944.825 5564119 Unknown Unknown 38 100 -6.37 30.0 30.0 0.1561 1888944.825 5564119 Unknown Unknown	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42	7.42
-0.31256 1892221.815 5587436 366 100 -28.65 27.80 -0.31256 1892221.815 5587436 27.80 -0.156.37 1892228.83 55.76139 Donnestic Christophy 37.2 100 -6.37 37.20 -0.156.37 1892220.34 55.76136 Donnestic Water Sup 37.5 100 -6.37 36.00 -0.156.39 1892327.465 55.75830 Donnestic Water Sup 37.5 100 -6.37 36.00 -0.156.39 1892327.465 55.75830 Donnestic Water Sup 37.5 100 -6.37 35.90 -0.10027 1892914.665 5589731 Unknown 37.5 100 -5.37 100 -5.37 35.90 -0.10027 1892914.665 5589731 Unknown Unknown 38 200 -6.37 20.00 -0.21574 188934.4.52 5564119 Unknown Unknown 38 200 -6.37 30.00 -0.21574 1889414.52 5564119 Unknown Unknown 38 200 -6.37 30.00 -0.21574 1889414.52 556419 Unknown 38 200 -6.37 30.00 -0.21574 1889414.52 556419 Unknown 38 200 -6.37 30.00 -0.2157 1892923.54 58 559334 Irrigation Unknown 38.4 100 -5.14 37.04 -0.01261 1895714.447 5586736 Donnestic Chrknown 38.4 100 -9.14 37.04 -0.01261 1895714.447 5586736 Donnestic Chrknown 38.4 100 -9.14 37.04 -0.01261 1895273.45 556438 Donnestic Chrknown 38 100 -6.37 31.46 -0.02891 1890318.24 5556438 Donnestic Chrknown 39 100 -6.37 31.46 -0.03891 1890318.24 5556438 Donnestic Chrknown 39 100 -1.13 100 -1.14 36.95 -0.04677 1890414 5586433 Donnestic Chrknown 39 100 -1.14 36.95 -0.04677 1890414 56 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04677 1890414 56 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518.08 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 5586431 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 56 5580631 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 56 5580631 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 56 5580631 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 56 5580631 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 56 5580631 Donnestic Chrknown 39 100 -1.14 36.95 -0.04671 1890518 56 5580431 Donnestic Chrknown 39	-0.85	29.42	34.80	29.63	29.53	0.37	33.84	16.94	24.80	-6.37	31.63	23.80	27.57	27.90	41.59	18.90	29.59	25.09	22.95	14.55	10.40	12.42	24.33
-0.31256 1892211.815 5587436 Onestic Stock Wat 27. 100 -2.8.65 -0.17310 1892221.8328.385 5576139 Domestic Mater Sup 37.2 100 -0.09 -0.15639 1892228.431 5576010 Irrigation Unknown 37.2 100 -6.37 -0.15639 1892327.465 5575863 Domestic Water Sup 37.5 100 -6.37 -0.00608 1901320.592 5576730 -0.00608 1901320.592 5576730 -0.00608 1901320.592 5576730 -0.00608 1901320.592 5576730 -0.00608 1901320.592 5576730 -0.00608 1901320.592 5576730 -0.00608 1901320.592 5576730 -0.00608 189936.36 5575089 Domestic Stock Wat 38 150 -7.2 -0.21574 1889344.525 5564119 Unknown Unknown 38 200 -6.37 -0.01569 1895474.83 5595134 Irrigation Unknown 38.41 100 -5.14 -0.012614 1895714.447 5586736 Domestic Stock Wat 38.4 100 -9.14 -0.012614 1895714.447 5586736 Domestic Unknown 38.41 100 -9.14 -0.01261 1895715.33 5576136 Domestic Unknown 38.41 100 -0.1261 -0.1261 1895718.345 5576386 Domestic Unknown 38.41 100 -0.1261 -0.1261 1895718.345 5576386 Domestic Unknown 38.41 100 -0.1261 -0.1261 1895718.345 5576386 Domestic Stock Wat 39.13 100 -0.1261 -0.1261 1895718.345 5576386 Domestic Stock Wat 39.13 100 -0.146 -0.04677 1899404.66 5586438 Domestic Stock Wat 39.13 100 -0.146 -0.04677 1899404.66 5586438 Domestic Stock Wat 39.13 100 -1.14 -0.0429 1895718.893 5586433 Domestic Stock Wat 39.13 100 -1.14 -0.0431 1890518.08 5586433 Domestic Stock Wat 39.13 100 -1.14 -0.0429 1100 -0.14 1000 -0.1	28.71	31.52	38.60	37.50	37.50	3.69		32.33	38.00	00:00		35.70	32.37	38.41	43.90	31.70	32.50	36.36	39.13	35.84	39.40	39.62	32.46
-0.31256 1892211.815 S587436	27.80	30.32	37.20	36.00	35.90	1.37	37.50	21.97	32.00	0.00	38.00	24.10	30.57	37.04	42.90	30.10	30.40	31.46	36.95	34.35	22.40	37.92	31.52
-0.31256 1892221.1815 5589.436 36.6 -0.13120 1892288.133 5576139 Domestic Stock Wat 37 -0.15637 1892220.341 5576126 Domestic Water Sup 37.2 -0.15637 1892220.345 5576230 Domestic Water Sup 37.5 -0.06068 1901320.545 5576330 Domestic Water Sup 37.5 -0.00068 1901320.545 55876330 Domestic Water Sup 37.5 -0.00271 1898091.686 5586336 37.5 -0.10027 1889396.36 5576730 Domestic Stock Wat 38 -0.21574 1889414.525 5564119 Unknown Unknown 38 -0.21574 1889414.525 5564119 Unknown Unknown 38 -0.00618 1895735.6893317 38 -0.00618 1895735.69 Domestic Stock Wat 38.4 -0.12614 1895714.447 5586736 Domestic Stock Wat 38.4 -0.12614 1895714.447 5586736 Domestic Unknown 38.61 -0.12614 1895714.47 5586736 Domestic Unknown 38.61 -0.12614 1895714.647 5586736 Domestic Unknown 38.61 -0.01429 1901328.948 5574786 Groundwater sampli 38.6 -0.126306 1892434.550 5574786 Groundwater sampli 38.6 -0.126306 189244.45 55674348 Domestic Unknown 39 -0.0283 1880318.08 55864348 Domestic Chrkwat 39 -0.04571 189044, 646 5580631	-28.65	-0.9	-2.4	-6.37	-6.37	-	-3.66	-5.03	-7.2	-6.37	-6.37	-0.3	ç.	-9.14	-1.31	-11.2	-0.81	-6.37	-14	-19.8	-12	-25.5	-7.19
-0.31256 1892221.815 5589346 -0.13557 1892288.385 5576139 Domestic Stock Watt -0.15637 1892280.345 5576139 Domestic Mater Supp. 0.15639 1892287.465 5575863 Domestic Water Supp. 0.15639 1892327.465 5575863 Domestic Water Supp. 0.15639 189244.655 5586370 Unknown -0.18695 1888936.36 5576730 Domestic Stock Watt. 0.21574 188934.4525 5564119 Unknown Unknown -0.18695 1889434.525 5564119 Unknown Unknown -0.12696 189244.825 5558419 Unknown Unknown -0.1269 1896735.88 5578366 Domestic Stock Watt. 0.1261 1895734.88 557836 Domestic Unknown -0.1285 1895165.131 5586438 Domestic Unknown -0.1285 1895165.131 5586438 Domestic Stock Watt. 0.1285 1895165.131 5586438 Domestic Stock Watt. 0.04329 1895148.083 5576438 Domestic Stock Watt. 0.04359 1895148.083 5556438 Domestic Stock Watt. 0.04359 1895148.083 5586438 Domestic Stock Watt. 0.04371 189044.466 5580631	100	100	100	100	100	100	150	100	150	200		150	100	100	150	100	100	100	100	100	250	100	100
0.31256 1892211.815 0.16557 1892288.385 0.14672 1892220.341 0.15673 1892220.341 0.15673 1892924.685 0.10027 1895914.685 0.18695 1898944.525 0.21574 1889414.525 0.21574 1889414.525 0.12661 1895714.447 0.012614 1895714.447 0.012614 1895714.447 0.012618 1896237.345 0.026918 1896345.33 0.12618 1896235.58 0.036918 1896237.345 0.036918 1896348.447 0.012618 1896237.345 0.026918 1896348.447 0.036918 1896348.447 0.036918 1896348.447 0.036918 1896348.447 0.036918 1896348.447 0.036918 1896348.447 0.036918 1896348.447	36.6	37	37.2	37.5	37.5	37.5	37.5	37.6	88	38	88	38.4	38.4	38.41	38.6	38.8	38.91	39	39.13	39.3	39.4	39.62	39.9
4538 4538 5509 5509 10899 10899 10899 4772 6714 6714 6714 12664 2077 4471 1476 4694 4694 4694 2077 2082 2083 2173 3581	-0.31256 1892211.815 5587436	-0.16557 1892288.385 5576139 Domestic Stock Wat							1888936.36												-0.05110 1896855.638 5589256	-0.11172 1886211.202 5571944 Irrigation Unknown	-0.05340 1897815.735 5587637
	11004	4538	2053	5359	5509	10899	10941	10889	4772	6714	15664	2077	4471	1476	4694	4156	2082	5173	3581	10981	4271	3770	10967



Well Group 40 – 49.99m

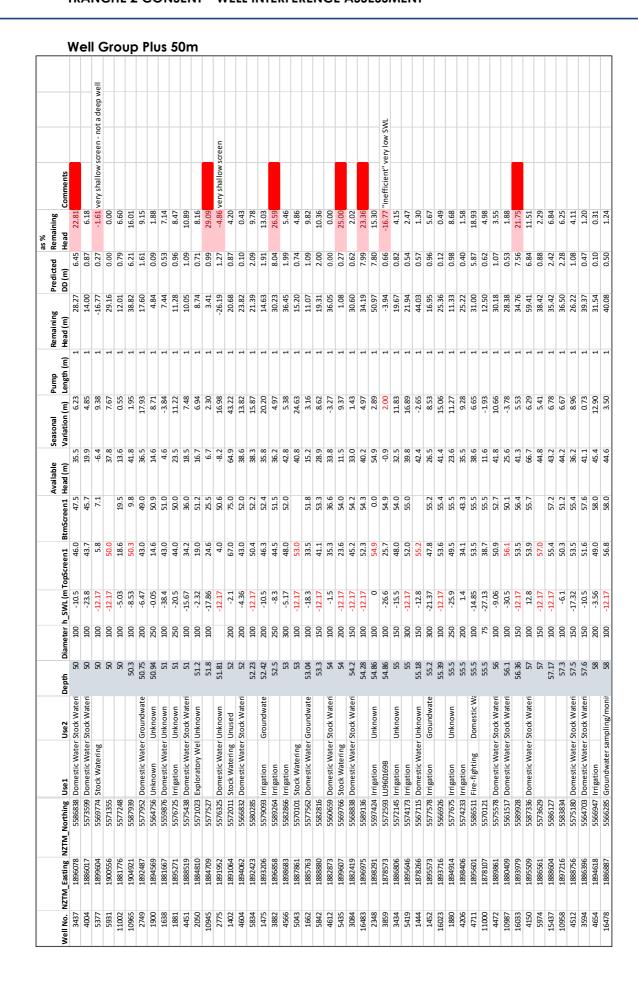
																																0% -1% very shallow top screen	pscreen		SWL														2% Very challow for creen	Top scient	
Comments																																very shallo	-1% shallow top screen		-2% Very deep SWL													17473 00 00	-2% Very shallo	very singin	
Remaining Head	%9	1%	3%	1%	2%	2%	1%	2%	1%	3%	1%	2%	1%	%0	1%	1%	1%	7%	4%	%0	1%	7%	1%	1%	2%	7%	1%	1%	1%	1%	1%	-1%	-1%	1%	-5%	%9	3%	2%	1%	1%	1%	%1	%0	1%	7%	1%	2%	1%	%2-	%0	2%
Predicted R DD(m) H	0.07	0.07	0.31	0.11 0.44	0.17	0.12	0.16	0.21	0.14	0.07	0.17	0.16	0.17	90.0	90.0	0.13	0.15	0.17	0.39	0.00	0.05	0.29	0.14	0.18	0.10	0.26	0.12	0.15	0.11	0.20	0.19	0.03	0.07	0.24	0.31	0.23	0.25	0.25	0.11	0.20	0.19	0.1/	0.05	0.11	0.50	0.10	0.18	0.18	0.04	0.00	0.50
Remaining P Head (m) D	4.	10.80	12.33	13.54	10.12	2.35	17.84	11.83	13.94	2.47	15.78	10.43	16.81	14.53	7.33	10.59	11.91	17.71	10.74	15.33	10.34	13.83	10.29	19.54	2.01	13.13	14.83	15.84	9.22	17.33	17.33	-10.88	-6.50	16.27	-18.72	3.83	8.04	16.48	12.79	17.56	18.53	17.19	12.88	12.64	27.45	16.64	11.31	14.59	-2.47	23.78	20.07
	П	П	Η,		1 41	н	-	П	-	н	П	П	1	н	н	τ τ			1 4	1 4	П	П	1	н	τΗ τ			. 4	1	Н.			1 41	Н	П	ਜ ਦ	1 4	1 41	1	1	₩,	- ·			П	н	₩ .		-1 -	1 41	,
Seasonal Pump Variation (m) Length (m)	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	00 04
Available Se Head (m) Va	18.80	28.46	29.99	31.20	27.78	20.01	35.50	29.49	31.60	20.13	33.44	28.09	34.47	32.19	24.99	28.25	75.35	27.30	28.40	32.99	28.00	31.49	27.95	37.20	19.67	30.79	32.49	33.50	26.88	34.99	34.99	52.34	11.16	33.93	-1.06	20.05	25.70	34.14	30.45	35.22	36.19	34.85	30.54	30.30	45.11	34.30	28.97	32.25	15.19	41.44	
	33.60	38.67	40.20	39.30	40.48	30.82		40.50	42.80	40.84	34.54	40.00	39.96		40.00	38.62	41.48	37.50	40.50		41.10	42.50	38.68	45.60	5	29.62	43.30	42.00	43.58	45.00	45.00	38.14	40.84	44.24	38.56	33.50	43.30	44.15	41.91	45.13		45.36	43.57	45.70	45.72	45.72	46.33	45.11	5 79	6.46	
TopScreen1 BtmScreen1	22.40	37.47	39.00	37.60	36.48	29.02	39.00	38.50	40.70	36.73	32.34	37.10	38.13	41.20	34.00	37.26	38.58	36.40	30.50	42.00	35.92	40.50	35.57	36.60	42.67	39.80	41.50	40.20	41.88	44.00	44.00	36.24	23.16	45.94	37.04	30.50	37.30	43.15	40.39	44.23	45.20	43.86	39.31	43.30	45.11	44.20	34.45	39.11	37.19	46.80	
	-3.6	-9.01	-9.01	-b.4 -12 96	-8.7	-9.01	-3.5	-9.01	-9.1	-16.6	1.1	-9.01	-3.66	-9.01	-9.01	-9.01	-9.01	2.0.	-2.1	-9.01	-7.92	-9.01	-7.62	9.0	-23	-9.01	-9.01	-6.7	-15	-9.01	-9.01	-3.9	-12	-9.01	-38.1	-9.01	-11.6	-9.01	-9.94	-9.01	-9.01	-9.0 <u>1</u>	-8.77	-13	0	-9.9	-5.48	-6.857	77-	-5.36	
Diameter h_SWL	150	150	100	9 5	125	100	100	100	100	150	100	150	150	150	200	100	55	3 5	250	100		100	100	200	100	000	100	100	100	100	100	200	150	100	100	200	200	100	100	100	100	8 8	250	100	100	100	250	700	3 5	100	
Depth	40	40	40.2	40.7	40.48	40.5	40.5	40.5	40.7	40.84			41.1	41.2	41.2	41.22	41.48	41.7	42	42	42.1	42.5	42.6	42.67	42.67	42.67	43.3	43.5	43.58	4	4 :	44.1	44.2	44.24	44.5	44.5	45	45	45.1	45.13	45.2	45.4	45.52	45.7	45.72	45.72	46.33	46.33	46.63	46.8	
Use2	5565269 Domestic Stock Wat	itering	5594379 Domestic Unknown	556 / 122 5578820 Domestic Stock Wat	5571738 Domestic Stock Wat	5586566 Domestic Unknown	5575779 Domestic Stock Wat	: Stock Wat	5586617 Irrigation Unknown	5569185 Agricultur Unknown	5576269 Domestic (Communa			5567947 Domestic Stock Wat			55/54/6 5575200 Domostic Stock Wat	55/6299 Domestic Mater Sun	5583800 Irrigation Unknown			5576505 Domestic Stock Wat	5568122 Domestic Stock Wat	5575828 Agricultur Unknown	5562026 Domestic Stock Wat	tering	5586636 Unknown Unknown	5567040 Domestic Stock Wat	5586317 Domestic Stock Wat			5570620 Unknown Unknown	5563519 Stock Wat Unknown		5587464 Domestic Unknown	5564676 Unknown Unknown	5564069 Unknown Unknown					55/6419 Domestic Water Sup	5587727 Groundwa Unknown	5586236 Domestic Unknown	5588440 Agricultur Unknown	5586260 Domestic Unknown	5576316 Irrigation Unknown	5574925 Irrigation Groundwe	IL OHKHOWH		
Use1	Domesti	5565401 Stock Watering	Domesti	Domecti	Domesti	Domesti	Domesti	5577256 Domestic	Irrigation	Agricultu	Domesti			Domesti	5571221 Irrigation		Jomoc i	Domesti	Irrigation	0		Domesti	Domesti	Agricultu	Domesti	5575423 Stock Watering	Unknow	Domesti	Domesti			Unknow	Stock Wa		Domesti	Unknow	Unknow					Domesti	Groundw	Domesti	Agricultu	Domesti	Irrigation	Irrigation Ctock Wo	STOCK We		
NZTM_ Northing Use1													5566922																										5586336												
NZTM_ Easting				1892/16		1895514	1891234	1892635	1895227		1892346			1894881	1884732		1892025			1900122	1897817					1895270			1895233		1890682	1895638				1889254			1895314			18921/2						1888469		1900721	П
- e	-0.070678836	-0.071889535	-0.313767697	-0.1053/0856	-0.167282118	-0.120130431	-0.156787641	-0.210293374	-0.136074429	-0.066729001	-0.171926308	-0.15891854	-0.166110097	-0.057952049	-0.055711322	-0.127552198	0.167077659	-0.16/9//030 -0.19599333	-0.388107073		-0.053301613	-0.289674926	-0.143078907	-0.180302403	-0.103747276	-0.257973239	-0.12023296	-0.148045067	-0.11235601	-0.198513163	-0.191447649	-0.128888693	-0.071554575	-0.241772706	-0.308973824	-0.228671883	-0.245210968	-0.249062769	-0.108700224	-0.200351739	-0.190682666	-0.169450118	-0.049767186	-0.107089	-0.49716299	-0.097697606	-0.181101655	-0.183602303	-0.042948189	0.303201111	שרניטרוטר ט
Point Samp ObjectID Grid Value	3615	2699	1447	3487	4545	2324	4952	5140	2068	1401	4344	17013	10977	4084	4996	5881	16402	4300	1482	16627	10966	2805	2251	2019	3082	5724	1433	3474	3593	15870	15935	2002	2931	15955	1439	1027	6716	15954	10947	15869	16264	5486	1485	2573	1358	1390	1394	1430	10900	10963	10CA



Well Group 40 – 49.99m

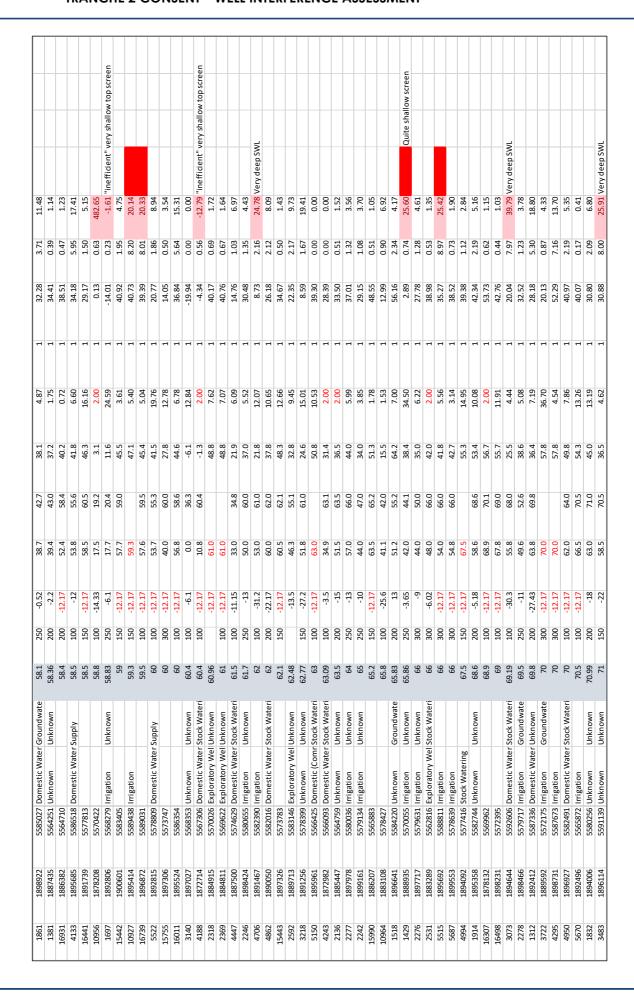
			21% deep SWL														-0.4% Shallow top screen		0 10/ door CM/I
%0	1%	1%	21%	1%	1%	7%	%0	1%	1%	%0	1%	1%	7%	1%	%0	%0	-0.4%	%0	7070
0.04	0.25	0.12	0.48	0.14	0.23	0.37	0.04	90.0	0.10	0.01	0.30	0.18	0.15	0.14	90.0	0.04	0.05	0.05	000
14.68	45.84	16.44	2.33	15.58	21.73	19.08	11.07	8.72	20.03	18.54	21.33	13.84	6.33	12.13	22.93	19.08	-11.03	20.54	10.0
1	1	1	П	1	1	П	₽	1	Н	П	1	Н	П	₽	1	Н	П	П	,
16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	16.66	77.74
32.34	63.50	34.10	19.99	33.24	39.39	36.74	28.73	26.38	37.69	36.20	38.99	31.50	23.99	29.79	40.59	36.74	6.63	38.20	00,
47.80	76.00	47.50	48.16	48.16	0.00	48.65	48.80	33.48	48.90	44.50	49.20		35.00	41.70		47.30	10.23	49.68	70 00
41.35	74.00	46.20	43.59	47.24	48.40	45.75	33.53	31.08	46.70	42.50	48.00	49.30	33.00	38.80	49.60	45.75	8.23	47.21	00 10
-9.01	-10.5	-12.1	-23.6	-14	-9.01	-9.01	-4.8	-4.7	-9.01	-6.3	-9.01	-17.8	-9.01	-9.01	-9.01	-9.01	-1.6	-9.01	77 77
200	150	100	150	100	200	150	100	150	150	100	200	200	100	150	150	150	100		5
47.8	48	48	48.15	48.2	48.4	48.65	48.8	48.9	48.9	49	49.2	49.3	49.5	49.5	49.6	49.6	49.68	49.7	100
	Unknown	Stock Wat	Unknown	Stock Wat	Unknown		Unknown	Unknown		Stock Wat			Water Sup			Water Sup	Unknown		
5590624	5570932 Irrigation Unknown	5586752 Domestic Stock Wat	5580097 Domestic Unknown	5586656 Domestic Stock Wat	5565095 Unknown Unknown	5574587	5576336 Unknown Unknown	5563940 Public Wa Unknown	5569010	5567208 Domestic Stock Wat	5577234	5571583 Irrigation	5575542 Domestic Water Sup	5587331	5564233 Irrigation	5591700 Domestic Water Sup	5573925 Stock Wat Unknown	5574025	04.4077
1897202	1889390	1895857	1891874	1895164	1889411	1896815	1884119	1886180	1893349	1871854	1895567	1887512	1891927	1895958	1886138	1898270	1884109	1884109	100001
-0.035065048 1897202	-0.247859295 1889390	-0.123521989	-0.484870799 1891874	-0.142749828 1895164	-0.230070555 1889411	-0.37345137	-0.036954085 1884119	-0.064208979 1886180	-0.101396064 1893349	-0.012105785	-0.299183047	-0.177412596 1887512	-0.146079949 1891927	-0.137034129 1895958	-0.061429994 1886138	-0.042346761 1898270	-0.047461835 1884109	-0.047122765 1884109	0444077 740004 47040000
15892	1532	4378	2730	2736	6718	16363	3188	1762	16371	3444	16567	4722	4912	2978	4128	5389	2903	10939	01001















																											23.67 Very deep SWL																				
77.7	1.32	1.54	4.41	1.92	1.80	0.23	0.57	0.45	00.00	0.52	11.56	4.65	0.40	0.00	2.47	6.05	3.56	0.57	1.95	0.39	8.76	1.04	0.66	4.06	4.05	4.89	23.67	1.98	2.85	0.53	0.00	0.54	0.00	0.45	10.03	12.61	8.05	3.85	0.50	1.46	0.00	1.52	1.30	0.79	00.00	2.04	1.35
17.1	0.61	0.54	1.81	0.70	0.79	0.10	0.09	0.23	0.00	0.12	2.11	1.12	0.25	0.00	1.02	1.91	1.84	0.24	1.09	0.17	1.10	0.64	0.23	2.12	2.38	0.84	1.49	0.86	0.86	0.24	0.00	0.34	0.00	0.23	4.34	0.78	5.70	2.23	0.24	0.99	0.00	0.24	0.71	0.56	0.00	0.54	0.54
53.I4	46.40	35.45	41.10	36.56	44.18	44.86	16.11	52.06	59.23	22.50	18.25	24.04	62.37	56.76	41.44	31.53	51.74	42.24	55.94	42.40	12.57	61.93	35.48	52.36	58.85	17.27	6.30	43.19	30.07	44.83	34.25	63.52	65.03	51.07	43.30	6.18	70.78	58.05	47.91	68.24	52.46	15.95	54.36	70.17	67.86	26.23	40.04
7	н	1	1	П	1	1	н	П	1	Н	1	П	П	1	Н	Н	1	1	1	П	1	1	1	1	1	1	н	1	т	П	-	н,		-1 -		ı ==	н	П	н	П	1	1	1	П	1	1	н
4.69	3.93	3.38	9.90	23.49	15.65	14.30	9.73	5.01	2.00	14.13	14.58	8.19	6.93	7.94	16.05	20.18	4.49	5.59	8.51	13.43	3.26	2.00	6.71	11.82	7.54	49.03	5.40	21.44	96.98	9.00	2.00	1.61	2.00	1.26	4.98	51.32	6.18	9.00	4.33	13.76	2.00	5.45	15.91	2.00	2.32	2.00	2.00
28.8		39.8	49.0	61.1	8.09	60.2	26.8	58.1	62.2	37.6	33.8	33.2	70.3	65.7	58.5	52.7	57.2	48.8	65.4	26.8	16.8	64.9	43.2	65.2	67.4	67.3	12.7	9:59	38.0	51.8	37.3	66.1	0.80	59.3	49.3	58.5	78.0	68.1	53.2	83.0	55.5	22.4	71.3	73.2	71.2	29.2	
	71.0	71.5	65.0	72.6		73.0	9.19	73.2		74.8	68.7	44.2	0.0		72.7	76.1	76.7	70.0	77.0	74.0	77.0		67.7		77.4	72.7	77.7	78.5	76.8	79.0	79.3	79.5		80.5	81.0	80.4		81.4	74.0	82.6		37.0				85.3	
O'T/	63.5	52.0	62.0	71.0	73.0	72.3	33.9	70.2	74.4	49.8	29.7	38.1	75.5	76.0	70.7	64.9	69.4	61.0	76.0	0.69	29.0	77.1	61.6	77.3	57.4	71.1	44.4	77.8	73.8	64.0	73.3	78.3	80.2	/L.5	61.5	70.7	81.4	80.2	68.0	76.6	83.5	34.0	83.4	85.3	85.3	6.09	83.0
-12.1/	-12.17	-12.17	-13	-9.95	-12.17	-12.17	-7.02	-12.17	-12.17	-12.17	-22.86	-4.87	-5.2	-10.3	-12.17	-12.17	-12.17	-12.17	-10.55	-12.17	-12.17	-12.17	-18.37	-12.17	10	-3.8	-31.7	-12.17	-35.8	-12.17	-36	-12.17	-12.17	-12.1/	-12.17	-12.17	-3.44	-12.17	-14.76	6.4	-28.04	-11.6	-12.17	-12.17	-14.16	-31.7	-40
		300	300	100	200	150	200	200	100	200	200	250	100	100	100	300	250	200	250		125	75	200	200	300	100	100		100	200	100	120		240			250		200	250	75	150	100	100	100	150	100
17	71	71.5	72.5	72.6	73	73.03	73.2	73.24	74.4	74.8	74.98	75.28	75.5	2/9	9/	76.05	76.7	76.81	77	77	77	77.1	77.11	77.35	77.39	7.77	7.77	78.5	79	79	79.25	79.5	80.2	80.5	2 2	81.2	81.4	82.42	83	83.5	83.5	8	84.44	85.34	85.34	85.34	85.34
			Unknown				Unknown				Groundwate	Groundwate	Unknown	r Stock Wateri				Unknown					Unknown	r Stock Wateri	Unknown	Unused					r Stock Wateri	ampling/moni				Unknown	Unknown	ampling/monit	Unknown	Groundwate	r Stock Wateri	Unknown		Unknown	r Stock Wateri	r Stock Wateri	r Stock Wateri
55/9529 Unused	5568248	5578022	5581374 Unknown	5573627	5571670 Irrigation	5567403	5565080 Unknown	5564341	5570421	5566631 Irrigation	5580465 Irrigation	5576143 Irrigation	5565106 Unknown	5564292 Domestic Water Stock Wateri	5576264 Stock Watering	5579094	5582842 Irrigation	5564670 Unknown	5575889 Irrigation	5565918 Irrigation	5579634	5571422	5564890 Unknown	5581321 Dome stic Water Stock Wateri	5584310 Irrigation	5571338 Unused	5582190	5574285	5573106 Irrigation	5564757	5559800 Domestic Water Stock Wateri	5577147 Groundwater sampling/moni	55/2623	5564942	5585936	5570750 Stock Watering Unknown	5586371 Exploratory Wel Unknown	5583147 Groundwater sampling/moni	5564222 Unknown	5577232 Irrigation	5560516 Dome stic Water Stock Wateri	5564625 Unknown	5569347	5565719 Unknown	5590741 Domestic Water Stock Wateri	5561067 Domestic Water Stock Wateri	5562259 Domestic Water Stock Wateri
1886310	1884207	1899303	1897687	1894116	1887096	1894468	1893998	1889357	1877006	1893704	1891572	1889640	1889372	1894456	1894072	1893210	1899529	1889264	1889691	1892498	1884881	1877806	1889635	1890478	1895977	1890930	1886185	1894088	1886646	1889388	1879720	1900477	18/8106	1889896	1898543	1890247	1896495	1895377	1889064	1894411	1878508	1889246	1886857	1881710	1903919	1878829	1878563
109/1	16913	16782	3204	10928	4913	16492	1836	16930	10972	4656	1826	1426	6713	4568	5671	16765	5167	6723	4685	5344	16454	10969	6721	4061	3870	3452	10890	16490	4302	15458	3658	16486	11001	16813	5723	4110	2160	16477	6715	2043	10983	6712	16094	1947	2913	3354	3757





Sa	55/110/ Groundwater sampling/moni	86.5	051	1- 1-	U. 42	7 6		5 6	١,٠	1 2	5.05	3.39
	a de la	87.12 07 E	120	17.17	03.0	8/.T	73.0	14.05		55.95 E3.4E	1.29 7.10	2.30
	T A S	87.6	300	-12.17	85.6	119.6	73.5	9.81		62.64	2.00	3.19
	Groundwate	88.2	200	-2	30.0	36.0	28.0	8.81	1 11	18.19	0.24	1.32
		89.19	100	-12.17	87.5	89.2	75.3	17.53	П	56.79	1.26	2.21
		89.68	150	-12.17	88.7	89.7	76.5	6.65	1	68.86	5.87	8.52
Nel	5580255 Exploratory Wel Unknown	6	300	6-	75.3	89.3	66.3	13.19	1	52.11	5.09	4.02
		90.11	100	-22.5	90.1		9.29	2.00	1	64.61	0.55	98.0
		90.9	300	-12.17	68.4	88.8	56.2	4.76	1	50.44	4.06	8.04
		91.2	300	-12.17	70.0	73.0	57.8	8.42	1	48.41	2.33	4.82
		92.28	75	-36.58	92.3		55.7	2.00	1	52.70	0.54	1.02
		93	150	-12.17	90.1	93.0	77.9	11.94	1	64.99	1.36	5.09
		94.5	200	-5.96	80.0	94.0	74.0	14.23	1	58.81	2.14	3.63
		94.5	100	-12.17	94.5		82.3	2.32	1	79.01	0.00	00:00
5590212 Stock Watering		95	100	-12.17	93.3	95.0	81.1	4.42	1	75.71	8.14	10.76
		96		-12.17	0.96		83.8	9.53	1	73.30	0.14	0.19
		96.9	150	-12.17	71.9	6.96	59.7	7.09	П	51.64	4.85	9.40
	Groundwate	97.5	300	-25.18	68.4	75.5	43.2	37.81	1	4.41	0.88	20.04 Very deep SWL
	Unknown	97.53	100	-10	97.5		87.5	2:00	1	84.53	0.57	0.67
	Unknown	86	250	9.8	92.0	98.0	101.8	19.79	1	81.01	0.92	1.13
	Unknown	86	250	-12.17	88.5	94.5	76.3	15.18	1	60.15	1.99	3.30
rsa	5581470 Groundwater sampling/monit	86	220	-2.65	98.0		95.3	15.19	1	79.16	2.13	2.69
		86	100	-12.17	72.7	98.0	9.09	3.12	1	56.45	0.00	00:00
		98.4	75	-19.51	29.9	98.4	10.4	2:00	1	7.39	0.54	7.24
		98.82	300	-12.17	86.6	98.8	74.4	19.42	1	54.01	0.52	96.0
		99.3	200	-17.96	80.4	99.3	62.4	3.04	П	58.39	0.41	0.70
	Unknown	102	300	6-	102.0	0.0	93.0	6.07	1	85.93	1.24	1.45
ater	5571461 Domestic Water Stock Wateri	102	150	4-	102.0		98.0	20.47	1	76.53	0.79	1.03
		102	150	-12.17	96.5	97.5	84.3	19.42	П	63.90	0.52	0.81
r sa	5564075 Groundwater sai Unknown	102.11	200	-43.73	102.1		58.4	4.30	П	53.08	0.23	0.43
		103.5	250	-12.17	103.5	109.5	91.3	8.49	1	81.84	1.09	1.34
rsa	5563969 Groundwater sa Exploratory \	105	150	-9.05	103.3	105.5	94.3	7.59	П	85.66	0.11	0.13
		108	125	-12.17	107.0	108.0	94.8	8.50	1	85.33	1.09	1.28
		109	92	-12.17	109.0		8.96	8.79	1	87.04	0.98	1.13
	Groundwate	110	250	-11.4	81.5	94.5	70.1	13.44	1	55.66	2.04	3.67
		110	250	-12.17	77.2	110.0	65.0	11.94	1	52.09	1.34	2.58
.sa	5576587 Groundwater sal Exploratory \	111	120	-1.7	108.0	111.0	106.3	09.9	1	98.70	06.0	0.91
		112	300	-3.6	112.0		108.4	27.60	1	79.80	0.92	1.15
		114.53	250	-12.17	114.5	119.8	102.4	17.84	1	83.52	1.23	1.47
		115.75	100	-12.17	193.2	211.2	181.0	3.90	П	176.09	8.44	4.79
		117	300	-12.17	111.0	117.0	98.8	5.38	1	92.45	4.16	4.50
5563893 Stock Watering		117.6	100	-12.17	96.8	117.6	84.7	2.59	1	81.09	0.31	0.38
)		118.5	150	-27	115.4	117.5	88.4	11.57	1	75.88	2.18	2.87
ersa	5571104 Groundwater sampling/monit	119	32	-12.17	117.0	119.0	104.8	52.49	1	51.34	0.81	1.59
	Unknown	121	250	-9.35	60.5	100.0	51.1	16.64	1	33.51	0.20	0.58
		121	100	-12.17	116.2	118.0	104.0	12.72	1	90.31	2.14	2.37
ter sa	5574904 Groundwater sampling/monit	122.8	100	-12.17	100.3	102.2	88.1	8.86	1	78.28	1.06	1.36
	Groundwate	123	200	-11.8	115.0	121.0	103.2	12.81	-	00.00	1	
)	121.0	7.001	12.21	7	59.39	0.75	1.27





4764	1896353	5579075	5579075 Irrigation		123	300	-12.17	96.0	120.0	83.8	8.28	-	/4.55	T.00	77.7	
5373	1900087	5579032	5579032 Irrigation		123	100	-12.17	67.2	68.5	55.0	2.68	1	51.35	0.73	1.41	
3702	1887644	5573215	5573215 Domestic Water Stock Wateri	Stock Wateri	123.5	200	-25.9	114.5	123.5	88.6	8.95	1	78.65	0.88	1.12	
4882	1891769	5572455	5572459 Irrigation		123.8	300	-10.1	109.5	123.8	99.4	37.27	1	61.13	0.87	1.43	
16549	1894879	5573143			126.17	300	-12.17	126.2		114.0	21.98	1	91.02	0.53	0.59	
4820	1888937	5570057	5570057 Irrigation		129	300	-12.17	123.0	129.0	110.8	34.52	1	75.31	0.74	0.98	
2041	1891082	5568784	5568784 Irrigation	Unknown	130	200	-5.8	97.1	110.1	91.3	32.74	1	57.55	0.45	0.79	
4761	1900489	5573732	5573732 Irrigation		130	300	-12.17	130.0		117.8	6.01	1	110.82	0.27	0.24	
4934	1887581	5570255	5570259 Irrigation		130	300	-12.17	130.0		117.8	22.40	1	94.43	0.74	0.79	
5445	1890301	5571106	5571106 Exploratory Well		130	100	-12.17	129.3	130.0	117.1	52.46	1	63.62	0.82	1.28	
5586	1884667	5567526	5567526 Irrigation		131	300	-12.17	119.0	131.0	106.8	3.61	1	102.22	0.59	0.58	
4593	1896642	5584217	5584217 Irrigation		134	300	17.5	111.6	129.0	129.1	7.00	1	121.10	2.34	1.93	
4673	1890920	5582975	5582979 Irrigation		136.5	200	-12.17	113.5	123.5	101.3	10.88	1	89.45	2.20	2.46	
4830	1894084	5576273	5576273 Irrigation		137	300	4.3	125.0	135.0	129.3	16.00	1	112.30	1.02	0.91	
5498	1893721	5575604	5575604 Irrigation		139.7	250	-12.17	132.7	139.7	120.5	18.27	1	101.26	1.04	1.03	
15938	1882853	5573882			141	150	-12.17	129.8	152.8	117.6	1.45	1	115.19	0.74	0.64	
3104	1896503	5576591	5576591 Irrigation	Groundwate	142	300	6.4	92.0	94.0	98.4	8.77	1	88.63	0.93	1.05	
15021	1894517	5581470	5581470 Groundwater sampling/moni	mpling/monit	148.5	110	-12.17	148.5		136.3	15.21	1	120.12	2.13	1.77	
4817	1888680	5580498	5580498 Domestic Water Stock Wateri	Stock Wateri	150	200	-12.17	150.0		137.8	8.72	1	128.11	1.59	1.24	
6720	1889625	5564890	5564890 Unknown	Unknown	150	200	-12.17	150.0	0.0	137.8	69.9	1	130.14	0.24	0.18	
15431	1896453	5587889	•		150	250	-12.17	134.0	150.0	121.8	5.61	1	115.22	7.45	6.47	
5427	1884623	5567526	5567526 Irrigation		152	100	-12.17	146.3	147.8	134.1	3.53	1	129.60	0.59	0.46	
1946	1881209	5566520	5566520 Unknown	Unknown	152.4	100	-12.17	152.4		140.2	2.00	1	137.23	0.57	0.41	
16563	1898976	5590999	•		153.5	200	-12.17	145.0	153.5	132.8	3.83	1	128.00	8.46	6.61	
16562	1898811	5590919	•		154.57	100	-12.17	153.4	154.6	141.2	3.90	1	136.33	8.44	6.19	
5497	1897384	5583557	5583557 Irrigation		157.1	300	-12.17	151.0	157.1	138.8	6.63	1	131.20	2.26	1.72	
5591	1891706	5581548	5581548 Irrigation		162.5	100	-12.17	133.0	135.0	120.8	13.35	1	106.48	2.14	2.01	
5392	1891736	5581595	5581595 Irrigation	Irrigation	163	300	-12.17	163.0		150.8	13.32	1	136.51	2.14	1.57	
4659	1900473	5573713	5573713 Exploratory Well		180	100	-4.5	180.0		175.5	90.9	1	168.44	0.27	0.16	
4631	1878233	5572504	5572504 Domestic Water Stock Water	Stock Wateri	206	100	-12.17	206.0		193.8	2.00	1	190.83	0.00	0.00	
2962	1893334	5564474			231	150	-12.17	231.0		218.8	8.69	1	209.14	0.11	0.05	
5997	1893389	5564459	•		238	300	-12.17	238.0		225.8	8.64	1	216.19	0.10	0.05	
4736	1897196	2576670	5576670 Irrigation		300	300	-12.17	300.0		287.8	7.58	1	279.25	0.92	0.33	
4270	1890450	5568344	5568344 Irrigation		307	200	-12.17	307.0		294.8	28.94	1	264.89	0.46	0.17	
4489	1888029	5572055	5572055 Irrigation		342	300	-13.4	48.4	72.0	35.0	20.97	1	13.03	0.83	6:39	
10998	1883026	5569136	5569136 Exploratory Well		1059	099	-12.17	1059.0		1046.8	2.55	1	1043.28	0.63	90.00	
10001	1004004	000000	1 - 14.		1177	0001										



TRANCHE 2 CONSENT - WELL INTERFERENCE ASSESSMENT

			Sample -									Seasonal	Pump	Remainin		as %	
IZTM_	NZTM_		Grid	Use1	Use2	Danash	Diamate	L 614/1	T	Diam'Cau'		variation	length	g Head	Predicte	remainin	
sting	Northing	Well No.	value	Use1	Use2	Depth	Diameter	h_SWL	TopScreer	BtmScree	head (m)	(m)	(m)	(m)	d DD (m)	g head	ts
1888814	5561887	3827	-0.13009	Domestic	Stock Wat	0	200	0.00			0.00	1.62		0 -1.62	0.13	-8%	Spring
1899720	00.202.	10920		Irrigation		0		0.00			0.00	1.62		0 -1.62			Spring
1897499		10922		Irrigation		0		0.00			0.00	1.69		0 -1.69			Spring
1896406 1896166		16356 16365	-0.14228 -0.13253			0.6	68 68	0.00	0.60 1.00	4.60 5.00		1.82		0 -1.22 0 -0.82			<2m deep
1897932	5570478 5577165	15031		Unknown		1.4	100	0.00	1.00	5.00	1.40	1.82		0 -0.82			<2m deep
1896464		16358	-0.31009	Ulkilowii		1.5	68	0.00	1.50	6.00		1.70		0 -0.30			<2m deep
1896197	5570879	16357	-0.15531			1.55	68	0.00	1.55	6.05	1.55	1.83		0 -0.28			<2m deep
1900930	5577261	15667	-0.13265			2		0.00			2.00	1.67		0 0.33	0.13	40%	<2m deep
1900108		2030		Domestic		3	1000	-1.92			1.08	1.59		0 -0.51			Potentially inefficient
1898987	5571262	10962		Domestic	Stock Wat	3.2	100	-2.07	8.00	10.00		1.76		0 -0.63			Potentially inefficient
1896881 1897096		15014 15025	-0.33293 -0.37452			3.3	150 1000	-1.92 -1.92			1.38	1.76		0 -0.38 0 -0.39			Unused Unused
1884001		2747		Irrigation	Unknown	3.6	750	-1.60	2.60	3.60		2.30		0 -0.39			Potentially inefficient
1896470		15028		Groundwa		3.9	100	-1.92	2.00	3.00	1.98	1.84		0 0.14			Unused/GW monitoring
1877285	5563499	15866	-0.08406			4	1000	-1.92			2.08	1.95		0 0.13	0.08	66%	
1901041		2902		Domestic		4.11	100	-1.45	2.11	4.11		1.80		0.86		42%	
1896292	00.02.0	15023		Groundwa		4.5	100	-1.92			2.58	1.85		0 0.73			Unused/GW monitoring
1896338		15024	-0.53588			4.6	100	-1.92	2.4-		2.68	1.85		0 0.83			Unused/GW monitoring
1898841 1893617		5137 4227		Groundwa		4.7 4.89	100 100	-1.92 -1.60	2.47 1.80	4.00		1.61 2.16		0 1.17 0 1.13			GW Monitoring GW Monitoring
1893617		4227 5532		Unknown	icer sampli	4.89	1200	-1.60 -1.92	1.80	4.40	3.29	1.96		0 1.13		25% 25%	OAA MOUNTONING
1904521		1357		Domestic	Unknown	5.18	100	-2.13	0.00	0.00		1.94		0 1.11		65%	
1897308		16346	-0.51372			6	150	-1.92	550	2.30	4.08	1.83		0 2.25		23%	
1896008	5578658	4620		Groundwa		6.8	100	-1.92			4.88	1.84		0 3.04	0.75		GW Monitoring
1899820		3590		Domestic		7.3	100	-5.50			1.80	1.55		0 0.25		52%	
1900138		3690		Domestic		7.5	100	-4.30		40	3.20	1.75		0 1.45		32%	
1880298 1889093		3175 4477		Stock Wat Groundwa		10.05	150	-3.2	6.2	10.05		2.46		0 -1.66 0 -4.44			very shallow top screen/potentially inefficient
1889093		3494		Domestic		10.2 10.5	100 150	-6.3 -3.2	6.2 5.35	10.20		4.34 2.37		0 -4.44 0 -0.22			GW Sampling very shallow top screen/potentially inefficient
1893666		4228		Groundwa		11.18	100	-10.35	8.4	11.20		7.27		0 -9.22			GW Sampling/potentially inefficient/very deep h
1889144		5356		Groundwa		11.8	50	-3.9	5.8	11.80		4.34		0 -2.44			Potenitally inefficient
1897594	5580050	3076	-0.5247	Unknown		11.99	150	-7.63			4.36	6.13		0 -1.77		-30%	GW Sampling/deep hSWL
1896860		16345	-0.38788			12	150	-3.9	11	12.00		7.38		0 -0.28			Potenitally inefficient
1898532		2979		Unknown		12.4	150	-9.35	11	12.00		5.19		0 -3.54			GW Sampling
1889128		5502		Explorato		12.4	50 150	-3.9	6.5	12.40		4.30 2.45		0 -1.70			Potenitally inefficient
1899118		3421 1414		Domestic Domestic		12.6 12.8	100	-5.95 -9.14	8.27 11.73	10.80		2.45 5.85		0 -0.13 0 -3.26			Potenitally inefficient Potenitally inefficient
1898417		10968	-0.3258	_omesut		13.1	150	-9.14	10.36	11.58		6.76		0 -3.20		27%	- I I I I I I I I I I I I I I I I I I I
1882212		2809		Domestic	Stock Wat	14.2599	150	-9	10.59	14.25		2.96		0 -1.37	0.02		Potenitally inefficient/deep hSWL
1893683	5587022	5662	-0.27821	Explorato	y Well	15.5	50	-3.9	11.5	15.50		7.30		0.30		91%	
1893715		2773		Domestic	Stock Wat	18	150	-7.1	12	18.00		4.65		0 0.25		84%	
1900702		5211		Irrigation		18.5	300	-3.9	9	18.50		3.63		0 1.47	0.39	27%	
1884667 1879709		16097 10931	-0.04906 -0.09053			18.5 20.1	100 200	-3.9 -2.13	6.5 10.12	18.50 20.12		2.77		0 -0.17 1 -4.48			Potenitally inefficient/shallow screen Potentially inefficient
1879938		1944		Domestic	Grounder	20.1	200	-2.13	7.06	17.06		11.47		1 -4.48			Potentially inefficient Potentially inefficient
1899019		10995		Agricultur		21.34	100	-2.13	16.10	18.90		6.47		1 -1.87			Potentially inefficient/ very deep hSWL
1893949		10948	-0.60286	3		22.6	100	-17.06	20.09	21.00		10.39		1 -8.36	0.60		Potentially inefficient/very deep hSWL
1879939	5566652	4780		Stock Wat	ering	25	150	-2.4	3.65	15.50		11.34		1 -11.09			Very shallow top screen?
1900145		3664		Domestic	Stock Wat	25.6	100	-8.02		25.60		4.88		1 0.13		82%	
1884307		11005	-0.04301			25.6	100	-11.28	18.26	19.78		11.02		1 -5.04			Potentially inefficient/deep hSWL
1898782		3843		Irrigation		28	300	-4.17	10.50	16.50		5.09		1 0.24		35%	Detection in officient (deep b514)
1894170 1893814		1396 10978	-0.25011	Domestic	Agricultur	28.35 28.35	400 100	-19.51 -9.44	26.67 21.88	28.35		10.91		1 -4.75 1 0.86	0.25	-5% 67%	Potentially inefficient/deep hSWL
1893814		2926		Domestic	Stock Wat	30.02	100	-14.15		28.03		7.42		1 -21.04			very shallow top screen level?
1881351		2729			Stock Wat		100	-3.26	9.45	10.82		7.42		1 -21.04			Shallow top screen, potentially inefficient
1889315		10895	-0.06095			30.9	75	-14.6	16.50	30.90		7.42		1 -6.52			Deep SWL and shallow topscreen
1892475	5575978	2779	-0.16728	Domestic	Unknown	31.08		-1.21	5.18	25.90	3.97	7.42		1 -4.45	0.17	-4%	very shallow top screen level?
1898902		2543		Explorato	Unknown	33.1	300	-1.36	8.00	10.20		7.42		1 -1.78			very shallow top screen level?
1880706		10894	-0.05748			34.7	100	-6.37	10.36	11.35		7.42		1 -4.43			Shallow top screen, potentially inefficient
1892212		11004	-0.31256 -0.06068			36.6	100 100	-28.65	27.80	28.71		7.42		1 -9.27 1 -8.05			Very deep SWL
1901321 1889415		10899 6714		Unknown	Unknow	37.5 38	200	-1 -6.37	1.37 0.00	3.69 0.00		7.42 7.42		1 -8.05 1 -14.79			very shallow top screen level?
1895638		2002		Unknown		44.2	200	-0.82	7.60	11.60		16.66		1 -14.79			very shallow top screen level? very shallow top screen
1886713		2931		Stock Wat		44.2	150	-0.62		40.84		16.66		1 -6.50			shallow top screen
1892348		1439		Domestic		44.5	100	-38.1	37.04	38.56		16.66		1 -18.72			Very deep SWL
1881261	5578098	2579		Stock Wat		46.63	100	-22	37.19	46.63	15.19	16.66		1 -2.47			deep SWL
1903920	5590842	10900	-0.36926			46.63	100	-9.01	4.27	5.79		16.66		1 -22.40			Very shallow top screen
1895744		3490		Domestic		47.01	100	-7.89	13.50	16.50		16.66		1 -12.05		-1%	Shallow top screen
1891874		2730		Domestic		48.15	150	-23.6	43.59	48.16		16.66		1 2.33			deep SWL
1884109	5573925	2903	-0.04746	Stock Wat	Unknown	49.68	100 100	-1.6 -23.77	8.23 21.88	10.23 23.07		16.66 16.66		1 -11.03 1 -19.55			Shallow top screen

Summary of All Flagged Wells >50m Deep

IZTM Easting	NZTM Northin	Well No.	Use1	Use2	Depth	Diameter	h SWL(m	TopScreen1	BtmScreen1	Available Head (m)	Seasonal Variation (m)	Pump Length (m)	Remaining Head (m)	Predicted DD (m)	as % Remaining Head	Comments			
1896078			Domestic Water			100	-10.5				6.23		28.27						
1899604	5569774	5377	Stock Watering		50	100	-12.17	5.8	7.1	-6.4	9.38		-16.77	0.27	-1.61	very shallows	creen - n	ot a deep w	ell
1884709	5577527	10945			51.8	100	-17.86	24.6	25.5	6.7	2.30	:	3.41	0.99	29.09				
1891952	5576325	2775	Domestic Water	Unknown	51.81		-12.17	4.0	50.6	-8.2	16.98		-26.19	1.27	-4.86	very shallows	creen		
1896858	5589264	3882	Irrigation		52.5	250	-8.3	44.5	51.5	36.2	4.97		1 30.23	8.04	26.59				
1899607	5569766	5435	Stock Watering		54	200	-12.17	23.6	54.0	11.5	9.37		1.08	0.27	25.00				
1896975	5589136	16483	_		54.28	100	-12.17	52.3	54.3	40.2	4.97		34.19	7.99	23.36				
1878573	5572593	3859	LU960169B		54.86	100	-26.6	25.7	54.9	-0.9	2.00		-3.94	0.66	-16.77	"inefficient" v	ery low S	WL	
1893979	5589928	16033			56.36	150	-12.17	53.5	56.4	41.3	5.53		1 34.76	7.56	21.75				
1878208	5570422	10956			58.8	100	-14.33	17.5	19.2	3.1	2.00		0.13	0.63	482.65	"inefficient" v	ery shallo	w top scre	an
1892806	5568279	1697	Irrigation	Unknown	58.83	250	-6.1	17.7	20.4	11.6	24.59		-14.01	0.23	-1.61	"inefficient" v	ery shallo	w top scre	an
1895414	5589438	10927	Irrigation		59.3	150	-12.17	59.3		47.1	5.40		40.73	8.20	20.14				
1896872	5589031	16739			59.5	100	-12.17	57.6	59.5	45.4	5.04		39.39	8.01	20.33				
1872714	5567306	4188	Domestic Water	Stock Water	60.4	100	-12.17	10.8	60.4	-1.3	2.00		-4.34	0.56	-12.79	"inefficient" v	ery shallo	w top scre	an
1891467	5582390	4706	Irrigation		62	100	-31.2	53.0	61.0	21.8	12.07		8.73	2.16	24.78	Very deep SW	/L		
1888935	5570055	1429	Irrigation	Unknown	65.86	250	-3.65	42.0	44.1	38.4	34.50		2.89	0.74	25.60		Quite sha	llow screer	1
1895692	5588811	5515	Irrigation		66	300	-12.17	54.0	66.0	41.8	5.56		1 35.27	8.97	25.42				
1894644	5592606	3073	Domestic Water	Stock Water	69.19	100	-30.3	55.8	68.0	25.5	4.44		20.04	7.97	39.79	Very deep SW	/L		
1896114	5591139	3483	Unknown	Unknown	71	150	-22	58.5	70.5	36.5	4.62		30.88	8.00	25.91	Very deep SW	/L		
1886185	5582190	10890			77.7	100	-31.7	44.4	77.7	12.7	5.40		6.30	1.49	23.67	Very deep SW	/L		
1890300	5572513	3774	Irrigation	Groundwate	97.5	300	-25.18	68.4	75.5	43.2	37.81		4.41	0.88	20.04	Very deep SW	/L		



APPENDIX 4

REPORT LIMITATIONS



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- IV. In addition, it is recognized that the passage of time affects the information and assessment provided in this Document. Lattey's opinions are based upon information that existed at the time of the production of the Document. It is understood that the services provided allowed Lattey to form no more than an opinion of the actual conditions of the site at the time the site was visited and cannot be used to assess the effect of any subsequent changes in the quality of the site, or its surroundings, or any laws or regulations.
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- VI. Where data supplied by the Client or other external sources, including previous site investigation data, have been used, it has been assumed that the information is correct unless otherwise stated. No responsibility is accepted by Lattey for incomplete or inaccurate data supplied by others.
- VII. The Client acknowledges that Lattey may have retained sub-consultants affiliated with Lattey to provide Services for the benefit of Lattey. Lattey will be fully responsible to the Client for the Services and work done by all of its sub-consultants and subcontractors. The Client agrees that it will only assert claims against and seek to recover losses, damages or other liabilities from Lattey and not Lattey's affiliated companies, and their employees, officers and directors.
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APPENDIX E – Shallow Groundwater Bores Assessment

Bay Geological Services Ltd

06 August, 2021 ref: BGS201_08

Consents Planner Hawkes Bay Regional Council Private Bag 6006 Napier 4110

Dear Sir/Madam,

RUATANIWHA BASIN TRANCHE 2 APPLICATION INVESTIGATION OF SHALLOW GROUNDWATER BORES IDENTIFIED IN ASSESSMENT OF WELL INTERFERENCE EFFECTS

As part of the Ruataniwha Basin Tranche 2 Application for a groundwater allocation, a Well Interference Assessment was completed in September 2020 by Lattey Group (Lattey). The potential effect of the proposed take was numerically modelled by Aqualinc (Weir, 2020) which resulted in a series of scenarios. The Lattey (2020) investigation reviewed groundwater wells across the basin, of which there are 703 as recorded by Hawkes Bay Regional Council (HBRC). The wells were categorised based on their recorded total well depths, with attention paid to wells within the 0 to 50 m depth below ground level (bgl) range which total 421. Of these, only fourteen wells were identified as requiring further investigation to ascertain if the wells may be adversely impacted by the proposed abstraction, or if the wells were no longer used, were inefficient or used for monitoring purposes.

1. INTRODUCTION

To investigate the potential effects of well interference on the wells within 50 m depth, the elevation of the top screen and static water level (SWL) were taken into consideration, along with adopted pump elevations to determine the available head of water. The predicted seasonal variation was also included in the assessment, determined from a review of state of the environment (SOE) wells' water levels recorded by HBRC over the past five years.

A contour map from numerical modelling the most likely effects scenario as a result of the proposed Tranche 2 groundwater abstraction was provided (refer Figure 1), which was overlain on the HBRC wells map. From this, predicted well interference values could be read off the map and applied to individual well locations across the basin.

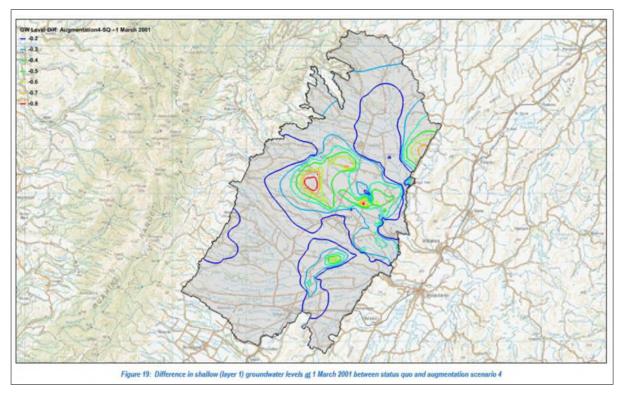


Figure 1. Difference in Shallow (Layer 1) Groundwater Level of the Aqualinc Scenario 4 (Weir, 2020)

An unwritten rule estimates that if the predicted drawdown (as a result of the pumped well) within a neighbouring bore is greater than 20% of the remaining head in the bore, then there is a possibility that the neighbouring bore may be adversely impacted by the additional drawdown.

The investigation revealed a number of wells that could potentially be affected by the proposed Tranche 2 abstraction. The bores identified as being installed for environmental monitoring along with those deemed inefficient, where the depth of the pump is not sufficient to mitigate being adversely affected by well interference from pumping surrounding wells, were excluded from the assessment.

As a result of this assessment, fourteen wells were flagged as potentially being affected by the proposed abstraction of the groundwater allocation as modelled (Lattey, 2020), and further investigation was suggested to ascertain status of the bores. The following Report provides additional information on the shallow wells identified within the Well Interference Report (Lattey, 2020).

2. RESULTS OF WELL INTERFERENCE ASSESSMENT

The Lattey (2020) report managed the large number of wells identified within the basin by dividing them into groups based on depth. The well groups and number of wells associated with each group is shown in Table 1.

From the 110 bores drilled to within 9.99 m depth, initially 28 wells were identified as potentially being adversely affected (i.e., where the predicted drawdown as a percentage of remaining head is greater than 20%). Furthermore, the bore review identified 14 bores with negative values which could be considered as "inefficient", 13 of which are wells that are 3.6 m deep or less, and one from a well with a recorded SWL that is deeper than the recorded well depths, so could be an error in the database.

The other 14 wells identified have values greater than 20%, with 7 of the wells which remain flagged within the 0 to 9.99 m depth range as circled in Table 1. A further 4 wells are included in the 10 to 19.99 m depth range and 3 in the 20 to 29.99 m depth. These bores have a greater than 20% value and their use is not specified or they are domestic or stockwater bores, where an adverse impact on the bores cannot be ruled out. Therefore, a field inspection and further investigation was required to confirm that the bores are still operational and being used, along with pump configuration.

Table 1. Summary of Flagged Wells (Lattey, 2020)

Well Group	Total No. Wells	Initially Flagged	Reduced	Comments
()-9.99m	110	28	1	"Inefficient" and monitor wells removed
10-19.99m	75	17	4	"Inefficient" and monitor wells removed
20-29.99m	/5	10	3	"Inefficient" and monitor wells removed
30-39.99mi	82	9	0	"inefficient" wells removed
40-49.99m	79	9	0	"inetficient" wells removed +1 deep SWI
+50m	236		-	Will have sile specific review when T2 wells drilled
Unknown	46			It not "inetticient" and reach +30m unlikely to be attected
Totals	703	73	14	, , , , , , , , , , , , , , , , , , , ,

The locations of flagged wells within the basin area are illustrated on the Aqualinc contour map presented as Figure 2 (Lattey, 2020).

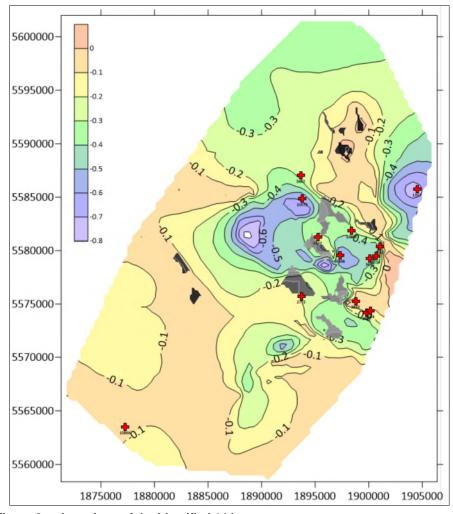


Figure 2. Locations of the Identified 14 bores.

The predicted well interference effect as a result of the proposed Tranche 2 abstraction is detailed in Table 2 (Lattey, 2020). The table provides detail on the bores, available head, pump length and seasonal variation which was determined from a review of state of the environment (SOE) wells, along with the numerical modelling predicted well interference at each bore location and an approximated estimate of remaining head.

Table 2. Predicted Well Interference within Identified Bores (Lattey, 2020)

			Predicted \	Nell Interfere	ence within Fla	gged Bores (I	_attey, 2020)	
Well No. (diam.)	Depth (m)	SWL (m) datum	Screen Top (m)	Available head (m)	Seasonal variation (m)	Pump length (m)	Remaining head (m)	Predicted DD (m) (as % of Remaining Head)
1357 (100mm)	5.18	-2.13	-	3.05	1.94	0	1.11	0.72 (65%)
2773 (150mm)	18.00	-7.10	12.00	4.90	4.65	0	0.25	0.21 (84%)
2902 (100mm)	4.11	-1.45	2.11	2.66	1.80	0	0.86	0.36 (42%)
3590 (100mm)	7.30	-5.50		1.80	1.55	0	0.25	0.13 (52%)
3664 (100mm)	25.60	-8.02	14.03	6.01	4.88	1	0.13	0.11 (82%)
3690 (100mm)	7.50	-4.30	-	3.20	1.75	0	1.45	0.47 (32%)
3843 (300mm)	28.00	-4.17	10.50	6.33	5.09	1	0.24	0.08 (33%)
5211 (300mm)	18.50	-3.90	9.00	5.10	3.63	0	1.47	0.39 (27%)
5532 (1200mm)	5.00	-1.92	-	3.08	1.96	0	1.12	0.28 (<mark>25%</mark>)
5662 (50mm)	15.50	-3.90	11.50	7.60	7.30	0	0.3	0.28 (93%)
10968 (150mm)	13.10	-2.37	10.36	7.99	6.76	0	1.23	0.33 (27%)
10978 (100mm)	28.35	-9.44	21.88	12.44	10.58	1	0.86	0.57 (<mark>66%</mark>)
15866 (1000mm)	4.00	-1.92	-	2.08	1.95	0	0.13	0.08 (62%)
16346 (150mm)	6.00	-1.92	-	4.08	1.83	0	2.25	0.51 (<mark>23%</mark>)

3. FIELD INSPECTION OF IDENTIFIED WELLS

The shallow wells flagged in the Lattey report were inspected in the field or landowners were contacted where available in order to gather additional information on well status. One site, being Well No. 5211 was not inspected as no contact with the landowner was able to be made.

Details of the identified bores are listed in Table 2, which includes grid reference, well and screen depth, aquifer, SWL and Use as recorded on the HBRC bore records. The landowners typically provided sufficient detail on pump and bore status. The bores that landowners advised as abandoned, not in existence or used soley for groundwater monitoring purposes are shaded grey.

Table 2. Details of Flagged Wells (source HBRC)

		Deta	ils of Flagged W	/ell within Ruataniw	ha Basin		
Well No. (diam.) Owner	Grid ref (NZTM)	Well Depth (m)	Screen (m) bgl	Aquifer	SWL (m) datum	USE (1,2	Pump
1357 (100mm)	1904521 5585738	5.18	-	bl/gr limestone	-2.13	domestic and stockwater supply	surface
2773 (150mm)	1893715 5575728	18.00	12.00-18.00	brown gravel	-7.10	domestic potable supply	surface
2902 (100mm)	1901041 5580364	4.11	2.11-4.11	blue/brown gravel	-1.45	domestic water supply	surface
3590 (100mm)	1899820 5574178	7.30	-	? blue limestone	-5.50	domestic and stockwater supply	surface
3664 (100mm)	1900145 5574351	25.60	14.03-25.60	brown, white limestone brown gravel	-8.02	domestic water supply	surface
3690 (100mm)	1900138 5579254	7.50	-	? medium red gravel	-4.30	domestic water supply	surface
3843 (300mm)	1898782 5575258	28.00	10.50-16.50	brown gravel	-4.17	pastoral farming irrigation	not used
5211 (300mm)	1900702 5579478	18.50	9.00-18.50	brown gravel	-3.90	cropping irrigation	unknown
5532 (1200mm)	1895261 5581232	5.00	-	grey gravel	-1.92	misc. industry wastewater: washwater	surface
5662 (50mm)	1893683 5587022	15.50	11.50-15.50	brown gravel	-3.90	expl drilling environmental	N/A
10968 (150mm)	1898418 5581833	13.10	10.36-11.58	coarse brown gravel	-2.37	stockwater supply	abandoned
10978 (100mm)	1893814 5584834	28.35	21.88-22.82	coarse gravel	-9.44	domestic supply	submersible
15866 (1000mm)	1877285 5563499	4.00	-	-	-1.92	domestic and stockwater supply	surface
16346 (150mm)	1895370 5574535	6.00	-	gravel	-1.92	expl. drilling environmental	N/A

The sites are described in detail as follows:

3.1 Well No. 1357, 1579 Argyll Road, Argyll

The shallow 100 mm diam. well was not closely inspected due to being unable to contact the landowner for access. The landowner later confirmed via telephone that the bore is 5.18 m deep and screened across a limestone aquifer (refer Figure 3). A surface pump is installed due to the shallow SWL and depth of the bore, which provides domestic and stock water.



Figure 3. Well No. 1357

3.2 Well No. 2773, 1396 Ongaonga Road, Ongaonga

The moderately shallow 150 mm diam. bore is screened across brown gravel from 12 to 18 m depth with a surface pump installed (refer Figure 4). The landowner advised that the well never runs dry, but sucks up gravels when pumped during the summer while providing a domestic water supply.



Figure 4. Well No. 2773

3.3 Well No. 2902, 103 The Brow, Waipawa

The shallow 100 mm diam. bore is drilled beside a spring-fed waterway that never dries up (refer Figure 5). The bore is screened from 2.11 to 4.11 m bgl with a surface pump installed (refer Figure 6). The landowner advised that the water levels remain stable within the bore, and provides a domestic water supply.



Figure 5. Well No. 2902 and nearby spring-fed stream



Figure 6. Well No. 2902 pump

3.4 Well No. 3590, 16 Swamp Road, Ongaonga

No screen details exist for the shallow 100 mm diam. bore which is drilled to 7.3 m depth. A surface pump is installed in the well (refer Figure 7) that provides a domestic and stockwater supply for several residences.



Figure 7. Well No. 3590

3.5 Well No. 3664, Stockade Road, Ongaonga

A surface pump is installed in the moderately shallow well that is screened from 14.03 to 25.60 m bgl across brown and blue gravels and limestone beds (refer Figure 8). The bore provides a domestic water supply.



Figure 8. Well No. 3664

3.6 Well No. 3690, 104 The Brow, Waipawa

A surface pump is used for the shallow 100 mm diam. bore which is drilled beside a spring-fed waterway that never dries up and retains a stable level (refer Figure 9). No screen details exist for the bore which is drilled to 7.5 m depth bgl. The water provides for a domestic supply.



Figure 9. Well No. 3690

3.7 Well No. 3843, 140 Swamp Road, Ongaonga

The moderately shallow, 300 mm diam. well was installed by previous landowners and has not been used by the current landowners. HBRC records show the bore is careened across brown gravels from 10.5 to 16.5 m depth bgl.

3.8 Well No. 5211, 11 The Brow, Waipawa

The HBRC records show that the moderately shallow, 300 mm diam. well is screened from 9 to 18.5 m depth bgl, and it is therefore likely that a surface pump is installed to abstract groundwater. No contact was made with the landowner; however, records indicate that the well is used for crop irrigation.

3.9 Well No. 5532, Hutts Aggregate Quarry, Waipawa River

The shallow 1200 mm diam. sump is installed to 5 m depth bgl. The bore owner confirmed that the well is installed beside the Waipawa River. The well is not currently used, but there is the intention to use the well in the near future with a surface pump, as it is critical for wastewater in aggregate production.

3.10 Well No. 5662, Holden Road, Tikokino

HBRC records show that the moderately shallow environmental bore is screened from 11.5 to 15.5 m bgl across brown gravel. Due to the bore being a monitoring well, there would likely be sufficient groundwater from which to complete sampling and it would not likely be adversely affected by the proposed take. However, several discussions were had with the bore owner, Central Hawkes Bay District Council (CHBDC) who are adamant the well does not exist. A search for the well in the field also did not find evidence of bore casing.

3.11 Well No. 10968, Tikokino Road, Tikokino

The landowner advised that the relatively shallow 150 mm diam. bore was abandoned several years ago due to stuck equipment down-hole.

3.12 Well No. 10978, 191 Glenalvon Road, Tikokino

HBRC records show that the domestic and stockwater supply bore is screened from 21.88 to 22.82 m depth bgl across brown clay which is logged from 21 to 23 m depth (refer Figure 10). The screen is likely to be placed across coarse gravel logged from 23 to 26 m depth. A new submersible pump has recently been installed in the well which was drilled in 1973 and supplies domestic water, while the landowner advised that the well water level occasionally falls below the pump level during the summer months due to greater demand.



Figure 10. Well 10978

3.13 Well No. 15866, Takapau

The 1000 mm diam. spring-fed, domestic supply well is recorded as being 4 m deep. The landowner indicates the source does not dry up and levels remain stable. A surface pump is installed at the bore.

3.14 Well No. 16346, Tikokino Road, Tikokino

The shallow 150 mm diam. bore is positioned at the hydraulically downgradient end of a cattle feedlot and is used as an environmental monitoring bore. Due to it being a monitoring well, there would likely be sufficient groundwater with which to complete sampling and it would not likely be adversely affected by the proposed take.

4. PREDICTED EFFECTS ON SHALLOW BORES

The updated Table 3 precludes the abandoned or those denoted as environmental monitoring bores, which leaves eleven wells to discuss, along with an estimate of how this may affect the bores.

Table 3. Remaining Bores that have the potential to be adversely affected.

		Details of	Flagged Well v	within Ruatan	niwha Basin	
Well No. (diam.) Owner	Screen (m) bgl (aquifer)	SWL (m) datum	USE (1,2)	Pump	Predicted DD (m) (as % of Remaining Head)	Potential for adverse effect (Y/N)
1357 (100mm)	(well is 5.18 m deep) (limestone)	-2.00	domestic & stockwater supply	surface	0.72 (65%)	Yes, due to large potential drawdown
2773 (150mm)	12.00-18.00 (brown gravel)	-7.10	domestic water supply	surface	0.21 (84%)	Yes, due to large potential drawdown, and that well currently pumps gravels during summer months
2902 (100mm)	2.11-4.11 (bl/brn gravel)	-1.45	domestic water supply	surface	0.36 (42%)	Yes, due to large potential effect on remaining head
3590 (100mm)	(well is 7.30 m deep) (? limestone)	-5.50	domestic & stockwater supply	surface	0.13 (52%)	Yes, due to large potential drawdown and the bore can run dry in the summer months.
3664 (100mm)	14.03-25.60 (brown, white limestone brown gravel)	-8.02	domestic water supply	surface	0.11 (82%)	Yes, due to large potential effect on remaining head
3690 (100mm)	(well is 7.50 m deep) (? red gravel)	-4.30	domestic water supply	surface	0.47 (32%)	Yes, due to large potential drawdown, even though the bore taps a spring-fed stream that has never run dry.
3843 (300mm)	10.50-16.50 (brn gravel)	-4.17	pastoral farming irrigation	not used	0.08 (33%)	Yes, due to large potential effect on remaining head (noting that the well is not currently used).
5211 (300mm)	9.00-18.50 (brn gravel)	-3.90	cropping irrigation	likely surface	0.39 (27%)	Yes, due to large potential drawdown and the potential effect on remaining head
5532 (1200mm)	(well is 5 m deep) (grey gravel)	-1.92	misc. industry wastewater: washwater	surface	0.28 (25%)	Possibly No, due to the water take abstracted adjacent to and within the bed of the Waipawa River.
10978 (100mm)	21.88-22.82 (gravel)	-9.44	domestic & stockwater supply	submersible	0.57 (66%)	Yes, due to large potential drawdown and potential effect on remaining head which may drop water level below pump.
15866 (1000mm)	(well is 4 m deep)-	-1.92	domestic water supply	surface	0.08 (62%)	Yes, due to large potential drawdown and the potential effect on remaining head

The lowering of groundwater levels as determined from the Aqualinc report (Weir, 2020) applied to the bores, along with knowledge of the bore status and pump configuration, infers that ten of the eleven wells would possibly experience adverse effects as a result of the proposed Tranche 2 take. However, the bore data for six of these wells indicate that well or screen depths are shallow (< 7.50 m depth) and do not have an adequate water column. It is therefore likely that these wells struggle already during periods of low groundwater levels.

The Well No. 5532 is described by the owner as a bore installed beside the river bed, and it is expected that the well is directly recharged by the Waipawa River. This is a local control on groundwater levels that the numerical model is unable to accommodate, and therefore the model's predicted lowering of 0.28 m is likely overstated.

5. SUMMARY

In September 2020, a Well Interference Assessment was completed by Lattey as part of the Ruataniwha Basin Tranche 2 Application for a groundwater allocation. The Lattey (2020) investigation reviewed groundwater wells across the basin, along with the potential effect of the proposed Tranche 2 take as numerically modelled by Aqualinc (Weir, 2020). A range of scenarios was provided by Aqualinc (Weir, 2020), and a contour map from numerical modelling of the most likely effects as a result of the proposed Tranche 2 groundwater abstraction was provided, from which predicted well interference values could be read off and applied to individual well locations.

The well review categorised bores based on their recorded total well depths, particularly the 421 wells within 50 m depth range. Of these, 14 bores were identified as requiring further investigation to ascertain if they may be adversely impacted by the proposed abstraction, or if the wells were no longer used, were inefficient or used for monitoring purposes.

A rule of thumb suggests that if predicted well interference (as a result of the pumped well) within a neighbouring bore is greater than 20% of the remaining head in the bore, then there is a possibility that the neighbouring bore may be adversely impacted by the additional drawdown. The 14 wells identified have values greater than 20% and their use was not specified or they are domestic or stockwater bores, where an adverse impact on the bores could not be ruled out. Therefore, a field inspection and further investigation of the bores was required to confirm that their status.

The shallow wells identified were inspected in the field or landowners were contacted where available in order to gather additional information on well status, apart from one site at Well Nos. 5211 where no contact was able to be made with the landowner.

The investigation determined that 3 of the 14 bores were either abandoned, not in existence or used as monitoring wells. Of the remaining 11 bores, the majority are shallow and provide domestic and stockwater, operated by surface pumps apart from the deepest Well No. 10978 which has a submersible pump.

Applying bore and aquifer data including screen elevation, SWL, seasonal variation, pump depth, and predicted well interference, only one of the 11 bores is thought unlikely to be adversely affected by the proposed Tranche 2 take. This is Well No. 5532, installed as a bore adjacent to the Waipawa River which is likely to be directly recharged by the river. This is a local control on groundwater levels that the numerical model is unable to accommodate, and therefore the model's predicted lowering of 0.28 m is likely overstated and therefore not regarded as an adverse effect on the bore.

The numerical modelling data suggests that the other ten bores may experience well interference of 25% to 84% of the Remaining Head of water, which may adversely affect security of supply to the landowners/occupiers. However, six of the ten bores are constructed relatively shallow, to depths <7.50 m, which results in a small available water column that is more sensitive to well interference.

6. REFERENCES

Hawkes Bay Regional Council website (www.hbrc.govt.nz).

Lattey Group, 2020: Ruataniwha Basin Tranche 2 Irrigation Water Permit Consent Application - Assessment of Well Interference Effects. Project No. J19220-Rep-01 T2-Wi. Lattey Group, Hastings, New Zealand.

Weir, J., 2020: Ruataniwha Basin: Tranche 2 Groundwater Modelling. Client Report Project No. WL18045 05/11/2020. Aqualinc Research Limited, Christchurch, New Zealand.

Report Limitations

This letter report is written based on conditions as they existed at the time of the study, and there is no interpretation made on potential changes that may occur across the sites or the verbal information provided by the landowners and occupiers. Subsurface conditions may exist across the project area that are not able to be detected or revealed by the investigation within the scope of the study, along with any incorrect information provided by the landowners/occupiers and are therefore not taken into account.

APPENDICES

APPENDIX A

Topo map illustrating SOE significant wetland areas

(data sourced from HBRC and map generated by Aqualinc)

A1.: Central & Southern Hawkes Bay area topo map with contours overlain demonstrating the Aqualinc (2020) difference in shallow (Layer 1) groundwater levels at March 2001 between status quo and augmentation Scenario 4 (Weir, 2020).

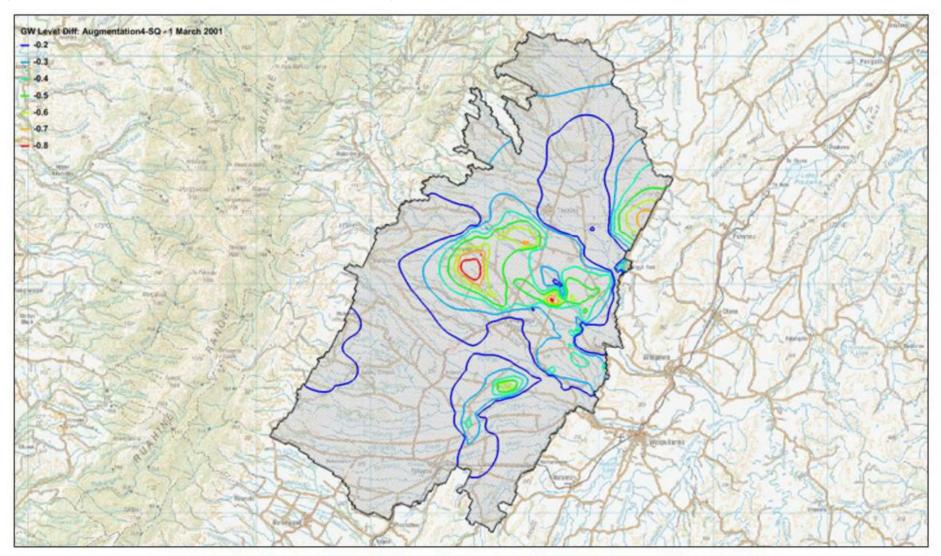


Figure 19: Difference in shallow (layer 1) groundwater levels at 1 March 2001 between status quo and augmentation scenario 4

APPENDIX B

Identified Wells bore logs

(data sourced from HBRC online database



IDENTIFICATION

WQ Site:

Easting: 1904521.139 Northing: 5585737.478

Method:

Address: 'TE HEKA' ARGYLL, TE

ONEPU RD,

Confined

WELL INFORMATION

Drill date: 19/01/1983

Driller: Honnor Drilling Limited

Casing Diameter (mm): 10

Bore Depth (m)

Well Depth (m): 5.18 Screen top (m): 0

Screen bottom (m): 0

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Aquifer Information

Inital Water Level Aquifer Condition

Aquifer Lithology

Aquifer Test

Test Reliability Unreliable

Specific Capacity Hydralic Conductivity

Storativity Transmissivity

Aquifer Thickness

Number Of Pumping Steps

Duration

Maximum Draw Down Maximum Pumping Rate

Report Number

Bore No 1357

Bore Log (m)

Lithology TOPSOIL with clay

From Depth 0 To Depth 2

Lithology brown GRAVEL

From Depth 2 To Depth 2

Lithology brown CLAY with limestone

From Depth 2 To Depth 3

Lithology blue/grey LIMESTONE

From Depth 3 To Depth 5



IDENTIFICATION

WQ Site:

Easting: 1893715.434 **Northing:** 5575727.936

Method:

Address: WAIPAWA-ONGA ONGA

ROAD, WAIPAWA

WELL INFORMATION

Drill date: 31/05/1990

Driller: Hill Well Drillers Ltd

Casing Diameter (mm): 15

Bore Depth (m)
Well Depth (m): 18
Screen top (m): 12

Screen bottom (m): 18

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Aquifer Information

Inital Water Level -7

Aquifer Condition Unconfined Aquifer Lithology Gravels

Bore Log (m)

Lithology TOPSOIL with gravel

 From Depth
 0

 To Depth
 0

Lithology brown GRAVEL

 From Depth
 0

 To Depth
 18



IDENTIFICATION

WQ Site:

Easting: 1901040.693
Northing: 5580363.708
Method: Hand-held GPS

Address: TE ONEPU ROAD, ARGYLL

WELL INFORMATION

Drill date: 25/02/1991

Driller: Baylis Brothers Limited
Casing Diameter (mm): 100

Bore Depth (m)

 Well Depth (m):
 4.11

 Screen top (m):
 2.11

 Screen bottom (m):
 4.11

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id WP180666T

Consent Type Ground-water consent

Use One Cropping
Use Two Imigation

Aquifer Information

Inital Water Level

Aquifer Condition Unconfined Aquifer Lithology Gravels

Aquifer Test

Test Reliability Unreliable

Specific Capacity Hydralic Conductivity

Storativity Transmissivity Aquifer Thickness

Number Of Pumping Steps
Duration 2

Maximum Draw Down
Maximum Pumping Rate 0

Report Number

Bore No 2902

Bore Log (m)

Lithology TOPSOIL From Depth 0

0 To Depth

 Lithology
 red GRAVEL

 From Depth
 0

 To Depth
 1

Lithology blue/brown CLAY
From Depth 1
To Depth 1

Lithology blue/brown GRAVEL From Depth 1
To Depth 4



IDENTIFICATION

2878

1899819.973 Easting: Northing: 5574177.986

Method:

WQ Site:

Address:

SWAMP ROAD, WAIPAWA

WELL INFORMATION

Drill date: Driller:

16/02/1995 Baylis Brothers Limited

Casing Diameter (mm): 100

Bore Depth (m)

Well Depth (m):

Screen top (m):

7.3

Screen bottom (m): Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id LU950015B Consent Type Bore consent Use One Pastoral Farming Use Two Stockwater Supply

Aquifer Information

Inital Water Level **Aquifer Condition** Unknown Aquifer Lithology Unknown

Bore Log (m)

Lithology TOPSOIL From Depth 0 To Depth

Lithology brown CLAY (light)

From Depth To Depth

Lithology red GRAVEL (metal)

From Depth 2 To Depth

Lithology brown CLAY

From Depth 2 To Depth 3

Lithology red GRAVEL with clay (metal, bentonitic)

From Depth

To Depth

Lithology brown CLAY with sand From Depth 9
To Depth 14

Lithology

yellow LIMESTONE 14 15 From Depth To Depth To Depth

Lithology blue CLAY with limestone (light)
From Depth 15
To Depth 19

blue LIMESTONE (hard) 19 22

Lithology From Depth To Depth

Lithology blue CLAY with shell From Depth 22
To Depth 24



IDENTIFICATION

WQ Site: Easting: 1900145.175 Northing: 5574350.167

Method:

Address: STOCKADE RD R D 2 ONGA

ONGA

WELL INFORMATION

Drill date: 10/07/1995
Driller: Hill Well Drillers Ltd
Casing Diameter (mm): 100

Bore Depth (m)

Well Depth (m): 25.6 Screen top (m): 14.03

Screen bottom (m): 25.6

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id LU950188B

Consent Type Bore consent

Use One Pastoral Farming

Use Two Stockwater Supply

Aquifer Information

Inital Water Level -8
Aquifer Condition Confined
Aquifer Lithology Other

Bore Log (m)

Lithology TOPSOIL
From Depth 0
To Depth 0

Lithology brown CLAY

From Depth 0 To Depth 1

Lithology brown/red GRAVEL with day (claybound)

From Depth 1 To Depth 3

Lithology white CLAY

From Depth 3 To Depth 4

Lithology brown CLAY

From Depth 4 To Depth 8

Lithology brown CLAY with gravel (hard)

From Depth 8 To Depth 12

Lithology brown LIMESTONE

From Depth 12 To Depth 13

Lithology brown CLAY with sand (hard)

From Depth 13 To Depth 16

Lithology brown LIMESTONE

From Depth 16 To Depth 17

Lithology brown CLAY

From Depth 17 To Depth 18

 Lithology
 blue CLAY

 From Depth
 18

 To Depth
 18

Lithology brown CLAY

From Depth 18 To Depth 19

Lithology brown LIMESTONE

From Depth 19 To Depth 19

Lithology brown CLAY with sand

 From Depth
 19

 To Depth
 20

Lithology brown LIMESTONE with shell

 From Depth
 20

 To Depth
 21

Lithology blue GRAVEL with shell

 From Depth
 21

 To Depth
 22

Lithology brown LIMESTONE

From Depth 22 To Depth 22 blue GRAVEL with shell

Lithology From Depth 22 To Depth 24

white LIMESTONE

Lithology From Depth 24 24 To Depth

blue CLAY with sand

Lithology From Depth To Depth 24 26



IDENTIFICATION

WQ Site: 2729

1900138.258 Easting: Northing: 5579253.713

Method:

11 ARGYLE EAST RD, Address:

WAIPAWA

WELL INFORMATION

Drill date: 09/08/1996 Driller:

Baylis Brothers Limited Casing Diameter (mm):

Bore Depth (m)

Well Depth (m):

7.5

Screen top (m):

Screen bottom (m): Open hole top (m): Open hole bottom (m):

Water level access: Yes

Bore Consents

Consent Id LU950259B Consent Type Bore consent

Use One Residential - Single property

Use Two Potable Supply

Aguifer Information

Inital Water Level Aquifer Condition Unknown Aquifer Lithology Unknown

Bore Log (m)

Lithology coarse blue GRAVEL (boulders)

From Depth 0 To Depth 6

Lithology blue/brown CLAY with silt

From Depth 6 To Depth 7

Lithology medium red GRAVEL (metal)

From Depth 10 To Depth



IDENTIFICATION

WQ Site:

1898782.044 Easting: Northing: 5575257.571 Differential GPS Method:

Address:

SWAMP ROAD, WAIPAWA

WELL INFORMATION

Drill date: Driller:

19/01/1996 Hill Well Drillers Ltd

300

Casing Diameter (mm):

Bore Depth (m)

Well Depth (m): 28 Screen top (m): 10.5 Screen bottom (m): 16.5

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id LU960099B Consent Type Bore consent Use One Pastoral Farming Use Two Imigation

Aquifer Information

Inital Water Level

Aquifer Condition Unknown Aquifer Lithology Gravels

Bore Log (m)

Lithology TOPSOIL From Depth 0 To Depth

Lithology brown CLAY

From Depth To Depth 3

blue GRAVEL Lithology

From Depth To Depth

Lithology brown GRAVEL (water bearing)

From Depth To Depth

Lithology brown CLAY From Depth 7 To Depth 8

Lithology brown GRAVEL (water bearing)

From Depth 8 To Depth 12

Lithology brown CLAY with gravel

From Depth 12 To Depth 12

Lithology brown GRAVEL (water bearing)

From Depth 12 To Depth 12

Lithology brown CLAY with gravel

 From Depth
 12

 To Depth
 13

Lithology brown GRAVEL (water bearing)

From Depth 13 To Depth 16

Lithology brown CLAY with gravel

 From Depth
 16

 To Depth
 17

Lithology brown GRAVEL (water bearing)

 From Depth
 17

 To Depth
 18

Lithology brown CLAY

From Depth 18 To Depth 20

Lithology brown GRAVEL

 From Depth
 20

 To Depth
 21

Lithology brown CLAY

 From Depth
 21

 To Depth
 21

Lithology brown/red GRAVEL

 From Depth
 21

 To Depth
 25

Lithology brown CLAY

From Depth 25 To Depth 26

 Lithology
 blue CLAY

 From Depth
 26

 To Depth
 28



IDENTIFICATION

WQ Site:

Easting: 1900701.622
Northing: 5579477.991
Method: Hand-held GPS

Address: 103 ARGYLL EAST RD,

WAIPAWA

WELL INFORMATION

Drill date: 21/09/2004

Driller: Honnor Drilling Limited

 Casing Diameter (mm):
 300

 Bore Depth (m)
 96

 Well Depth (m):
 18.5

 Screen top (m):
 9

Screen bottom (m): 18.5

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id LU040215B
Consent Type Bore consent
Use One Cropping
Use Two Irrigation

Consent Id WP180666T

Consent Type Ground-water consent

Use One Cropping
Use Two Irrigation

Aquifer Information

Inital Water Level -5
Aquifer Condition Confined
Aquifer Lithology Gravels

Aquifer Test

Test Reliability Unreliable Specific Capacity 26 Hydralic Conductivity 1111

Storativity

10000 Transmissivity Aquifer Thickness 9 Number Of Pumping Steps 1 Duration 2 Maximum Draw Down 2 Maximum Pumping Rate 50 230 Report Number Bore No 5211

Bore Log (m)

Lithology brown GRAVEL with silt

From Depth 0 To Depth 7

Lithology blue CLAY (greasy)

From Depth 7 To Depth 10

Lithology brown GRAVEL (water bearing)

From Depth 10 To Depth 10

Lithology brown GRAVEL with clay/silt

From Depth 10 To Depth 14

Lithology brown GRAVEL (water bearing)

From Depth 14 To Depth 19

Lithology brown CLAY

 From Depth
 19

 To Depth
 20

Lithology brown GRAVEL (larger than previous gravel and slightly WB)

 From Depth
 20

 To Depth
 30

Lithology fine brown GRAVEL (less water bearing)

From Depth 30 To Depth 34

Lithology brown GRAVEL (water bearing)

 From Depth
 34

 To Depth
 36

Lithology fine brown GRAVEL (less water bearing)

 From Depth
 36

 To Depth
 40

Lithology brown GRAVEL (water bearing)

From Depth 40 To Depth 43

Lithology brown GRAVEL with clay (little water)

From Depth 43 To Depth 51

Lithology blue CLAY

From Depth To Depth

fine brown GRAVEL (little water) 52 54 Lithology

From Depth To Depth

fine blue/brown GRAVEL (little water) 54 68 Lithology

From Depth To Depth

Lithology blue SAND with peat/veg/wood/silt (traces of pumice)

From Depth 68 96 To Depth



IDENTIFICATION

WQ Site:

Easting: 1895261.385
Northing: 5581232.095
Method: Hand-held GPS

Address: SH50, TIKOKINO

WELL INFORMATION

Drill date: 05/05/2006

Driller: INFRACON LIMITED

Casing Diameter (mm): 1200 Bore Depth (m) 5 Well Depth (m): 5

Screen top (m): Screen bottom (m): Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id LU060280B
Consent Type Bore consent

Use One Miscellaneous Industry
Use Two Wastewater - Wash Water

Aquifer Information

Inital Water Level

Aquifer Condition Unconfined Aquifer Lithology Gravels

Aquifer Test

Test Reliability Unreliable Specific Capacity 13

Hydralic Conductivity

Storativity Transmissivity Aquifer Thickness

Number Of Pumping Steps

Duration 300 Maximum Draw Down 2 Maximum Pumping Rate 20

Report Number

Bore No 5532

Bore Log (m)

To Depth

Lithology grey GRAVEL

5

From Depth 0



IDENTIFICATION

WELL INFORMATION

WQ Site: Easting: 1893682.943 Northing: 5587021.816 Method:

Hand-held GPS

Address: HOLDEN ROAD, TIKOKINO Drill date: 08/03/2007 **Baylis Brothers Limited** Driller:

Casing Diameter (mm): Bore Depth (m) Well Depth (m): 15.5 15.5 Screen top (m): 11.5 Screen bottom (m): 15.5

Open hole top (m): Open hole bottom (m):

Water level access: Yes

Bore Consents

LU070160B Consent Id Consent Type Bore consent Use One **Exploratory Drilling** Use Two **Environmental Purposes**

Aquifer Information

Inital Water Level -14 **Aquifer Condition** Unconfined Aquifer Lithology Gravels

Bore Log (m)

Lithology TOPSOIL with gravel

From Depth 0 To Depth 0

Lithology brown GRAVEL

From Depth To Depth 16



IDENTIFICATION

WELL INFORMATION

WQ Site: Easting:

1898417.493 5581833.22

Northing: Method:

Address:

WAIPAWA - TIKOKINO ROAD,

WAIPAWA

Drill date: 12/12/1991

Driller: Honnor Drilling Limited 150

Casing Diameter (mm): Bore Depth (m)

Well Depth (m): 13.1 Screen top (m): 10.36

Screen bottom (m): 11.58

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Aquifer Information

Inital Water Level

Unconfined **Aquifer Condition** Aquifer Lithology Gravels

Bore Log (m)

TOPSOIL with gravel Lithology

From Depth To Depth 0

Lithology coarse brown GRAVEL

From Depth 0 To Depth

Lithology red GRAVEL with clay (tight compact metal)

From Depth To Depth

Lithology brown GRAVEL

From Depth 7 To Depth 8

Lithology coarse brown GRAVEL with clay (traces of clay)

From Depth To Depth 13



IDENTIFICATION

WELL INFORMATION

WQ Site:

Easting: 1893813.563 Northing:

Method:

5584834.302

TIKOKINO Address:

Drill date: 30/07/1973

Driller: Honnor Drilling Limited

Casing Diameter (mm): Bore Depth (m)

28.35 Well Depth (m): 21.88 Screen top (m): Screen bottom (m): 22.82

Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Aquifer Information

Inital Water Level **Aquifer Condition** Confined Aquifer Lithology Gravels

Bore Log (m)

Lithology red GRAVEL with clay (metal)

From Depth 0 To Depth 19

Lithology coarse red GRAVEL (metal)

From Depth 19 To Depth 21

Lithology brown CLAY (firm)

From Depth 21 To Depth 23

Lithology coarse GRAVEL with clay (water yielding, metal)

From Depth 23 To Depth 26

Lithology brown CLAY (firm)

26 From Depth To Depth 28

Lithology red GRAVEL (water yielding, metal)

From Depth 28 28 To Depth



IDENTIFICATION

WQ Site:

1877285.248 5563499 22 Easting: Northing: Hand-held GPS Method:

5122 SH2 Address:

5122 SH2 Takapau

WELL INFORMATION

Drill date: Driller:

Casing Diameter (mm): Bore Depth (m) Well Depth (m): 1000 4

Screen top (m):

Screen bottom (m): Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Aquifer Information

Inital Water Level

Aquifer Condition Confined

Aquifer Lithology



IDENTIFICATION

WQ Site: Easting: 1897308.177

Northing: 5579545.339 Method: Hand-held GPS

Address: 1365 Tikokino Road, Waipawa

WELL INFORMATION

Drill date: 15/04/2015

Driller: Honnor Drilling Limited

Casing Diameter (mm): 150 Bore Depth (m) 6 Well Depth (m): 6

Screen top (m): Screen bottom (m): Open hole top (m): Open hole bottom (m):

Water level access: Unknown

Bore Consents

Consent Id LU150068B
Consent Type Bore consent
Use One Investigations

Use Two Environmental Purposes

Aquifer Information

Inital Water Level -4

Aquifer Condition Aquifer Lithology

Bore Log (m)

Lithology TOPSOIL From Depth 0
To Depth 2

 Lithology
 GRAVEL

 From Depth
 2

 To Depth
 6

APPENDIX F - Proposed Resource Consent Conditions



RESOURCE CONSENT

Water Permit

In accordance with the provisions of the Resource Management Act 1991 (RMA), and subject to the attached conditions, the Hawke's Bay Regional Council (the Council) grants a resource consent for a discretionary activity to:

[Consent Applicant NAME] [Address]

to take and use Tranche 2 groundwater to irrigate the following:

Consent Holder Name	Type of Cro	р		Area (ha)
Te Awahohonu Forest Trust	Pasture, horticulture	crops	and	850
Papawai Partnership	Pasture, horticulture	crops	and	320
Tuki Tuki Awa	Pasture, horticulture	crops	and	136
Plantation Road Dairies	Pasture, horticulture	crops	and	459
Springhill Dairies	Pasture, horticulture	crops	and	188
I & P Farming	Pasture, horticulture	crops	and	310
Buchanan Trust No.2	Pasture, horticulture	crops	and	230
Purunui Trust	Pasture, horticulture	crops	and	93

[Notes:

- 1. The applicant seeks final conditions that allow for flexibility of use of any Tranche 2 water taken. This will allow crop rotations (temporary) and permanent transitions to other land uses. It is not possible at this stage to specify exactly when, and on what parcels of land that landuse change will occur. Any change in land use, to a less water demanding crop, will also allow a greater area of land to benefit from irrigation. This will increase the positive effects arising from the water take, while any effects will be addressed in the land use consent. The precise wording of these conditions will need to be further discussed with Council.
- 2. While the draft conditions are presented in a collective form to ensure consistency and avoid duplication, the Applicants are requesting that individual water permits be granted to each applicant.]

LOCATION Address of site

[address]

Legal description

Site of use: [legal]

and as illustrated on the Site Map adjacent

LAPSING OF CONSENT

This consent shall lapse in accordance with section 125 of the RMA on [date] if it is not given effect to before that date.

CONSENT DURATION

This consent is granted for a period of 20 years commencing on 1 June [202X], and expiring on [date].



Malcolm Miller Manager Consents

REGULATION GROUP
Under authority delegated by Hawke's Bay Regional Council
[Date]

CONDITIONS

- 1. The site of the take shall be from Well no [insert] at map reference NZMG [easting / northing] and [legal description of site].
- 2. The rates of taking for irrigation and augmentation shall not exceed the following limits:

Consent Holder Name	Rate of take for irrigation (from all wells in combination) (L/s daily average)	Rate of take for augmentation (from all wells in combination) (L/s daily average)	Total rate of take (from all wells in combination) (L/s daily average)
Te Awahohonu Forest Trust	340	189	529
Papawai Partnership	126	24	150
Tuki Tuki Awa	94	5	99
Plantation Road Dairies	280	103	383
Springhill Dairies (formerly Ingleton Farms)	85	38	123
I & P Farming (formerly Abernethy Partnership)	115	22	137
Buchanan Trust No.2	91	51	142
Purunui Trust	43	14	57

3. The volume taken for irrigation and augmentation shall not exceed the following limits:

Consent Holder Name	Volume of take for Irrigation - from all wells in combination within the 12-month period (1 October to 30 September in consecutive calendar years) (m³)	Volume of take for augmentation - from all wells in combination within the 12-month period (1 November to 31 October in consecutive calendar years) (m³)	Total volume of take from all wells in combination in any 28-day period (m³)	Total volume of take from all wells in combination within the 12-month period (1 October to 30 September in consecutive calendar years) (m³)
Te Awahohonu Forest Trust	2,841,220	2,073,700	823,200	4,914,920
Papawai Partnership	1,010,817	464,700	304,552	1,475,517
Tuki Tuki Awa	678,100	29,100	228,480	952,400
Plantation Road Dairies	2,418,225	1,333,000	677,040	3,751,225

Consent Holder Name	Volume of take for Irrigation - from all wells in combination within the 12-month period (1 October to 30 September in consecutive calendar years) (m³)	Volume of take for augmentation - from all wells in combination within the 12-month period (1 November to 31 October in consecutive calendar years) (m³)	Total volume of take from all wells in combination in any 28-day period (m³)	Total volume of take from all wells in combination within the 12-month period (1 October to 30 September in consecutive calendar years) (m³)
Springhill Dairies (formerly Ingleton Farms)	588,313	416,900	205,950	1,005,213
I & P Farming (formerly Abernethy Partnership)	916,010	284,000	278,880	1,200,010
Buchanan Trust No.2	786,594	359,200	220,080	1,145,794
Purunui Trust	370,321	184,600	104,160	554,921
Total	9,609,600	5,145,700	2,842,342	15,000,000

4. Prior to 1 September each year, the consent holder shall notify Council (Manager Compliance) in writing what percentage of the total consented area has been developed for irrigation with the consented Tranche 2 groundwater, what maximum seasonal volume of consented Tranche 2 groundwater will be taken to irrigate the developed area, and what associated augmentation rate will be applied over the next water year (1 October to 30 September).

This condition shall apply until such time as the entire consented area has been developed for irrigation and the maximum seasonal volume of consented Tranche 2 groundwater can be taken to irrigate the total developed area.

[Note: This condition does not apply to Tuki Tuki Awa]

- 5. The taking of Tranche 2 groundwater for irrigation by Tuki Tuki Awa shall only occur when their existing surface water take (Consent No. WP XXX) is restricted due to low flow bans on the Tukituki River.
- 6. Should at any stage a consent holder intend to drill a new well for the purpose of taking water for irrigation and/or augmentation authorised by this consent, having first obtained the necessary bore permit (RRMP Rule 1 or 2) and having subsequently drilled the well the consent holder shall thereafter apply for a change of consent conditions under s127 of the RMA (or its successor) to add the well to Condition X and shall at that time also submit a report from a suitably qualified expert to the Council (Manager Compliance) (or nominee) that includes the following information:
 - a) Details of the new well, including its depth, location, screening and static water level;
 - b) An assessment of potential adverse (well interference) effects of take(s) from the well(s) on neighbouring groundwater users within a 2 km radius of each well; and

- c) Results of a pump test that demonstrates that the well can sustain the intended rate of take.
- 7. A water meter with a data logger and telemetry unit(s) compatible with the Council's telemetry system shall be installed on each well used for irrigation and/or augmentation prior to the use of the well for those purposes, and be operated and maintained to measure the volume of water taken to an accuracy of +/- 5%.
- 8. The device(s) required by Condition 7 shall be installed and maintained in accordance with the Council's "Technical Specifications and Installation Requirements for Flow Meters" (February 2010) (See Advice Note I).
- 9. Water take and use data supplied to the Council in accordance with conditions of this consent shall be collected by a water measuring device or system that has been verified by a suitably qualified person to be accurate to within +/-5% at that point of take within the following time periods:
 - a) For existing devices or systems: within the previous 5 water years (water year is 1 October 30th September); or,
 - b) For new devices or systems: before the end of the first water year (ending 30 September) for that water permit.
- 10. All water measuring devices or systems shall be re-verified by a suitably qualified person as accurate to within +/-5% within a maximum of 5 years from the date of the previous verification.
- 11. Where a portable pump is used to take water as authorised by this consent, both the water meter and telemetry devices must be installed, operated and maintained in accordance with the Council's Technical Publication "HBRCs Requirements for the use of Portable pumps used to report water use" (February 2013) (see Advice Note I).
- 12. The telemetry unit(s) shall record the rate and volume of take every 15 minutes. Each 15 minute interval of data shall be date and time stamped with the New Zealand Standard Time at the end of the 15 minute interval.
- 13. Data shall be transmitted to the Council's telemetry system at least once per day.
- 14. The telemetry unit(s) shall be installed so as to provide an accurate record of the flow meter data by a suitably qualified person. A record of installation shall be provided to the Council (Manager Compliance) in writing using the Council's "Telemetry Installation Form" within one week of installation of the new or reinstalled unit(s) having occurred (see Advice Note I).
- 15. A manual water meter reading shall be taken during the month of September each year. The water meter reading and the date and time the reading was taken shall be provided in writing to the Council (Manager Compliance) prior to 10 October each year.
 - <u>Advice note</u>: It is recommended that a photograph of the meter, with the meter reading clearly visible, is also provided at the same time as the reading required by condition 15.
- 16. Where the telemetry equipment fails, the consent holder shall notify the Council (Manager Compliance) of the failure within 3 working days, shall read the water meter at daily intervals and shall provide the Council with a record of the following:
 - a) The meter reading (in cubic metres); and,

- b) The daily volume of water taken (in cubic metres); and,
- c) The date and time of each reading.

This information shall be supplied no later than 7 days after the end of each calendar month. Where the telemetry equipment is returned to full operation, the information shall instead be supplied within 7 days of this return to full operation occurring.

- 17. The consent holder shall, upon request by the Council (Manager Compliance), supply details of the crop type and areas irrigated under this consent.
- 18. The consent holder (except Tuki Tuki Awa) shall commence the discharge of augmentation water (sourced from Tranche 2 groundwater taken in accordance with this consent) when the Council provides notification that the low flow rate at any of the following river sites is triggered:

River	Low Flow Rate (L/s)	
Waipawa River at RDS/State Highway 2	2,725	
Tukituki River at Tapairu Road	2,360	
Tukipo River at State Highway 50	155	
Tukipo River at Ashcott	1,085	
Mangaonuku River at Upstream of the Waipawa River Confluence	1,295	

Augmentation shall be undertaken regardless of whether the consent holder is irrigating at the time, using Tranche 2 groundwater authorised by this consent.

- 19. Tuki Tuki Awa shall discharge augmentation water (sourced from Tranche 2 groundwater authorised by this consent) when, and for the duration that, they are using Tranche 2 groundwater for irrigation authorised by this consent.
- 20. Augmentation required under Conditions 18 and 19 shall be discharged at the following relevant minimum rates and locations:

Consent Holder Name	Augmentation Discharge Location	Minimum Rate of Augmentation Discharge (L/s daily average)
Te Awahohonu Forest Trust	Mangaonuku Stream	189
Papawai Partnership	Discharge to shallow groundwater well adjacent to the Waipawa River	24
Tuki Tuki Awa	Tukituki River	5
Plantation Road Dairies	Kahahakuri Stream	103
Springhill Dairies (formerly Ingleton Farms)	Mangaonuku Stream	38

I & P Farming (formerly Abernethy Partnership)	Discharge to unnamed stream, tributary of the Tukituki River	22
Buchanan Trust No.2	Ongaonga Stream	51
Purunui Trust	Waipawa River	14

- 21. The consent holders (except Tuki Tuki Awa) shall cease augmentation when either:
 - a) the flow rates at all river sites exceeds the low flow rates identified under Condition 18; and/or
 - b) the volume of augmentation has reached the maximum volume of take for augmentation for the relevant 12-month period allowed under Condition 3.
- 22. No water shall be taken during "no take" periods specified by the Council for the purpose of obtaining accurate hydrological measurements, provided that:
 - a) the "no take" period specified by Council is no longer than twenty four (24) hours in duration; and,
 - b) the Council gives at least 7 days' notice to the consent holder of the start and finish time of the "no take" period; and,
 - c) consecutive "no take" periods are separated by an interval of at least 14 days.
- 23. All works and structures relating to this resource consent shall be designed and constructed to conform to best engineering practices and at all times maintained to a safe and serviceable standard.
- 24. The consent holder shall undertake all operations in accordance with any drawings, specifications, statements of intent and other information supplied as part of the application for this resource consent. In the event that there is conflict between the information supplied with the application and any consent condition(s), the condition(s) shall prevail.
- 25. Where spray filling and/or fertigation or injection of agrichemicals into the irrigation system (chemigation) is to occur, the consent holder shall ensure that the irrigation system is designed, constructed and maintained in accordance with the Irrigation New Zealand "New Zealand Guideline for the Safe Management of Irrigation Systems with Effluent, Fertiliser and/or Agrichemical Injection" (28/02/14) (see Advice Note VI) and to prevent the movement of contaminants into groundwater or surface water. The consent holder shall provide the details and specifications of the back flow prevention device/system at the request of the Council (Manager Compliance).
- 26. If an event occurs on-site that may lead to contamination of groundwater or surface water the Consent Holder shall notify the <insert name of registered drinking water supply> and the Hawke's Bay Regional Council (Manager Compliance) of the event as soon as reasonably practicable after the event occurs.

<u>Advice Note:</u> Such an event might include for example a chemical or effluent spill. The <name of registered drinking water supply> can be contacted on <insert phone number>. The Regional Council 24 hour Pollution Hotline should also be contacted on 0800 108 838.

- 27. To minimise the risk of contaminants entering groundwater, the consent holder shall:
 - a) Ensure that well headworks are constructed and maintained to prevent any leakage and/or movement of water or contaminants between the ground surface and groundwater, and shall ensure that there are no openings through which contaminants might enter the well. This shall include (but not be limited to) ensuring that there are no gaps around any pipework and/or cables at the wellhead.
 - b) Ensure that the well is maintained and serviced by a suitably qualified and experienced person at a frequency suitable for ensuring that condition 13(a) is met, and provide records of this maintenance and servicing to the Council (Manager Compliance) upon request.
 - c) In the absence of sufficient records to demonstrate to the satisfaction of the Council (Manager Compliance) that condition 13(a) is met, the consent holder, upon request by the Council (Manager Compliance), shall engage at their cost a suitably qualified and experienced person to inspect and certify that the wells(s) meet the requirements of condition 13 (a). The certification shall be provided to the Council (Manager Compliance) within 7 days of its receipt.

<u>Advice note</u>: For the purposes of this condition, an acceptable "suitably qualified and experienced person" is a professional well driller or well engineer (or equivalent), with demonstrable experience in the field of wellhead security, design, construction and maintenance.

REVIEW OF CONSENT CONDITIONS BY THE COUNCIL

The Council may review conditions of this consent pursuant to sections 128, 129, 130, 131 and 132 of the RMA. The actual and reasonable costs of any review undertaken will be charged to the consent holder (unless specified otherwise), in accordance with section 36 of the RMA.

Times of service of notice of any review: During the month of May, in any year.

Purposes of review:

To deal with any adverse effect on the environment which may arise from the exercise of this consent, which it is appropriate to deal with at that time or which became evident after the date of issue;

To require that the installation and reading of the water-measuring device or water meter data reporting system is consistent with any policies or rules in a regional plan, a National Environmental Standard;

To modify any monitoring programme, or to require additional monitoring if there is evidence that current monitoring requirements are inappropriate, in accurate or inadequate;

To ensure that the rate and volume of water authorised by the consent is consistent with actual water needs for an efficient take and is physically able to be taken;

To modify or add any condition to ensure that water is allocated in accordance with an operative plan;

To modify and/or add conditions of consent in order to ensure that it is consistent with the operative provisions of a regional plan. This shall include (but not be limited to) conditions specifying any maximum or minimum levels, minimum flows and associated implementation

timeframes, and/or abstraction rates or volumes (including allocation limits) (see Advice Note)

REASONS FOR DECISION

The effects of the activity on the environment will not be more than minor. Granting the consent is consistent with the purpose and principles of the RMA, the requirements of any relevant NPS, Regulations, NES regulations and with all relevant plans and policies.

ADVICE NOTES

Water Meter Technical Specifications

I. The following documents are available from the Council's website "Technical Specifications and Installation Requirements for Flow Meters" (February 2010) (www.hbrc.govt.nz/services/water/water-metering/meters/) and "HBRCs Requirements for the use of Portable pumps used to report water use" (February 2013) (www.hbrc.govt.nz/assets/Document-Library/Technical-Publications/Technical-Specifications-and-Installation-Requirements-for-portable-pumps-March-2013.pdf). The Telemetry System Installation Form is provided to telemetry installers by the Council upon request.

Water Take Records

II. Where no water is taken over an extended period the Council (Manager Compliance) may authorise that records be provided at intervals exceeding one month

Notification of Changes to Details

III. It is the responsibility of the consent holder to inform the Council (Manager Consents) if any details regarding this consent change, including any sale / purchase of the property and any change to contact details.

Spray filling, Fertigation and Chemigation

IV. The guideline referred to in condition 17 is available from the Irrigation New Zealand website (www.irrigationnz.co.nz). An appropriate backflow prevention mechanism for spray filling might include (but not be limited) the maintenance of an air gap between the inflow pipe and the receiving spray fill tank.

Water Quality Testing

V. It is the responsibility of the consent holder to ensure that the water abstracted under this resource consent is of suitable quality for its intended use. Where water is to be used for human consumption, the consent holder should have the water tested prior to use and should discuss these requirements with a representative of the Ministry of Health and should consider the following Drinking Water Standards (see link below): https://www.health.govt.nz/system/files/documents/publications/drinking-waterstandards-2008-jun14.pdf

MONITORING NOTE

Routine monitoring

Routine monitoring inspections will be undertaken by Council officers at a frequency of no more than once every year to check compliance with the conditions of the consent. The costs of any routine monitoring will be charged to the consent holder in accordance with the Council's Annual Plan of the time.

Non-routine monitoring

"Non routine" monitoring will be undertaken if there is cause to consider (e.g. following a complaint from the public, or routine monitoring) that the consent holder is in breach of the conditions of this consent. The cost of non-routine monitoring will be charged to the consent holder in the event that non-compliance with conditions is determined, or if the consent holder is deemed not to be fulfilling the obligations specified in section 17(1) of the RMA shown below.

Section 17(1) of the RMA states:

Every person has a duty to avoid, remedy, or mitigate any adverse effect on the environment arising from an activity carried on by or on behalf of the person, whether or not the activity is carried on in accordance with

- a) any of sections 10, 10A, 10B, and 20A; or
- b) a national environmental standard, a rule, a resource consent, or a designation.

Consent Impact Monitoring

In accordance with section 36 of the RMA (which includes the requirement to consult with the consent holder) the Council will levy additional charges for the cost of monitoring the environmental effects of this consent, either in isolation or in combination with other nearby consents. Any such charge would generally be set through the Council's Annual Plan process.

DEBT RECOVERY

It is agreed by the consent holder that it is a term of the granting of this resource consent that all costs incurred by the Council for, and incidental to, the collection of any debt relating to this resource consent, whether as an individual or as a member of a group, and charged under section 36 of the RMA, shall be borne by the consent holder as a debt due to the Council, and for that purpose the Council reserves the right to produce this document in support of any claim for recovery.

CONSENT HISTORY

Consent No.	Date	Event	Relevant Rule	Relevant Plan
	Xx/xx/xxx	Consent initially granted	55	Regional Resource Management Plan